MUSKOGEE COMMUNITY RECYCLING AND DISPOSAL FACILITY MUSKOGEE COUNTY, OKLAHOMA ODEQ PERMIT NO. 3551020

TIER III PERMIT MODIFICATION LANDFILL EXPANSION

VOLUME 3 OF 4

Prepared for

Waste Management of Oklahoma, Inc.

October 2023



Prepared by

Weaver Consultants Group, LLC CA 3804 PE 06/30/2025 6420 Southwest Boulevard, Suite 206 Fort Worth, Texas 76109 817-735-9770

WCG Project No. 0086-364-11-19

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CONTENTS

APPENDIX G Landfill Gas Management Plan

APPENDIX H Surface Water Management Plan

APPENDIX I USACE Information

APPENDIX J Alternative Final Cover System Design

APPENDIX K Quality Assurance/Quality Control Plan for Liner and Leachate Collection System Installation and Testing

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APPENDIX G

LANDFILL GAS MANAGEMENT PLAN

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CONTENTS

1	INTI	RODUCTION	G-1
2	МЕТ	HANE GAS MONITORING	G-2
	2.1	Perimeter Monitoring	G-2
		2.1.1 Perimeter LFG Monitoring Network	G-2
		2.1.2 Existing LFG Monitoring Probes	G-4
		2.1.3 Proposed LFG Monitoring Probes	G-4
		2.1.4 Monitoring Procedures	G-4
		2.1.5 Maintenance Procedures	G-5
	2.2	Facility Structure Monitoring	G-5
		2.2.1 Monitoring Procedures	G-5
		2.2.2 Maintenance Procedures	G-6
	2.3	Recordkeeping/Reporting	G-6
3	МЕТ	HANE GAS EXCEEDANCE ACTION PLAN	G-7
	3.1	Exceedance Response Measures	G-7
		3.1.1 Initial Action	G-7
	3.2	Notification Procedures	G-8
	3.3	Remediation Plan	G-8
4	LFG	CONTROL	G-9
	4.1	LFG Collection and Control System	G-9
APP Lano	ENDIX dfill Gas	G-1 Monitoring Probe Locations and Detail	PROFESSION 42
APP Prop	ENDIX posed at	G-2 nd Existing LFG Monitoring Probe Information	

APPENDIX G-3

Recordkeeping Form

Tables



Table 1	List of Existing and Proposed LFG Monitoring Probes	G-3
Table 2-1	Proposed LFG Monitoring Probe Information	G-2-1

1 INTRODUCTION

This Landfill Gas (LFG) Management Plan has been prepared consistent with Oklahoma Administrative Code (OAC) 252:515-15 requirements pertaining to the control of explosive gases from the Muskogee Community Recycling and Disposal Facility (Muskogee Community RDF). The site currently has an approved LFG monitoring system which includes nine (9) existing LFG monitoring probe locations. The approved LFG monitoring system in this document refers to gas monitoring probes currently installed along the permit boundary of the Muskogee Community RDF. As a result of the proposed landfill expansion, twenty-six (26) new LFG monitoring probes will be installed, and nine (9) existing LFG monitoring probes will remain in-place.

Section 2 of this plan describes the perimeter LFG monitoring system, and general practices and procedures for performing monitoring and maintenance of the landfill's perimeter LFG monitoring system. This section also includes an overview of the monitoring and maintenance procedures for the landfill's onsite structures. Finally, Section 2 includes an overview of the recordkeeping and reporting requirements contained within OAC 252:515-15.

Section 3 describes an action plan for the Muskogee Community RDF if explosive gases are detected above Oklahoma Department of Environmental Quality (ODEQ) compliance levels at either the perimeter LFG monitoring probes or within on-site structures.

Section 4 describes the landfill gas collection and control system (GCCS).

2 METHANE GAS MONITORING

Compliance with OAC 252:515-15 requires landfills to implement a routine methane monitoring program to verify that (1) methane concentrations do not exceed 25 percent of the lower explosive limit (LEL), or 1.25% methane by volume in air, within facility structures (excluding gas control or recovery system components) located inside the permit boundary and (2) the concentration of methane does not exceed the LEL, or 5% methane by volume in air for methane at the facility permit boundary.

The purpose of this section is to provide guidelines for the evaluation of LFG migration at the points of compliance. This will be verified by monitoring LFG concentrations at the facility permit boundary and within the onsite structures.

2.1 Perimeter Monitoring

2.1.1 Perimeter LFG Monitoring Network

The current ODEQ approved LFG monitoring probe network includes a total of nine (9) existing LFG monitoring probes located along the existing permit boundary as shown on Figure G-1-1 in Appendix G-1. These 9 existing LFG monitoring probes will remain in-place as no changes are proposed to the permitted landfill area.

Table 1 summarizes the LFG monitoring probes that will remain in-place, and the probes that will be added as a result of the proposed landfill expansion. The interprobe spacing of the proposed new probes will be approximately 500 feet per regulation 252:515-15-4(b)(1).

The new LFG monitoring probes will be installed prior to the development of the western landfill unit.

Existing Probes to Remain In-Place	Proposed Probe Locations to Be Added
GP-1R	GP-10
GP-2R	GP-11
GP-3	GP-12
GP-4R	GP-13
GP-5R	GP-14
GP-6	GP-15
GP-7	GP-16
GP-8	GP-17
GP-9	GP-18
	GP-19
	GP-20
	GP-21
	GP-22
	GP-23
	GP-24
	GP-25
	GP-26
	GP-27
	GP-28
	GP-29
	GP-30
	GP-31
	GP-32
	GP-33
	GP-34
	GP-35

Table 1List of Existing and Proposed LFG Monitoring Probes

2.1.2 Existing LFG Monitoring Probes

Locations of the existing LFG monitoring probes are shown on Figure G-1-1 in Appendix G-1. The as-built boring logs for the existing LFG monitoring probes are included in Appendix G-2.

The proposed landfill expansion includes a separate unit to the west of the permitted landfill area. No changes are proposed to the permitted landfill area. Therefore, no change is proposed to the currently approved perimeter LFG monitoring network.

2.1.3 Proposed LFG Monitoring Probes

As part of the proposed landfill expansion, twenty-six (26) LFG monitoring probes designated as GP-10 through GP-35 as listed in Table 1 will be installed around the new permit boundary prior to the development of the western landfill unit. The new LFG monitoring probes will be installed in accordance with applicable rules in OAC 252:515-7-3. After evaluating the site's soil, hydrogeologic, and hydraulic conditions surrounding the facility, the new LFG monitoring probes are designed to be single-completion probes. At a minimum, the new probes will extend from ground surface down to the proposed lowest bottom of waste elevation.

The single-completion probe design was chosen since it assures that all soil layers are monitored, reducing the possibility of undetected gas monitoring through an unsaturated zone. In the event that LFG migration is detected, and knowledge of the specific zone of migration is needed for development of the remediation plan, additional temporary probes may be installed next to the original probe and within the suspected zones of migration.

The new LFG monitoring probes will be installed by an Oklahoma-licensed monitoring well installer. The new boreholes will be logged by a geologist or engineer. Within 90 days of installation, detailed as-built drawings of the probes will be submitted to the ODEQ. Please refer to Figure G-1-3 for the proposed LFG monitoring probe detail.

2.1.4 Monitoring Procedures

Methane concentrations will be measured using a portable gas detection device pre-calibrated against reference methane standard prior to the beginning of each sampling event. The portable gas detection device will be equipped with a suction sampling line. The sampling line will be connected to the top of each probe to enable gas samples to be drawn directly into the monitoring instrument without diluting the sample. The instrument will give a direct reading of the methane concentration in either percent of the LEL or percent methane by volume. A qualified landfill representative or a qualified consultant will conduct compliance monitoring. The monitoring equipment used will be maintained and calibrated in accordance with the manufacturer's recommended procedures prior to use.

Monitoring data will be recorded on the Landfill Gas Monitoring Report (LGMR) form in Appendix G-3, or a similar form, and maintained in the Site Operating Record.

If LFG monitoring determines that methane has been detected in concentrations exceeding the regulatory threshold, notification procedures, as described in Section 3.2, and remediation procedures, as described in Section 3.3, will be implemented and followed.

2.1.5 Maintenance Procedures

During the LFG monitoring events, the sampler will inspect the integrity of the monitoring probes. The sampler will record pertinent information on the LGMR form (Appendix G-3) or similar form.

If damage or excessive wear to the monitoring probe is observed, it will be reported to the landfill manager and the monitoring probe will be repaired. If it is not possible to repair the monitoring probe and the damage could potentially affect the accuracy of future monitoring results, the monitoring probe may need to be abandoned and replaced with a new monitoring probe, with approval from the ODEQ.

2.2 Facility Structure Monitoring

2.2.1 Monitoring Procedures

Onsite enclosed structures used for human occupation will be equipped with a continuous LFG monitor/alarm that provides an audible alarm if methane concentrations exceed, at a minimum, 25 percent of the LEL for methane (or 1.25 percent methane by volume). During each quarterly sampling event, the continuous monitors will be checked per manufacturer recommendations. The verification will be documented on the LGMR form.

If methane concentrations exceeding the regulatory thresholds are detected within a structure, notification procedures, as described in Section 3.2, and remediation procedures, as described in Section 3.3, will be implemented and followed. If existing enclosed structures are removed from the site to allow for the continued development of the landfill, the monitors/alarms installed in these structures will be decommissioned.

2.2.2 Maintenance Procedures

The continuous LFG monitors/alarms will be maintained in accordance with the manufacturer's recommendations and specifications. In addition, on a quarterly basis the monitors/alarms will be inspected to ensure they are properly installed and connected to power.

2.3 Recordkeeping/Reporting

Records of LFG monitoring, whether for routine monitoring, notification, or remediation purposes, will be maintained and placed in the Site Operating Record. The LFG monitoring probes and alarms will be monitored at a minimum of quarterly, and the results will be placed in the Site Operating Record.

3.1 Exceedance Response Measures

This action plan has been prepared consistent with OAC 252:515-15-5 in order to protect human health in the event that concentrations of methane exceed ODEQ compliance thresholds either in facility enclosed structures within the permit boundary or at the LFG monitoring probe. The appropriate emergency response is different for each situation; therefore, the plan will address the situations for buildings and probes separately.

These action plans will be implemented upon the initial exceedance of a perimeter monitoring probe or enclosed structure monitor.

3.1.1 Initial Action

The initial action in the event methane is detected at levels above regulatory threshold is to immediately take necessary steps to protect human health. The specific response depends on the circumstances of the situation.

Building/Structures. If a monitoring device in a facility within the permit boundary is triggered or if continuous LFG monitor/alarm equipment indicates that 25 percent of the LEL (1.25 percent methane by volume) has been exceeded, the building is to be immediately evacuated of all personnel and the Landfill Manager will be notified. Personnel (except for qualified monitoring personnel) will not be allowed to reenter the affected structure until additional measures are taken. Notification procedures will be implemented as described in Section 3.2.

Perimeter Monitoring Probes. If a level above the regulatory threshold of methane is detected at the permit boundary in one of the monitoring probes, the Landfill Manager will be notified immediately. The immediate emergency response measure will be for the Landfill Manager to determine if any nearby buildings (including off site structures) are at risk and if evacuation of the building(s) should be requested. Notification procedures will be implemented as described in Section 3.2.

3.2 Notification Procedures

When methane levels above the regulatory threshold have been accurately detected, notification will be made to ODEQ. The site will also submit a written report to ODEQ describing the methane gas levels detected and the steps taken to protect human health. This report will be submitted to ODEQ within seven days following detection.

3.3 Remediation Plan

Once methane levels above regulatory threshold has been accurately detected in the facility buildings/structures or in one or more LFG monitoring probes, a specific remediation plan will be submitted to the ODEQ within 30 days of detection, describing the nature and extent of the problem and the proposed remedy. The remediation plan will then be implemented within 60 days of detection or as approved by the ODEQ. The ODEQ will be promptly notified in writing that the remediation plan has been implemented and a copy will be placed in the site operating record. However, the ODEQ may establish an alternate schedule.

The initial remediation action will be an investigation of the cause of the methane levels. The remediation plan may include some or all of the following elements, depending on the circumstances:

- Bar-hole probe or hydropunch testing in the vicinity of the impacted monitoring probe
- Sampling and laboratory analysis of gas samples collected from the monitoring probe to determine the source of the gas and the concentration of methane and other compounds
- A gas analysis to determine the source
- Additional LFG monitoring
- Adjustments to nearby LFG extraction wells

Using accumulated data, an assessment will be made to determine an appropriate course of action to mitigate the LFG migration, if needed. Such action may vary with the specific incident, but may include (and are not limited to) increasing the vacuum or re-tuning the existing LFG collection and control system (GCCS) and/or installation of the following:

- Passive vents
- Cut-off trenches
- Expansion to the existing GCCS

4.1 LFG Collection and Control System

The Muskogee Community RDF is currently installing an initial landfill gas (LFG) collection and control system (GCCS) at the site. The initial GCCS consists of vertical LFG extraction wells, horizontal LFG collectors, a piping network, condensate management system, and LFG control facility as shown in Figure G-1-1 on Appendix G-1. The gas collection piping system conveys the extracted LFG from the collection points (i.e., vertical wells and horizontal LFG collectors) to the flare facility.

The initial GCCS will be expanded as needed to control LFG and in accordance with 40 CFR 60 New Source Performance Standards, Subpart XXX and NESHAP 40 CFR 63 Subpart AAAA for MSW Landfills. The installation of future GCCS components is addressed in the site's NSPS/NESHAP GCCS Design Plan.

APPENDIX G-1

LANDFILL GAS MONITORING PROBE LOCATIONS AND DETAIL





<u>LEGEND</u>



EXISTING PERMIT BOUNDARY PROPOSED PERMIT BOUNDARY PERMITTED LIMITS OF WASTE PROPOSED LIMITS OF WASTE STATE PLANE COORDINATE GRID EXISTING CONTOUR EXISTING OVERHEAD EASEMENT IN-PLACE FINAL COVER EXISTING LANDFILL GAS MONITORING PROBE LFG EXTRACTION WELL LFG COLLECTION PIPING CONDENSATE SUMP ISOLATION VALVE LCR CONNECTION REMOTE WELLHEAD HORIZONTAL LFG COLLECTOR BLIND FLANGE HDPE CAP ROAD CROSSING CONDENSATE FORCEMAIN AIR SUPPLY LINE CONDENSATE/AIR VALVE AIR STUB

NOTES:

- 1. EXISTING CONTOURS AND ELEVATIONS PROVIDED BY HYDREX ENVIRONMENTAL COMPILED FROM AERIAL PHOTOGRAPHY FLOWN 01-25-2023.
- 2. THE EXPANSION WILL ADD A SEPARATE UNIT TO THE WEST OF THE EXISTING LANDFILL. NO CHANGES ARE PROPOSED TO THE EASTERN UNIT.
- 3. THE CITY OF MUSKOGEE LANDFILL IS NOT PART OF THE PROPOSED EXPANSION AND IS SHOWN FOR REFERENCE PURPOSES.
- 4. PERMIT BOUNDARY AND EXISTING OVERHEAD EASEMENTS WERE REPRODUCED FROM LEGAL DESCRIPTION PREPARED BY WEAVER CONSULTANTS GROUP, SIGNED BY MICHAEL D BYTNER, LLS# 1986.
- GCCS SHOWN REPRESENTS DESIGN LOCATIONS PROVIDED BY BIGGS & MATHEWS ENVIRONMENTAL, INC.

MANA	PREPARED FOR GEMENT OF OKLAHOMA, INC.	TIER III PERMIT	MODIFICATION
	REVISIONS		
DATE	DESCRIPTION	MUSKOGEE CO MUSKOGEE COU	DMMUNITY RDF NTY, OKLAHOMA
		WWW.WCGRP.COM	FIGURE G-1-1



0 300 SCALE IN	0 600 FEET
LEGE	ND
	EXISTING PERMIT BOUNDARY
	PROPOSED PERMIT BOUNDARY
	PERMITTED LIMITS OF WASTE
	PROPOSED LIMITS OF WASTE
N 273,000	STATE PLANE COORDINATE GRID
640	EXISTING CONTOUR
	IN-PLACE FINAL COVER
561	BORROW AREA CONTOUR
770	FINAL COVER CONTOUR
	PROPOSED DRAINAGE SWALE
	PROPOSED DRAINAGE CHOTE
	RIPRAP/GABIONS
OHE	EXISTING OVERHEAD EASEMENT
⊛ ^{GP−5R}	EXISTING LANDFILL GAS MONITORING PROBE
⊚ ^{GP-10}	PROPOSED LANDFILL GAS MONITORING PROBE

NOTES:

- EXISTING CONTOURS AND ELEVATIONS PROVIDED BY HYDREX ENVIRONMENTAL COMPILED FROM AERIAL PHOTOGRAPHY FLOWN 01-25-2023.
- 2. THE EXPANSION WILL ADD A SEPARATE UNIT TO THE WEST OF THE EXISTING LANDFILL. NO CHANGES ARE PROPOSED TO THE EASTERN UNIT.
- 3. THE CITY OF MUSKOGEE LANDFILL IS NOT PART OF THE PROPOSED EXPANSION AND IS SHOWN FOR REFERENCE PURPOSES.
- PERMIT BOUNDARY AND EXISTING OVERHEAD EASEMENTS WERE REPRODUCED FROM LEGAL DESCRIPTION PREPARED BY WEAVER CONSULTANTS GROUP, SIGNED BY MICHAEL D BYTNER, LLS# 1986.
- PERMITTED FINAL COVER CONTOURS AND DRAINAGE REPRODUCED FROM THE TIER I PERMIT MODIFICATION PREPARED BY WEAVER CONSULTANTS GROUP, LLC IN NOVEMBER 2021 AND APPROVED BY ODEQ ON JANUARY 24, 2022.
- 6. LOCATIONS OF PROPOSED LFG MONITORING PROBES ARE APPROXIMATE. ACTUAL LOCATIONS MAY VARY BASED ON FIELD CONDITIONS AT THE TIME OF INSTALLATION.

PREPARED FOR MANAGEMENT OF OKLAHOMA, INC.	TIER III PERMIT PROPOSED LFG	MODIFICATION PROBE LAYOUT
REVISIONS		
ATE DESCRIPTION	MUSKOGEE CC MUSKOGEE COU	DMMUNITY RDF NTY, OKLAHOMA
	WWW.WCGRP.COM	FIGURE G-1-2



APPENDIX G-2

PROPOSED AND EXISTING LFG MONITORING PROBE INFORMATION

Table 2-1¹

Proposed LFG Monitoring Probe Information Muskogee Community Recycling and Disposal Facility Muskogee County, Oklahoma

Probe I.D.	Probe Ground Surface Elevation (ft-msl) ²	Lowest Bottom of Waste Elevation (ft-msl)3	Proposed Probe Bottom Elevation (ft-msl)	Proposed Probe Boring Depth (ft-bsg) ⁴
GP-10	626	616	616	10
GP-11	630	616	616	14
GP-12	632	616	616	16
GP-13	636	616	616	20
GP-14	636	616	616	20
GP-15	632	616	616	16
GP-16	636	616	616	20
GP-17	640	616	616	24
GP-18	646	616	616	30
GP-19	656	616	616	40
GP-20	648	616	616	32
GP-21	646	616	616	30
GP-22	630	616	616	14
GP-23	622	616	612	10
GP-24	614	616	604	10
GP-25	622	616	612	10
GP-26	618	616	608	10
GP-27	628	616	616	12
GP-28	642	616	616	26
GP-29	644	616	616	28
GP-30	644	616	616	28
GP-31	640	616	616	24
GP-32	638	616	616	22
GP-33	634	616	616	18
GP-34	632	616	616	16
GP-35	630	616	616	14

¹ The information presented on this table is approximate. Actual elevations and dimensions will be determined based on site conditions at the time of installation.

² Probe ground surface elevations based on January 24, 2022, aerial survey provided by Hydrex Environmental.

³ Lowest bottom of waste elevations based on Drawing 8 – Top of Liner Plan included in Permit Drawings of the proposed Tier III permit modification.

⁴ Although the bottom of waste is only few feet below the ground surface, a minimum of 10 feet of probe depth will be used and the probe depth is measured in feet below surface grade (ft-bsg).

EXISTING PROBE BORING LOGS

SITE	NAME	AND	LOCAT	ION:	DRILLING ME	THOD: Holl	low-ster	Auger				BORIN	IG NUI	HBER:	
h	Musko	gee	Com	munity Landfill								GP	-1R		
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DATUM	1:			ELEVATION: 631,75	CASING D	EPTH						9/9/	97	9/9	/97
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				road gravel, asphalt (base	of road determin	ned by drillin	ng char-						L	ı 	
- 1				acteristics, rock caugh	t in dase of sam	pler.)			\mathbb{N}	L.	e.				
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					Filter Placement:	9/9/97	16: 04	9/9/97	16: 15	
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000	200	Depth	String (s)	Elevation						
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		from su	rface, concrete	e pad above seal						
		Bontoplin Control	l-Inch Bentonit	anallate						
		bentonite Seak i/A	ed during place	ment						ith.
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		Filter Pack: 20/40	Colorado Silica	Sand, filter pack						<u> </u>
	1	immedia	itely beneath b	entonite seal					27	SIV:
		1/4-inc	h pea gravel, e	extending 1 foot						IPEF
••		above	and below scre	ened interval						ស
T.D. =	= 11.25 ft.	vi -								1



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	CLAY, brown			-Concrete	•			Γ
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				-Bentonit	e — 1		1	
	5.0			630			1	
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							1	
				-Sand 20/40	— + ⊟.		1	
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	26.0	1	······································	618-Well Ca	, <u>;</u> ;	- ر ک		
	27.0 WEATHERED SHALE+, yellowish-brown			617				
_	Boring Terminated at 27 Feet							
	Stratification lines are approximate. In-situ, the transition may be	e gradual.	Hammer Type: A	utomatic			l	L
			+Classification es and petrographic	timated from d analysis may re	isturbed samples eveal other rock to	. Core sa /pes.	mples	
volvano Pow	æment Method: er Auger		Notes:					
band	onment Method:	-						
	WATER LEVEL OBSERVATIONS				I			
	Not Encountered While Drilling	1] [are=	Well Started: 5/29/2)14	Well Complete	d: 5/29/2	014	
	Not Encountered After Boring		Drill Rig: ATV 945		Driller: TJ			
		Tuisa, Okia	noma Project No.: DD1445	05				

				SOI	L BORE	HOL	E LO	G				.,				
SITE	NAME		OCAT	ION:	DRILLING ME	THOD: H	lollow-stem /	luger				BORIN	IG NUI	IBER:		
	lusko	gee	Comin	unity Landfill								GP	-3			
	lusko Gas P	igee, robe	Ok. Inst	allation	SAMPLING ME	THOD: (5-foot Conti	nuous Sa	moler	• • • • • • • • • • • • • • • • • • •	[Sheet	1 of 2		
3	32788	3.100											DRIL	LING		Inc.
					1		INI			STA	TIC	STA	RT	F1N	ISH	nts,
					WATER LI	EVEL						TIN	IE	TI	ME	sulta
					TIME							12:4	18	13:	32	G
ក់សារបំ	14			ELEVATION 640 46		CDTU	<u> </u>							DA	TE	acon
	RTG:	Truck-	-mount	ed CME	CASING D			NS: Cla	voa	d built o) Silty	2/24	fill on	south	1/95	Lerr:
ANGLE	:	11 UQA	mount	BEARING:	<u></u>	end of	landfill		<u>y pa</u>		i Ontj	City			(1103)	
SAMPL	E HAM	MER T	ORQUE	E: ft.−lbs.												C10
50	[ay 11 - 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1					tur.		1	TES	TRES	ULTS		TRA
TION	/6"	ERY	5	nre	CRIPTION			E E	ТҮР	/FT SING	9	2	0	0	STS	
HIN	OWS	20%	YMB	Mi Mi	OF			MPL	PLE	OWS	TER		STIC 11 %	CIFI VIT	TE	NG NI
EPT (EL)	ы В В В	RE	ŝ					A S	SAM	8 B B B B	MA		PLA LIM	SPE	HEB	RILL
<u> </u>	1	L										<u></u>		1	5	-
				Fill (Clay pad)				1		-				T	T	1.
								1								
			1	CLAY, Silty (fill); Dk. yellow	ish orange (IOYR	6/6)]					-	
-2	CLAY, Silty (fill); Dk. y				•			CS	Y							
				- 					LA							
- 3			1							-						
-			1						$\ \cdot \ $							ţ.
- 4			$\mathbb{V}_{\mathbb{A}}$: iron oxide nodules					1	_						
_																
- 5									\mathbb{N}							
*									\mathbb{N}							
-6														1		
		100	1					ÇS					1			
-7		ļ							$ \Lambda $							
-																
- 8										-					1	
-			-	and the standard state of the s		1	. 1		1	· · •					5	
- 9 -			1/1	ish brown (IOYR 5/4)	to dark vellwish	om moder orange (I	ate yellow- IOYR 6/6) w/		T	1 -					2	
L			1X	abundant FeO ₂ nodu	les, blocky struc	ture				-						
			1/4							-						
- 11			1/						11	-						
- "		70	1					27	_ ¥	-						5
- 12			1					03					ĺ			Smi
		:	1							-						0.
F			1/2					1		-			ľ			
han set			V/							1						60
- 14			1/4	: as above					_	1 _						GGE
F			1				s	CS	X	, -			ŀ			1.9
- 15			1/2	1					+	4 -						

				SOIL	BORE	HOL	E LC)G						م مسالم			
SITEN	IAME		OCAT	ION:	DRILLING ME	THOD: H	ollow-sterr	n Auger					BORIN	IG NUN	HBER:		l
М	usko	gee	Comn	nunity Landfill						 	GP-3						
M G	usko as Pi	ğee, robe	Ok. Inst	allation	SAMPLING METHOD: 5-foot Continuous Sampler							Sheet 2 of 2					
3	2788	แก่ก											DRILLING				Inc.
Ú.	<i>.</i> ,00						INI				STA	тіс	START FINISH			ISH	ants,
					WATER L	EVEL							TIM	1E	TIME		sult
					TIME	-					_		12:-	18	13:	32	Co
													DA" DA	ie Vos	DA 2/2	TE	ACON
DATUM:		-	mount	ELEVATION: 640.45	LASINGL	SUBEAC			Clay	Dac	built or	Silty	clay	fill on	south	lwest	ferr
ANGLE:	10. 1	HUCK		BEARING:		end of	landfill										Į,
SAMPLE	НАМ	MER 1	ORQU	E: ftlbs.		-			<u></u>		•••••••			·····			
tia										ш			TES	TRES	SULTS		ļ
FEE	/6" "LER	ΞRY	ы.	nes	CRIPTION			f		TYP	/FT	3	2	0.	05	STS	ĉ
NI H				рцс М/	OF					ЫЕ	OWS	TER		STI 111	E E F	Ш Н м	
ЕРТ) (ЕЦ	ON	문	S					6	n∢	SAM	щS	MAN		PLA L	SPE	E E	14
<u> </u>									l						1	0	-
											-						
- 16										$\setminus /$	-						
										\mathbb{N}	-						
17				: roots					CS		-				ŀ		1
			47					ŀ		A	1						
18]		ŀ				
-			1														
- 19			1/4					-			-			ľ			
-			1							\mathbb{N}	, I I						
20			X	1							-		Ļ				
- -			5														1
- 21			1	: wet at 21 ft still in	fill				00	IV							
-			V/	1					ΰS	I.	-						l
22 			ΎΧ	SILT, clayey (ML); base of	fill (?) dark ye	llowish or a	ange w/			IA	-	.:	ŀ				ľ
-			M	iron oxide staining, Fe along paleo-surfaces	O2 nodules, MnO (tractures)	2 staining				$\ \ $	-		ł				
- 23	ľ		\mathbb{W}		(// 00/11/00)					$\ $	-		-				
- 			ľł.	1						L	-						
- 24				TD = 24 ft.							-						
- 25																	
-											-				1		
- 26																	ľ
-																	
- 27										ŀ	_						ľ
F																	
- 18		 -									-						
E		ľ									1		Ĩ				
- 29											-	1					
ŀ.								ŀ		Ì		11					ľ
- 30	1	ł	- E							1	1 -	11		4	1		

						WELL	NoGI	⁵ -3	*****		_
			LANDFILL	GAS		Boring No X-	-Ref	GP-3		· · · · · · · · · · · · · · · · · · ·	-
			MONITORI	NG PRO	BE CONST	RUCTION	SUMM	ARY			
	P		Survey Coords _E	=2,730,451.54	4, N=271,705.52	Elevation Ground Le	evel640).45			-
						Top of Ca	asing4	3.04	• • •		- -
			Drilling Summary			Construction Tir	ne Log				
			Total Depth (ft): 24			Taek	Sta	rt Time	Fin Date	18h Time	11
• 0 🕅		8 22	Casing Stickup Heigi	n): 8.00 nt (ft): 3.19		Drilling HSA:	2/24/95	12: 48	2/24/95	13: 32	andf
XX	XX		Driller: Terracon Cor	sultants Inc.		Casing					nity L
						C ₁ Prot.:	4/14/95		4/14/95		mmur ahon
6	000000		Rig: Truck mounted (CME		C ₂ I" PVC:	2/24/95	13: 34	2/24/95	13:36	CC OK
:		ŏ ▼	Bit (s): Hollowstem a	uger		u <u>r</u> 1 vo.	2/24/33	13, 30	2/24/33	13. 50	soge voge
0	o∃so		Drilling Fluid: NONE v	/ HSA		Filter Placement: Development: N/A	2/24/95	13: 40	2/24/95	13: 52	Musi
5			_			Bentonite Seal:	2/24/95	13: 52	2/24/95	13: 53	AME
000	E o		Protective Casing: 4	-Inch diameter	r Aluminum	Cementing:	4/14/95	2	4/14/95		TE N CATI
000	= 00		Probe Design &	Specificatio	ons						LO I
000	100		Basis: Geologic Log	Geophysic	al Log					 	4
So E	00		Casing String (s); C	= Casing S =	= Screen	Probe Comment	S				
00	= 000		Depth	String (s)	Elevation						
00 =	000		+3.33 - 1.0	C ₁	643.78 -639.45						
00		0	+3.19 - 2.81	C ₂	643.64 -637.64						
000			2.81 - 22.81	5 <u>1</u>	637.64 -617.64						8
So E	00	04	-		_						
	0				I						
00	-00	04	Casing: CI 4-inch	diameter alumin	um						
00			Casing: C2 1-inch d	lameter PVC, S	ich. 40, flush joint						
00	Elo				and AO share tates						
	00		Screen: SI 1-Inch d 0.020-i	iameter PVC, S nch Slot	ich.40, flush joint						
000	1000			- *							
	000	00		70 task disead							
000	0		Filter Pack: 3/8 - 5 2.0 - 2	4.0 ft.	ter pea graver					•	
000	000		20/40	(no. 3) Colorad	lo Silica Sand						8
000		00	1.5 - 2.6) ft.							ECKE
000	000	0	Sand Seal: Bentoni	te							5
000	-09		concre	ie pad		1					Smith
000	000		1								Ö
			7) BY 1/95
000	0.000		: 1/4	-inch bentonit	e pellets	-					11SEL
3	0.00.0		1								PERV TE
											Sul
T.	0. = 24	4 ft.	1								



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SITE	NAME	AND L	OCAT	ION:	DRILLING MET	THOD: Ho	llow-sten	Auger				BORIN	G NUM	IBER:		
M	usko	gee	Com	nunity Landfill	· · · · · · · · · · · · · · · · · · ·							GP-4R				
G	as P	robe	Inst	allation	SAMPLING ME	THOD: 5	-foot Cor	itinuous Sa	mple	•		Sheet 1 of 1				
1(01806	3.200) .	<i>1</i> 1	2								DRILL	ING		
					WATEDIE					STA	TIC	STAF	RT	FIN	FINISH	
				2	TIME	YEL	13:27					12:4	3	13:	ME 27	
					DATE		9/9/97		i			DAT	E	DA	TE	
MUTA	:	1.12		ELEVATION: 641.98		ЕРТН						9/9/	97	9/9	/97	
	RIG: 1	ruck-	-mount	TED CME 75		SURF AC	E CONDIT	IONS: At	the w	estern i	edge o	f the l	haul ro	o bac	n wes	
AMPLE	E HAM	MER T	ORQUI	E: ftlbs.		side of t	anotili, çia	iy with sor	ne gr	ass, flat				· · · · ·	·	
ШЭ	~					L			-		T	TES	T RES	ULTS		
1710/	s/6 PLEI	ΈRΥ	ğ		DESCRIPTION			BIT	ТҮР	/FT SING			0.50	05	STS	
LEVA	LOWS	202 202	SYME	in.	OF MATERIAL			AMPI ND B	PLE	OWS I CAS	TENT		ILLS I	CIFI VIT	E E	
E E E	a S	ι Έ				:		0.4	SAN	Φő	SON €	25	53	SPE B.R.	THE	
			KN	CLAY, Silty (CL); mediur	n to soft Dark yellow	ish orange	e (IOYR 6/6	3) - (N	=		1	1			
-1 -2		12.5		to moderate yellow blocky, roots, rock	ish brown (10YR 5/4) fragments, dry to da) mottling, amp	massive to) CS	W	Lini						
- 3									ΙΛ	كمنان						
- 4		······		: as above with son	ne yellowish gray mot	tling, med	ium		$\left(\right)$	ulu						
-5 -8				: moist, very silty c	lay zone at 5.1 to 5.3	3 feet, abi	undant		\mathbb{N}	în lî						
-7		100		roots, color mode yellowish brown (l	rate yellowish brown OYR 4/2)	(IOYR 5/4) to dark	CS	X	Î.î.l						
- 8			\mathcal{U}	1 : moist at 5.8 to 5.8	3					- international distribution of the second sec						
-9			\mathcal{U}	Moist at 7.6 to 7. slight organic odd) and 8.0 to 8.4 feet or, possibly old slope	, abundan face	t roots,		$\left(\right)$. uilu						
- 10 - 11		100		: às above, slity cli	ay, less moist			CS	X		;					
- 12				increase in silt co	ntent, structure boo	ming block	iyi		- r	Luil			4			
- 13				: iron oxide nodules	common, with some	manganes	e staining			u În						
- 14			\mathcal{I}	at 12 feet	e. with blocky struct	ure. abuño	tant			The						
- 16			X	manganése staini	ng, moist					l III						
- 17 18				: slickensided clea fracturing	vage planes, 60 ⁰ pre	ferential				ալար						
- 19 - 20			H	: as above, medium	plasticity, moist					ساسات						
21				: as above with de	crease in staining					- Indu						
- 23				TD @ 22.7	r ft.					[111]			- - -		,	
- 25																
- 28												ľ				
27			ŀ] .	
- 20	l											1	1	1		

							WELL	NoG	P-04F	}		_
					~ * ~		Boring No X-	-Ref	GP-0	4R		
				LANDFILL	GAS							
	ī			MONITORI	NG PRO	BE CONST	RUCTION :	SUMM	ARY			
				Survey Coords	=272,299.63		Elevation Ground Le	evel 641	.98			
	ĺ				2,730,167.65		Pin Elevation642.	70 T	op of PN	/C Casing	645.6	0
									•			1
				Drilling Summary			Construction i ir	ne Log				
-				Total Depth (ft): 22	.5			Ste	irt	Fin	iah	
-0	XX			Borehole Diameter	(in): 6,62"			Date	Time	Date	Time	ndfil
	X			Driller: Terracon Co	nt (11): 3.02		0 - 22.5 feet	9/9/9/	10:35	9/9/9/	11:00	, L a
	1			Audie Thornburg, Lie	# 0135					:		mity
	X			and an off of the prove 🔍 t			Casing:					lahc
	K		111/	Rig: Truck mounted	CME 75		C ₁ Prot.:	9/11/97		9/11/97		ŭ ð
				Bit (s): Hollowstem a	uger with		Sell" PVC	9/9/97	15 56	9/9/97	15.57	999
	1			Tooth Bit					10,00	0,0,0,	10.07	lusk Isko
				Drilling Fluid: NONE			Filter Placement:	9/9/97	16: 04	9/9/97	16: 15	Z Z
-5				Brotactive Casing	i-inch diamatai	anodized aluminum	Development: N/A					AME
1				FIOTECTIVE Casing.			Bentonite Seal:	9/9/97	16: 15	9/9/97	16: 30	ATA
				Probe Design &	Specificatio	ons.	Cemerning/pact	5/11/5/		5/11/5/		LO 1
	-			Rasis Geologic Log			·	l.				
Ĩ			K	Casing String (s):	= Casing S =	Screen	Drobe Comment					1
	00	00			·		Probe Comment	5				
1	00	000		Depth	String (s)	Elevation						
	200	= 50		+3.82 - 1.18	C ₁	645.80 -640.80						
- 10	10	E		+3.62 - 9.00	C2	645.60 -632.98						
1	00	E		9.00 - 21.50	s ₁	632,98 -620,48		14				
·	00	Ξo		-								
8	000	E		1 -		-		•				
1	00	E			,							
	100	E		Casing: C1 5-foot	x 4-inch Dia. A	nodized Aluminum						
	00	o≡so		With loc	king cap Vometer BVC S	ch 10 Eluch loint						
	00	E			nameter rvu, o							
- 15	00	E		Screen: St 1-Inch o	liameter PVC, S	ch.40, flush joint						
	00	Ē		0.020-1	nch Slot							
	20											
	00	o∃ so		1								
1	00	o∃ õo		Grout Seal: Bentoni	te (5%)/ cemer	nt to approx 0.5-fee	t					
	00			from su	rface, concrete	e pad above seal						
	00			1								G
	00			2								
- 20				Bentonite Seal: 1/4	-inch bentoniti	e pellets.						1
	20			hydrate	ed during place	ment						lth.
1	00	o≡°o		4.3 - 7	feet BGS							Sm
Į.	00	00000		1								9
1	00	00000		1								97 97
1				Fliter Pack: 20/40	Colorado Silica	Sand, filter pack						SED 7/9/
1				immedia	itely beneath b	entonite seal						RVI 5
			l.	1/4-inc	n pea gravel, e and below sere	ened interval						SUPE
-	τ σ	<u> </u>	<u> </u>	abuve	שוע שבועת שעום	enea linchial						
1	્રાન્દ		011L	1			1					I.



SITE I	NAME	AND	LOCAT	ION:	DRILLING ME	THOD: H	ollow-ster	Auger				BORIN	IG NUI	HBER:	
М	lusko	gee	Com	nunity Landfill								GP	-5R	È	
G	lusko las P	gee, robe	Ok. Inst	allation	-foot Con	tinuous S	aminio			Sheet 1 of 1					
10	01800	3.200):				1000 001		ampie				DRILI	LING	
				54 1.1			INI			STA	TIC	STA	RT	FIN	ISH
					WATER LI	EVEL	dry					TIM	E	TI	ME
							8:29					8:0	0	8:	29
				ELEVATION: 635.43	CASING D	- EPTH	9/9/9/					DAT 9/9/	E 07		TE 707
RILL F	RIG: 1	Fruck	-mount	ed CME 75		SURFAC	E CONDIT	IONS: At	the (astern (edgé o	f the	haul r	oad or	n wes
IGLE:				BEARING:		side of	the landfill	•						and a second	
MPLE	E HAM	MER 1	ORQUE	E: ftIbs.			•••••		- <u>-</u>	· · · · · ·			:		
ц N Q L	ER.	۲۲			-				ы	μœ		TES	T RES	ULTS	ر ا
NI VI	WS/6	OVE %	MBOI	DES	OF			PLEF	L ⊥ U	IS/F	5 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	<u> </u>	Ц. К	ΞŢ	EST
ELE	BLO JN S	REC	γ		ATERIAL	1.		SAM	MPL	UC NO	NTEN	UDI	AST	ECII	ER T
5					· · · · · · · · · · · · · · · · · · ·				ŝ	_0	8			50	DTH
			IN	CLAY, Silty (CL); soft, mott	led dark yellowis	h orange	(IOYR 6/6)	with		F		1	r	İ	<u> </u>
	,			minor yellowish gray (5 massive, roots, iron oxi	Y 7/2), high plasi de nodules (5mm	ticity, mas 1	sive,		\mathbb{N}						
2		12.5	M	: as above with color c	hange to include	some rec	ldish brown	20	V	-					
			22					00	$ \Lambda $			ŀ			
3			R		e ^r			e N	$\ \ $			l.			
4			11	: as above with trace of	of fine sand, no r	oots, nod	ules commo	۶, I	1	=				-	
5		: :	R	westnered, moist	x .					-					
			11						M					:	
D		100	W						١V.					1	
7			11					CO	I				ŀ		
8			H												
			R						$ \rangle$].	ŀ
9			HH.	: as above					\mathbf{t}		1		^ی ن		
10		100	11					cs	IV						
ff:			HH	: black staining at 10.8	feet				$ \Lambda $				ŀ		
			M	L : slickensided cleavage	planes, blocky s	structure		-	1-						
12.	,		H I	· · · · · · · · · · · · · · · · · · ·									ŀ		
3			XЦ	: dry to moist			-	- `Z -		L I					
2				TD @ 13.0 ft.									-		
					,										i.
5				λ.											
16															
17	;			ξ. 	e'										1
ur.												1			
8							-1								
19									1						
			1					1	1	1 -1	1	1	1	1	1

G-2-14

Γ	ند <u>بج</u> ست. ۳۰						WELL	No	P-05F	<u>}</u>		
	F	=]∥			CAC	,	Boring No X	-Ref	GP-0	5R:		_
				LANUF ILL	UAJ Ng DDA		DUATTAN					
ļ				MUNITURI	NG PRU	DE CUNSI	RUCTION	5UMM	AKY			
				Survey Coords _N	I=273,262.44	,	Elevation Ground L	evel <u>638</u>	5.43			- 7 L
				<u> </u>	2,730,156.77	<u>.</u>	Pin Elevation <u>635.</u>	<u>69</u> T	op of PV	/C Casing	638.50	2
	ŀ			Drilling Summary			Construction Tir	ne Log			, , , , , , , , , , , , , , , , , , ,	1
				Total Depth (ff): 13.	0			Charles C	int-	Fin	4 oh	
Lo				Borehole Dlameter	(in): 6.62"		Tesk	Date	Time	Date	Time	
Ĩ			M	Casing Stickup Heig	ht (ft): 3.07		Drilling HSA:	9/9/97	8: 00	9/9/97	8: 29	and
	N		111	Driller: Terracon Cor	nsultants Inc.		0 – 13 feet					a liv
			M	Audie Thornburg, Lic	. #0135		Casino:					unmu modi
	N		20	RIG: Truck mounted	CME 75		C _l Prot.:	9/11/97		9/11/97		OKI2
-			M	Bit (s): Hollowstem a	uger with		C2 1" PVC and	in to time		الشفرة عراجا		gee, Jee,
			M	Tooth Bit			SINAA	9/9/9/	11:29	9/9/9/	11: 30	usko
				Drilling Fluid: NONE			Filter Placement:	9/9/97	11: 30	9/9/97	11: 51	X PR
			M	Protective Casing 4	l-inch diamete	r anódized aluminúm	Development: N/A		a di serrat	الشقار عار في ا	' in ca	AME
				Tracourte adding.			Cementing/pad:	9/9/97	11:51	9/9/97	11: 55	A T A
			M	Probe Design &	Specification	ons					-	L SI
				Basis: Geologic Log	Geophysic	al Log 🗌						
	000	200	M	Casing String (s): C	= Casing S =	= Screen	Probe Comment	S				
+5	200	000		Danih	Okelna (a)	Elevelles						
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	000	$\equiv 000$		+3.07 - 5.50	91. - Co	638,50 -629,93						
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APPENDIX G-3

RECORDKEEPING FORM

MUSKOGEE COMMUNITY RECYCLING AND DISPOSAL FACILITY LANDFILL GAS MONITORING REPORT FORM

Sampled by:		Date:
Time:	(Start)	(Finish) Temperature:
Weather:		Barometric Pressure (optional):
Monitoring Equ	uipment:	Date of Calibration:

Probe No.	% Methane 0-100	% ¹ LEL 0-100	Static Pressure "w.c." ² (Optional)	O ₂ % (Optional)	Probe Integrity Verified Yes/No
Oneite Struct	-	Verify if Conti	nuous LFG Alarm	Continu	ous LFG Alarm
Unsite Struct	lures	is Ope	erational	Activated	(LEL>25%) During
		YES	NO	YES	NO
		YES	NO	YES	NO
		YES	NO	YES	NO
		YES	NO	YES	NO
		YES	NO	YES	NO

% LEL = (20) x (observed % methane)
"w.c." - Inches Water Column

MUSKOGEE COMMUNITY RECYCLING AND DISPOSAL FACILITY MUSKOGEE COUNTY, OKLAHOMA ODEQ PERMIT NO. 3551020

APPENDIX H

SURFACE WATER MANAGEMENT PLAN

Prepared for

Waste Management of Oklahoma, Inc.

October 2023



Prepared by

Weaver Consultants Group, LLC CA 3804 PE – 06/30/2025 6420 Southwest Boulevard, Suite 206 Fort Worth, Texas 76109 817-735-9770

WCG Project No. 0086-364-11-19

CONTENTS

LIST	Г ОГ ТА	BLES	H-iii
LIST	Г OF AP	PENDICES	H-iv
1	INTF	RODUCTION	H-1
	1.1	Purpose	H-1
	1.2	Stormwater Management System	H-1
	1.3	Erosion and Sedimentation Control Plan	H-2
	1.4	Stormwater System Maintenance Plan	H-4
2	DRA	INAGE SYSTEM DESIGN	H-6
	2.1	Hydrology	H-6
		2.1.1 Wright-McLaughlin Adjustment of the Rational Method	H-6
		2.1.2 HEC-1	H-7
	2.2	Hydraulics	H-9
3	EXIS	TING AND POST-DEVELOPMENT DRAINAGE INFORMATION	H-11
	3.1	Site Drainage Patterns	H-11
	3.2	Effect of Proposed Development on Peak Flows Generated	
		from the Site	H-12
	3.3	Summary	H-12
4	CON	CLUSIONS	H-14
		Marsh	
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10/13/2023

LIST OF TABLES

<u>Table</u>		<u>Page</u>
3.1	Existing and Post-Development Comparison of Peak Flow Rates for the Design Storm Event	H-12

LIST OF APPENDICES

APPENDIX H-1 Figures

APPENDIX H-2 Proposed Post-Development Drainage Analysis

APPENDIX H-3

Post-Development Final Cover Erosion Control Structure Design

APPENDIX H-4

Post-Development Perimeter Channel, Stormwater Detention Pond, and Culvert Design

APPENDIX H-5 Post-Development Final Cover Soil Loss Calculations



1.1 Purpose

This Surface Water Management Plan is prepared as part of this Tier III Permit Modification for the Muskogee Community Recycling and Disposal Facility (RDF) consistent with OAC 252:515-17. This drainage plan addresses surface water drainage design and erosion control. Permit level plans and details are presented for the proposed drainage system in this appendix. Appendix H also includes a demonstration that the proposed landfill development will not significantly alter the existing drainage patterns. Note that the scope of this appendix is limited to the surface water drainage design and erosion controls.

This appendix includes the design of the final cover erosion control structures (i.e., chute and swale system), perimeter drainage channels, stormwater detention ponds, and culverts, as well as hydrologic calculations for the post-development condition only. Details and designs for the existing condition are included in the Tier 1 modification dated November 2021. Consistent with OAC 252:515-17-2, these facilities are designed to convey runoff produced from the 25-year storm event.

1.2 Stormwater Management System

This Drainage Design Report has been prepared in accordance with OAC 252:515-17. The proposed drainage improvements for this expansion include providing final cover erosion control structures (i.e., chutes and swales), perimeter drainage channels, and stormwater retention and detention areas. Figures depicting the layout of the drainage system are presented in Appendix H-1. As shown on Figure H-1-3, stormwater runoff in the post-development condition will be collected in swales located near the upper grade break on the landfill and on the 4 (Horizontal) to 1 (Vertical) sideslopes, and then conveyed to drainage letdown structures (chutes) and down the 25 percent slopes to the perimeter drainage system. The perimeter channels will be constructed before or in conjunction with fill being placed above existing grade in each landfill phase. Analysis of the post-development perimeter drainage system components is provided in Appendix H-4. The site is designed so that it does not significantly alter existing drainage patterns. As the site develops, the stormwater management system, including permanent erosion control structures (i.e., chutes and swales) will be constructed. This will provide for the conveyance of all stormwater generated from the developed portions of the site.

Surface runoff will be managed throughout the active life of the landfill to minimize the amount of stormwater that will come in contact with waste or enter the leachate collection system. Surface water will be controlled through the use of diversion berms, stormwater diversion channels, and stormwater detention ponds. Stormwater that comes into contact with waste at the working face area will be considered contaminated water and treated as leachate. Contaminated water at the working face will be contained by the containment berm as shown in Appendix L.

In accordance with OAC 252:515-17-3, the facility has been designed to prevent discharge of pollutants into waters of the State or waters of the United States, as follows:

- No discharge of solid waste or pollutants into or adjacent to waters of the State, including wetlands, that is in violation of the requirements of the Oklahoma Water Quality Management Plan will occur. During the active life of the facility all stormwater coming into contact with solid waste will be retained as contaminated water and treated or disposed of as outlined in Appendix L.
- No discharge of pollutants into or adjacent to waters of the United States, including wetlands, that violates any requirement of the Clean Water Act. Waste Management of Oklahoma, Inc. has received a permit from ODEQ to discharge stormwater runoff consistent with an OPDES Multi-sector General Permit for industrial activity.
- No discharge of dredged or fill materials to waters of the United States, including wetlands, that is in violation of the requirements under the Federal Clean Water Act, §404 will occur.
- No discharge of a nonpoint source pollution of waters of the United States, including wetlands, that violates any requirement of an area-wide or statewide Water Quality Management Plan that has been approved under the Federal Clean Water Act, §208 or §319, as amended will occur.

1.3 Erosion and Sedimentation Control Plan

During site development, measures such as best management practices (BMPs) will be employed to control erosion and sedimentation. BMPs may include the use of temporary rock riprap, silt fences, straw bales, check dams, interceptor swales and berms, temporary and permanent seeding and sodding, surface roughening, matting and mulching, sediment traps, and surface wetting for dust control.

Swales and chutes will be constructed upon placement of the final cover. In addition, each final cover area will be seeded upon the completion of final cover placement with introduced and/or native grasses. Mulch may be used to protect the seed against erosive velocities, allowing the native grass seed time to germinate. The vegetative layer will consist of a minimum of 36 inches of earthen material that is capable of sustaining plant life and will be seeded immediately following the application of the final cover in order to minimize erosion. A soil loss demonstration for the vegetative layer is included in Appendix H-5.

Drainage structures with flow velocities less than 5 feet per second (fps) will be protected from erosion by vegetation established over the structure upon its completion. Structures with flow velocities between 5 fps and 10 fps will be protected from erosion by using turf reinforcement or rock riprap/gabions. Structures with flow velocities greater than 11 fps will be protected from erosion by using rock riprap/gabions. See Figure H-1-3 for locations of riprap and gabion placement. Gabion and riprap design calculations are presented in Appendix H-4. Note that alternative materials such as turf reinforcement matting, concrete riprap, grouted riprap, or HDPE panels designed by a qualified engineer may be used as substitute materials. Velocity calculations for the drainage chutes and swales are presented in Appendix H-3. Velocity calculations for the perimeter channels are provided in Appendix H-4.

The following provides general guidelines of how the erosion control features will minimize sediment discharge from the site:

- Final cover will be placed as the site develops and permanent erosion control structures (chutes, swales, vegetation) constructed.
- Vegetation will be established on above-grade intermediate cover areas that remain inactive for long periods. The temporary vegetative cover will minimize erosion potential.
- All uncontaminated stormwater runoff from the site will be channeled through the perimeter channel system and/or stormwater detention ponds before being discharged from the site. Sediment that collects in the channels and stormwater detention ponds will be removed consistent with the stormwater system maintenance plan presented in Section 1.4 of this appendix.
- The operator is required by its OPDES stormwater permit to keep an updated stormwater pollution prevention plan (SWP3) on site at all times. This plan will contain details of specific measures used to control erosion throughout development of the site.

Runoff from a storm event that comes in contact with the active fill area will be contained by a containment berm (see Appendix L, Appendix L-7 – Containment and Diversion Berm Calculations for Details).

1.4 Stormwater System Maintenance Plan

Waste Management of Oklahoma, Inc. will restore and repair constructed stormwater systems such as channels, drainage swales, and chutes in the event of wash-out or failure from extreme storm events. In addition, the BMPs discussed in Section 1.3 will also be replaced or repaired in the event of failure. Excessive sediment will be removed, as needed, so that the drainage structures, such as the perimeter channels and stormwater detention ponds, function as designed. Site inspections will be performed by the landfill personnel.

The following items will be evaluated during the inspections:

- Erosion of daily and intermediate cover areas, final cover areas, perimeter ditches, chutes, swales, stormwater detention pond, berms, and other drainage features.
- Settlement of intermediate cover areas, final cover areas, perimeter ditches, chutes, swales, and other drainage features.
- Silt and sediment build-up in perimeter ditches, chutes, swales, and stormwater detention ponds.
- Obstructions in drainage features.
- Presence of erosion or sediment discharge at offsite stormwater discharge locations.
- Presence of sediment discharges along the site boundary in areas which have been disturbed by site activities.

Maintenance activities will be performed to correct damaged or deficient items noted during the site inspections. These activities will be performed as soon as possible after the inspection. The time frame for correction of damaged or deficient items will vary based on weather, ground conditions, and other site-specific conditions.

Maintenance activities will consist of the following, as needed:

- Placement of additional vegetation.
- Placement, grading, and stabilization of additional soils in eroded areas or in areas which have settled.

- Replacement or repair of riprap or other structural lining.
- Placement of riprap in eroded areas or in areas which have settled.
- Removal of obstructions from drainage features.
- Removal of silt and sediment build-up from drainage features.
- Repairs to erosion and sedimentation controls.
- Installation of additional erosion and sedimentation controls.

2.1 Hydrology

Hydrologic analyses were performed to derive design flow rates for the postdevelopment swales and chutes. The Wright-McLaughlin adjustment of the Rational Method, as described in Section 2.1.1, was used to determine the flow rates for the chutes and swales. The swale and chute drainage areas are presented in Appendix H-3, as well as the supporting calculations used to analyze the design of the chutes and swales. Perimeter channels and the stormwater ponds were modeled using HEC-1, as discussed in Section 2.1.2. The evaluation of these structures, including culverts, is included in Appendix H-4.

2.1.1 Wright-McLaughlin Adjustment of the Rational Method

Peak flow rates for the final cover erosion control structures (i.e., chutes and swales) were computed by the Wright-McLaughlin Adjustment of the Rational Method (Rational Method) using the following equation:

$$Q = C_f CIA$$

where:

- Q = Flow rate, cubic feet per second (cfs)
- $C_{\rm f}$ = Runoff correction factor
- C = Runoff coefficient
- I = Rainfall intensity, inches per hour (in/hr)
- A = Drainage area, acres (ac)

Runoff coefficients were obtained from Figure 7.6-A of the 2014 Oklahoma Department of Transportation (DOT) *Drainage Design Manual*. These coefficients are a function of land use, percent imperviousness, type of soil, and topography.

The rainfall intensity is determined from Figure 7.6-P in the 2014 Oklahoma DOT *Drainage Design Manual for Zone 2* as follows:

$$I = a / (t_c + b)^c$$

where:

a = 67 (constant for Muskogee County, 25-year storm)

- b = 10 (constant for Muskogee County, 25-year storm)
- c = 0.75 (constant for Muskogee County, 25-year storm)
- t_c = time of concentration, minutes (min)

The hydrologic analyses of the post-development chutes and swales are included in Appendix H-4.

2.1.2 HEC-1

HEC-1 was utilized to model the post-development conditions to determine peak flows entering and leaving the site. HEC-1 was developed by the USACE Hydrologic Engineering Center to simulate the surface runoff response of a watershed. The HEC-1 model represents a watershed as a network of hydrologic and hydraulic components. The modeling process results in the computation of stream-flow hydrographs at desired locations in the watershed. The hydrologic analysis of the proposed post-development condition is presented in Appendix H-2.

2.1.2.1 Watershed Subareas and Schematization

The landfill areas and upland area were delineated to derive a peak flow entering and leaving the site. The drainage areas for the post-development condition are shown in Appendix H-1, Figure H-1-1. Offsite drainage areas that drain on to the permit boundary are shown on Figure H-1-2.

2.1.2.2 Time Step

The time step, or the program computation interval, is the time interval that the flow rates for the hydrographs are generated by the program. The time step used for the design storm event is 5 minutes.

2.1.2.3 Hypothetical Precipitation

The hypothetical precipitation of the 25-year design storm was obtained from Technical Paper No. 40 Rainfall Frequency Atlas of the United States and National Oceanic and Atmospheric Administration (NOAA) Technical Memorandum HYDRO-35 for the project area. For this analysis, the design storm utilized was the 25-year, 24-hour storm event. The precipitation is assumed to be evenly distributed over the landfill area for each time interval.

2.1.2.4 Precipitation Losses

Precipitation losses (the precipitation that does not contribute to the peak runoff) for each drainage area are calculated using the Soil Conservation Service (SCS) Curve Number (CN) method. The CN is a function of soil cover, land use, and antecedent moisture conditions. A conservative CN of 88 was selected to represent the vegetative layer at the site. A CN of 100 was selected to represent the drainage area for the stormwater detention ponds. A CN of 84 was selected to represent the non-landfill areas and perimeter channels. A CN of 75 or 79 was selected to represent the upland areas, depending on the existing soil type.

2.1.2.5 Hydrograph Information

Two different types of hydrograph generation methods were used in the postdevelopment analyses including distributed runoff methods and the Snyder unit hydrograph method. Muskingum-Cunge and reservoir routing (pond storage discharge) methods were used for hydrograph routing through the stormwater detention ponds. Information for the model parameters used for the postdevelopment condition is included in this Appendix H-2.

Distributed Runoff Methods

The distributed runoff methods (e.g., kinematic wave) are applicable to small-water catchments with uniform slopes, channels, and drainage patterns. Conceptual drainage elements (overland flow length and channels) are used to represent the actual transfer of runoff within the drainage area in the distributed runoff methods. HEC-1 utilizes Kinematic Wave and Muskingum-Cunge routing to implement these methods. Landfill final cover areas consist of relatively short (typically 120 feet for the proposed post-development condition) overland flow lengths that drain into landfill final cover swales.

The kinematic wave and the Muskingum-Cunge methods have been used for estimating peak runoff rates for the drainage areas that drain to the perimeter channels and stormwater ponds. A hydrograph was developed using the kinematic wave method to simulate overland flow (e.g., landfill areas to swales, etc.), and the Muskingum-Cunge method was utilized to model channelized flow. Distributed runoff methods utilize a simplified form of the energy equation and is based on the characteristics of the drainage area, swale, or channel. Both of these methods use physical (measurable) characteristics (e.g., flow lengths, slopes, surface roughness coefficients, channel cross sections) of a watershed to estimate peak discharges.

<u>Snyder Unit Hydrograph Method</u>

The Snyder unit hydrograph method was used to develop hydrographs for the upland and non-landfill areas. This method is applicable to basins with a wide range of basin area, watershed length, slope, impervious cover, and conveyance characteristics. Several different methods have been developed to estimate Snyder unit hydrograph parameters (watershed lag and peaking coefficient). The Espey "10-Minute" method was used in this project to estimate Snyder unit hydrograph parameters.

Hydrograph Routing

The hydrograph at the pond outlets was generated by routing the inflow hydrograph through the detention volume for the stormwater detention ponds. Pond routing

was performed by utilizing the storage-discharge relationship for the stormwater detention pond by defining surface area versus elevation. Additionally, characteristics of discharge structures (low level outlet and spillway) were utilized for pond routing.

2.2 Hydraulics

Drainage structure details are provided in Appendix H-1. Drainage chutes will be trapezoidal with 8-foot (minimum) bottom widths and 2 (Horizontal) to 1 (Vertical) side slopes above the energy dissipater. The energy dissipater spacing and sizing are presented in Appendix H-3. Chutes are lined with geomembrane to protect against erosion. Drainage swales will be triangular and grass-lined unless noted otherwise on the permit drawings.

The swales, chutes, and perimeter channels are designed to safely convey the 25-year, 24-hour storm. These drainage features will also reduce maintenance at the site after closure by minimizing erosion. Hydraulic analyses of the swales, chutes, and channels have been conducted using Manning's uniform flow formula. The uniform flow formula assumption is applicable to long prismatic channels of uniform slopes, as proposed at the site.

The general form of Manning's equation is:

in which

V = Velocity of flow, fps (feet per second)

n = Manning's Roughness Coefficient

R = = Hydraulic radius, ft (feet)

- S = Friction slope for nonuniform flow or channel slope for uniform flow, ft/ft
- A = Area of water perpendicular to direction of flow, sf (square feet)

P = Wetted perimeter, ft

Using the relationship

Q = VA

Manning's equation can also be written as

Q =

The uniform flow assumption equates the channel slope to the friction slope; therefore, the slope of the channel can be used for "S" in Manning's formula for computation of uniform flow.

Typical values for Manning's Roughness Coefficient are presented in the 2014 Oklahoma DOT *Drainage Design Manual* (Figure 8.3-B). A value of 0.04 is used for all perimeter channels. A value of 0.03 is used for all swales. A value of 0.01 is used for all geomembrane-lined chutes. These values will yield maximum expected depths of flow in the drainage structures after vegetation has been established. Hydraulic calculations for the post-development swales and chutes are presented in Appendix H-3. Analyses of the post-development perimeter channels are presented in Appendix H-4.

3 EXISTING AND POST-DEVELOPMENT DRAINAGE INFORMATION

This section has been developed to show that the proposed landfill development will not significantly alter the existing drainage patterns at the site. The drawings depicting the proposed landfill development are included and Appendix H-1. Supporting calculations for the proposed landfill development are presented in Appendix H-2 (Post-Development Drainage Analysis). Drainage design information for the existing condition is included in the Tier I modification, dated November 2021. A comparison summary of the existing and post-development condition flows produced by the 25-year storm event are presented on Table 3.1 of this section.

The following three sections discuss: (1) site drainage patterns and (2) the effect of the post-development on peak flows generated from the site; and (3) a summary of the results.

3.1 Site Drainage Patterns

To demonstrate that the development of the landfill will not adversely impact existing drainage patterns, drainage areas were delineated (or subdivided as needed) to calculate peak flow rates entering and exiting the permit boundary. The results of these calculations are shown on Figure 3-1 for the existing and post-development conditions. The hydrologic calculations and drainage areas used for both conditions are included in Appendix H-2.

As shown on Figure 3-1, peak 25-year 25-hour flow rates entering the permit boundary from the offsite areas (O1 through O4) are the same for the existing and post-development conditions. Offsite runoff entering the permit boundary towards the eastern portion of the site is managed by a series of ponds and channels before discharging into roadside ditches. Offsite runoff entering the permit boundary to the western portion of the site either sheet flows off the site or is collected by a series of channels that discharge into a retention pond to the west of the proposed landfill development.

In both analyzed conditions, runoff exits the permit boundary at the same six locations, DP1 through DP6. The peak flows exiting the permit boundary are less than the existing flow rates exiting the permit boundary at the same locations.

3.2 Effect of Proposed Development on Peak Flows Generated from the Site

To assess the effects the proposed landfill development will have on runoff that is generated at the site, an analysis was developed to address peak flows from the landfill. In addition, Table 3.1 has been developed to facilitate a comparison of peak flow rates discharging offsite (from the permit boundary) at the discharge points and onsite (flow entering the permit boundary) from the upland area.

Table 3.1Existing and Post-Development Comparison of Peak Flow Rates for the
Design Storm Event

Upland Area and Stormwater Discharge Points	Existing Condition 25-Year, 24-Hour Peak Flow Rates (cfs)	Post-Development Condition 25-Year, 24-Hour Peak Flow Rates (cfs)
01	48	48
02	150	150
03	24	24
04	28	28
DP1	85	85
DP2	297	253
DP3	43	36
DP4	14	14
DP5	173	8
DP6	137	2

As shown on Table 3.1, the peak flow rates discharging offsite from the postdevelopment condition are the same or are less than the peak flow rates discharging offsite for the existing condition.

3.3 Summary

From the hydraulic and hydrologic evaluations of the post-development drainage plan, runoff generated from the landfill will be attenuated and controlled by the perimeter drainage structures prior to discharging offsite. Given that (1) the stormwater discharge outfall locations are consistent with the existing site discharge locations, (2) runoff from the 25-year design storm event will be attenuated and controlled by the stormwater management system, (3) erosion control features will minimize sediment discharge from the site, and (4) maintenance activities and site inspections will be performed on the stormwater management system following storm events, it is concluded that the post-development drainage plan will not adversely alter existing or permitted drainage patterns at the site.



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FIGURE 3-1

4 CONCLUSIONS

The stormwater management plan for this facility is designed to convey runoff from the 25-year, 24-hour storm runoff volumes and to minimize surface water flow onto the active fill area. The flow due to the development of the expanded disposal facility will be attenuated by the perimeter drainage structures before discharging offsite. As noted in Section 3, the post-development plan does not result in significant alteration of the existing drainage patterns. Temporary and permanent erosion control measures are provided to minimize potential sediment generated from the site. With this stormwater management system, no adverse impacts to the adjacent surface waters are anticipated.

The following conclusions summarize the results of this drainage analysis:

- The design criteria used for these drainage calculations meet the requirements of the Municipal Solid Waste Management Regulations of the ODEQ.
- Drainage analyses were conducted in accordance with the *Drainage Design Manual*, November 2014, of the Oklahoma Department of Transportation (DOT).
- Drainage structures (swales, chutes, culverts, perimeter channels, and stormwater detention ponds) are designed to convey the peak flow rates from the 25-year, 24-hour rainfall event.
- Erosion will be reduced using BMPs during site development.

APPENDIX H-1

FIGURES



CONTENTS

FIGURE H-1-1 Post-Development HEC-1 Drainage Areas

FIGURE H-1-2 Offsite Drainage Area Plan

FIGURE H-1-3 Post-Development Drainage Plan

FIGURE H-1-4 Perimeter Drainage Plan

FIGURE H-1-5 Drainage Details

FIGURE H-1-6 Drainage Details

FIGURE H-1-7 Drainage Details

FIGURE H-1-8 Drainage Details





		WWW.WCGRP.0	юм	FIGUR	E H-1-1
	DESCRIPTION	MUS MUSK	SKOGE OGEE	E COMMUNII COUNTY, OF	TY RDF KLAHOMA
REVISIONS			DRAI	NAGE ARI	EAS
PREPARED FO	OKLAHOMA, INC.	TIER III POST-	PER DEV	MIT MOD	IFICATION T HEC-1
0.0124	CHD	13.62		0.0213	
0.0141	СНС	4.52		0.0071	
0.1134	СНВ	10.94		0.0171	
0.0322	СНА	0.61		0.0010	
(SQ MILES)	P9	15.54		0.0243	
FFSITE	P8	45.30		0.0708	
	P7	5.84		0.0091	
	P6	1.34		0.0021	
	P5	1.96		0.0031	
	P4	0.68		0.0011	
	P3	3.52		0.0009	
	P2	0.60		0.0000	
	P1	4.77		0.0075	
	50	/.5/		0.0075	
	S7	20.69		0.0323	
	S6	6.97		0.0109	
	S5	3.79		0.0059	
	S4	3.02		0.0047	
	S3	0.62		0.0010	
	S2	51.48		0.0804	

600	L

<u>LEGEND</u> 640--561-770 ---OHE---⊕^{MW-5R} ♦ MW-9 (PWCG-1) ●^{GP-5} ⊙^{GP-10} ----(LF1)

LF1

LF2

LF3

LF4

LF5

LF6

S1

EXISTING PERMIT BOUNDARY PROPOSED PERMIT BOUNDARY PERMITTED LIMITS OF WASTE PROPOSED LIMITS OF WASTE STATE PLANE COORDINATE GRID EXISTING CONTOUR BORROW AREA CONTOUR FINAL COVER CONTOUR DRAINAGE SWALE DRAINAGE CHUTE DRAINAGE CHANNEL RIPRAP/GABIONS APPROXIMATE OG&E POWER LINE EASEMENT EXISTING GROUNDWATER MONITORING WELL PROPOSED GROUNDWATER MONITORING WELL EXISTING LANDFILL GAS MONITORING PROBE PROPOSED LANDFILL GAS MONITORING PROBE DRAINAGE AREA BOUNDARY

DRAINAGE AREA DESIGNATION

0.0401

0.0210

0.0223

0.0333

0.0362

0.0213

0.0011

0 0804

HEC-1 DRAINAGE AREAS - ONSITE

DRAINAGE AREA AREA (ACRES) AREA (SQ MILES)

25.64

13.41

14.29

21.29

23.14

13.63

0.69





HEC-1 DRAINAGE AREAS - OFFSITE			
DRAINAGE AREA	AREA (ACRES)	AREA (SQ MILES)	
01	20.60	0.0322	
02	72.56	0.1134	
03	9.01	0.0141	
04	7.91	0.0124	

	LICENSED	Charles R. Marsh 24599 10/13/2023
MA, INC.	TIER III PERMIT OFFSITE DRAINA	MODIFICATION AGE AREA PLAN
	MUSKOGEE COMMUNITY RDF MUSKOGEE COUNTY, OKLAHOMA	
	WWW.WCGRP.COM	FIGURE H-1-2





WWW.WCGRP.COM

FIGURE H-1-3




NOTES:

1. EXISTING CONTOURS AND ELEVATIONS PROVIDED BY HYDREX ENVIRONMENTAL COMPILED FROM AERIAL PHOTOGRAPHY FLOWN 01-25-2023.



PREPARED FOR		
ANAGEMENT OF OKLAHOMA, INC.	TIER III PEI	RMIT MODIFICATION
REVISIONS	PERIMETER	R DRAINAGE PLAN
TE DESCRIPTION	MUSKOGE MUSKOGEE	E COMMUNITY RDF COUNTY, OKLAHOMA
	WWW.WCGRP.COM	FIGURE H-1-4









1. REFER TO DRAWING $\rm H{-}1{-}3$ STORMWATER MANAGEMENT PLAN FOR LOCATION OF DETAILS. 2. SEE APPENDIX H-3 FOR CHUTE DESIGN SUMMARY.

PREPARED FOR MANAGEMENT OF OKLAHOMA, INC.	TIER III PERMIT MODIFICATION				
REVISIONS	DRAINAGE DETAILS				
DATE DESCRIPTION	MUSKOGE OKLAHOMA	E COMMUNITY RDF COUNTY, OKLAHOMA			
	WWW.WCGRP.COM	FIGURE H-1-7			





DRAFT FOR PERMITTING PURPOSES ONL ISSUED FOR CONSTRUCTION	Y	PREPARED FOR WASTE MANAGEMENT OF OKLAHOMA, INC.			TIER III PE DRAIN	RMIT MODIFICATION	
DATE: 10/2023	DRAWN BY: JDW	NO	DATE	REVISIONS			
CAD: FIG H-1-8.DWG	REVIEWED BY: CRM	NU.	DATE	DESCRIPTION	MUSKOGEE COMMUNITY RDF		
Woover Consult	ants Group				MUSKOGEE COUNTY, OKLAHOMA		
CA 3804 PE - 06/	CA 3804 PE - 06/30/2025				WWW.WCGRP.COM	FIGURE H-1-8	



APPENDIX H-2

PROPOSED POST-DEVELOPMENT DRAINAGE ANALYSIS

Includes Pages H-2-1 through H-2-67



CONTENTS

PROPOSED POST-DEVELOPMENT HEC-1 PEAK FLOW RATE ANALYSIS	H-2-1
Hypothetical Storm Data	H-2-2
Precipitation Loss Data	H-2-4
Hydrograph Development Information	H-2-11
Proposed Post-Development Hydrologic Analysis: HEC-1 Output	H-2-18



PROPOSED POST-DEVELOPMENT HEC-1 PEAK FLOW RATE ANALYSIS

HYPOTHETICAL STORM DATA

MUSKOGEE COMMUNITY RDF 0086-364-11-19 APPENDIX H HYPOTHETICAL STORM DATA

Hypothetical Storm Data

Precipitation data taken from TP-40 and Hydro 35 rainfall data.

Time	5 min	15 min	60 min	2 hr	3 hr	6 hr	12 hr	24 hr
25-Year Event	0.71 in.	1.54 in.	3.20 in.	3.88 in.	4.30 in.	5.28 in.	6.25 in.	7.25 in.

PRECIPITATION LOSS DATA

<u>Required:</u>	Determine the SCS curve numbers for the on-site drainage areas and the upland drainage areas for use in the HEC-1 analysis.
References:	1. Dodson's and Associates, Inc., Hands-On HEC-1, 1997.
	 United States Department of Agriculture, National Resource Conservation Service, Web Soil Survey for Muskogee County, Oklahoma (http://websoilsurvey.nrcs.usda.gov)
<u>Solution:</u>	The final cover system will be in-place and the vegetative layer will control precipitation loss. A curve number for the landfill erosion layer (e.g., drawing area LF1) was selected using the chart on page H-2-6 (Ref.1) based on soil information obtained information obtained from pages from pages H-2-7 through H-2-10 (Ref. 2).

Use: CN = 88

Perimeter channel drainage area (e.g., drainage area CHA) curve numbers were selected assuming fair vegetation establishment.

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Offsite areas (O1 to O4) and non-landfill areas (S1 to S9) curve numbers were selected based on the in-situ soil type in the area.

Use:	CN = 79	Hydrological Soil Group: D
Use:	CN = 75	Hydrological Soil Group: C/D
Use:	CN = 70	Hydrological Soil Group: C

The pond area is assumed to be at its capacity. Therefore, no losses from the pond surface will occur.



TABLE 5.3 Values of SCS
Curve Number for Rural Areas

Source: [McCuen, 1982]

	Н	ydrologic	Soil Gro	up
Land Use Description	Α	В	С	D
Fallow:				
Straight Row	77	86	91	94
Row Crops:				
Straight Row, Poor Condition	72	81	88	91
Straight Row, Good Condition	67	78	85	89
Contoured, Poor Condition	70	79	84	88
Contoured, Good Condition	65	75	82	86
Contoured and Terraced, Poor	66	74	80	82
Condition				
Contoured and Terraced, Good Condition	62	71	78	81
Small Grain:				
Straight Row, Poor Condition	65	76	84	88
Straight Row, Good Condition	63	75	83	87
Contoured, Poor Condition	63	74	82	85
Contoured, Good Condition	61	73	81	84
Contoured and Terraced, Poor Condition	61	72	79	82
Contoured and Terraced, Good Condition	59	70	78	81
Close-Seeded Legumes or Rotation Meadow				
Straight Row, Poor Condition	66	77	85	89
Straight Row, Good Condition	58	72	81	85
Contoured, Poor Condition	64	75	83	85
Contoured, Good Condition	55	69	78	83
Contoured and Terraced, Poor Condition	63	73	80	83
Contoured and Terraced, Good Condition	51	67	76	80
Pasture or Range:				
Poor Condition	68	79	86	89
Fair Condition	49	69	79	<mark>84</mark>
Good Condition	39	61	74	80
Contoured, Poor Condition	47	67	81	88
Contoured, Fair Condition	25	59	75	83
Contoured, Good Condition	6	35	70	<mark>79</mark>
Meadow, Good Condition	30	58	71	78
Woods or Forest Land:				
Poor Condition	45	66	77	83
Fair Condition	36	60	73	79
Good Condition	25	55	70	77
Farmsteads:	59	74	82	86

Initial and Uniform Loss Rate

An initial loss in inches (*STRTL*) and a constant loss rate (*CNSTL*) in inches per hour are specified for this method. All rainfall is lost until the volume of initial loss is satisfied. After the initial loss is satisfied, rainfall is lost at the constant rate.

This section provides guidance in selecting the values used for the initial loss and uniform loss rate in two ways:

- 1. By consulting previous studies of actual rainfall events for a particular watershed or region.
- 2. By relating the parameters to the SCS Curve Number, which can be estimated using the information presented earlier in this chapter.

Previous studies by the U.S. Army Corps of Engineers or other public agencies may provide guidance on selecting appropriate values for the initial loss and uniform loss rate for a particular location. Tables 5.4 through 5.6 list the values of initial and



H-2-7

Hydrologic Soil Group-Muskogee County, Oklahoma



Hydrologic Soil Group

Man unit symbol	Man unit name	Pating	Acros in AOI	Barcant of AOI
wap unit symbol		Kaung	Acres III AOI	Fercent of AOI
2	Bates loam, 1 to 3 percent slopes	С	9.6	1.9%
3	Bates loam, 3 to 5 percent slopes	С	31.0	6.2%
5	Bates-Coweta complex, 1 to 3 percent slopes	С	1.2	0.2%
6	Bates-Coweta complex, 3 to 5 percent slopes	С	7.6	1.5%
12	Dennis silt loam, 1 to 3 percent slopes	C/D	29.4	5.9%
13	Dennis silt loam, 3 to 5 percent slopes	C/D	16.2	3.2%
14	Dennis silt loam, 3 to 5 percent slopes, eroded	C/D	3.4	0.7%
17	Dennis-Verdigris complex, 0 to 12 percent slopes	C/D	35.4	7.1%
43	Oil waste land		6.9	1.4%
70	Taloka silt loam, 0 to 1 percent slopes	D	238.3	47.7%
71	Taloka silt loam, 1 to 3 percent slopes	D	107.3	21.5%
W	Water		13.3	2.7%
Totals for Area of Intere	st		499.7	100.0%

Description

Hydrologic soil groups are based on estimates of runoff potential. Soils are assigned to one of four groups according to the rate of water infiltration when the soils are not protected by vegetation, are thoroughly wet, and receive precipitation from long-duration storms.

The soils in the United States are assigned to four groups (A, B, C, and D) and three dual classes (A/D, B/D, and C/D). The groups are defined as follows:

Group A. Soils having a high infiltration rate (low runoff potential) when thoroughly wet. These consist mainly of deep, well drained to excessively drained sands or gravelly sands. These soils have a high rate of water transmission.

Group B. Soils having a moderate infiltration rate when thoroughly wet. These consist chiefly of moderately deep or deep, moderately well drained or well drained soils that have moderately fine texture to moderately coarse texture. These soils have a moderate rate of water transmission.

Group C. Soils having a slow infiltration rate when thoroughly wet. These consist chiefly of soils having a layer that impedes the downward movement of water or soils of moderately fine texture or fine texture. These soils have a slow rate of water transmission.

Group D. Soils having a very slow infiltration rate (high runoff potential) when thoroughly wet. These consist chiefly of clays that have a high shrink-swell potential, soils that have a high water table, soils that have a claypan or clay layer at or near the surface, and soils that are shallow over nearly impervious material. These soils have a very slow rate of water transmission.

If a soil is assigned to a dual hydrologic group (A/D, B/D, or C/D), the first letter is for drained areas and the second is for undrained areas. Only the soils that in their natural condition are in group D are assigned to dual classes.

Rating Options

Aggregation Method: Dominant Condition Component Percent Cutoff: None Specified Tie-break Rule: Higher

JSDA

HYDROGRAPH DEVELOPMENT INFORMATION

HYDROGRAPH DEVELOPMENT INFORMATION

Landfill Areas

Direct runoff methods (i.e., kinematic wave) were used for landfill areas that drain to the perimeter drainage system. The kinematic wave method was used to model the 4(H):1(V) sideslope areas before the flow is intercepted by perimeter channels. The kinematic wave method is a physically based method using slope, surface roughness, catchment lengths, and areas. This method does not consider attenuation for flood wave; as a consequence, this method provides for a conservative analysis. To demonstrate how the HEC-1 model was developed for landfill areas, typical parameters used for the kinematic wave method are (1) listed below for drainage area LF1 and (2) shown graphically on Figure H-2-17.

Kinematic Wave parameters for overland flow:

Slope: 0.25 ft/ft landfill sideslopes

- N: 0.3 Manning's coefficient for overland flow
- L: Represents a typical distance between swales for overland flow

Percentage of drainage area represented by this element is 100 percent.

Kinematic Wave routing data for swales:

- Swale length (ft): Longest swale length for the drainage area was used.
- Swale bottom slope (ft/ft): 0.01
- Channel roughness coefficient: 0.03
- Channel type: A trapezoidal channel was used with no bottom width to simulate a triangular channel.

Drainage Areas

The onsite drainage areas used for this analysis are shown on Figure H-2-17. The routing scheme is shown in the HEC-1 output file.

MUSKOGEE COMMUNITY RDF 0086-364-11-19 APPENDIX H UNIT HYDROGRAPH DATA

Snyder's Hydrograph Coefficients (Espey's 10 Minute Method)

Area No.	Area	Max. Flow	S	$I(\%)^{1}$	Manning	Φ^2	T _r ³	T_{lag}^{4}	T _{lag}	Area ⁵	q_p^{6}	C_p^7
	(acres)	Length (L)	(ft/ft)		"n"1		(min)	(min)	(hr)	(sq mi)	(cfs/sq mi)	Ŷ
		(ft)										
01	20.60	2,240	0.010	2	0.04	0.86	40.3	37.8	0.63	0.0322	695.2	0.68
O2	72.56	2,845	0.008	2	0.04	0.86	45.0	42.5	0.71	0.1134	587.2	0.65
03	9.01	900	0.009	2	0.04	0.86	33.5	31.0	0.52	0.0141	874.4	0.71
04	7.91	412	0.030	2	0.04	0.86	20.7	18.2	0.30	0.0124	1470.2	0.70
S1	0.69	550	0.004	2	0.04	0.86	36.7	34.2	0.57	0.0011	880.6	0.78
S2	51.48	2,816	0.006	2	0.04	0.86	48.2	45.7	0.76	0.0804	552.6	0.66
S3	0.62	946	0.025	2	0.04	0.86	26.3	23.8	0.40	0.0010	1263.5	0.78
S4	3.02	895	0.012	2	0.04	0.86	31.2	28.7	0.48	0.0047	987.9	0.74
S5	3.79	153	0.030	2	0.04	0.86	16.5	14.0	0.23	0.0059	1932.1	0.70
S6	6.97	577	0.006	2	0.04	0.86	33.5	31.0	0.52	0.0109	884.3	0.71
S 7	20.69	2,200	0.003	2	0.04	0.86	54.2	51.7	0.86	0.0323	506.0	0.68
S8	7.57	145	0.040	2	0.04	0.86	15.2	12.7	0.21	0.0118	2056.7	0.68
S9	4.77	1,014	0.006	2	0.04	0.86	38.1	35.6	0.59	0.0075	781.5	0.72

¹ Non-landfill areas were assumed to be 2 percent impervious (maximum) to provide a conservative convenyance coefficient.

² Conveyance efficiency coefficient from Dodson & Associates Inc., ProHec-1 Program Documentation, 1995, pages 6-19 and 6-20.

³ $T_r = 3.1(L^{0.23})(S^{-0.25})(I^{-0.18})(\Phi^{1.57})$

 4 T_{lag} = T_r - 5/2

⁵ From area summary sheet

 6 q_p = 31600(A^{-0.04})(T_r^{-1.07})

7
 C_p = 49.375(A^{-0.04})(T_r^{-1.07})(T_{lag})

 T_r = surface runoff to unit hydrograph peak (min)

L = distance along main channel from study point to watershed boundary (ft)

S = main channel slope (ft/ft)

I = impervious cover within the watershed (%)

 $T_{lag} = watershed lag time (min)$ $q_p = unit hydrograph peak discharge (cfs/sq mi)$ $C_p = Snyder's peaking coefficient$

ESPEY 10-MINUTE SAMPLE CALCULATION

Snyder Unit Hydrograph uses lag time (T_{lag}) and peaking coefficient accounting for flood wave and watershed storage conditions.

Upland Area 1 "O2" is used in this example.

Estimated Watershed specific parameters

A =	72.56	acres	watershed area
L=	2845	feet	maximun flow length with this watershed
S =	0.008	feet/feet	watershed slope
$\mathbf{I} =$	2	percent (%)	watershed imperviousness
n =	0.04		Manning's coefficient

Calculate Tr: time beginning of surface runoff to the unit hydrograph peak in minutes

 $T_r = 3.1(L^{0.23})(S^{-0.25})(\Gamma^{-0.18})(\Phi^{1.57})$ Estimate : conveyance efficiency coefficient

 Φ = for 0.86 percent impervious cover and n = 0.04 Φ= 0.86

 $T_r = 3.1(2845^{0.23})(0.008^{-0.25})(2^{-0.18})(0.86^{1.57})$ $T_r = 45.0$ min

Calculate T_{lag}: watershed lag time

 $T_{lag}=\ Tr - (\Delta t/2)$ Δt is calculation interval, and 5 minutes is used $T_{lag} = 42.5$ in the HEC - 1 modeling in this project minutes $T_{lag} = 0.71$ hours A= A/640 A= 0.1134

<u>Calculate q_p </u>: peak discharge of unit hydrograph per unit area (cfs/sq. mi).

square miles

$$q_{p}= 31600(A^{-0.04})(T_{r}^{-1.07})$$

$$q_{p}= 31600(0.1098^{-0.04})(45.0^{-1.07})$$

$$q_{p}= 587.2 cfs/sq. mi$$

Calculate Peaking coefficient Cp:

$$C_{p}= 49.375(A^{-0.04})(T_{r}^{-1.07})(T_{lag})$$

$$C_{p}= 49.375(0.1098^{-0.04})(45.0^{-1.07})(0.71)$$

$$C_{p}= 0.65$$

compute the value of Snyder's peaking coefficient C_p for use in HEC-1 analyses. First, the watershed lag time T_L is determined by subtracting one-half of the computation interval from the time to rise ($T_L = T_r - \Delta t/2$). Then, C_p may be computed by substituting the known values of T_L and q_p into Snyder's equation for peak unit hydrograph flow rate and solving for C_p .

$$C_p = \frac{q_p \times T_L}{640}$$

In another study, Espey [1977] derived the following equation for computing the time from the beginning of surface runoff to the unit hydrograph peak:

$$T_r = 3.10 L^{0.23} S^{-0.25} I^{-0.18} \Phi^{1.57}$$

in which:

 T_r = time from beginning of surface runoff to unit hydrograph peak (minutes)

L = total distance along main channel from study point to watershed boundary (feet)

S = main channel slope between the reference point and a point 0.2L downstream from the upstream watershed boundary (feet per foot)

I = impervious cover within the watershed (percent)

 Φ = description of conveyance efficiency of the watershed drainage system.

The conveyance efficiency coefficient Φ is determined using the relationships illustrated on Figure 6.12.



This equation was derived from records for 41 watersheds in Texas, Tennessee, Mississippi, Pennsylvania, North Carolina, Colorado, Kentucky, and Indiana. The range in the watershed characteristics used to develop the equations for urban areas were:

Area : From 0.0128 square miles to 15.00 square miles

L: From 555 feet to 35,600 feet

6.30

Espey "10-Minute" Method for Estimating Snyder Parameters

6.31

FIGURE 6.12 Determination of Conveyance Efficiency Coefficient Φ

S: From 0.0005 ft. per ft. to 0.0295 ft. per ft.

I : From 2% to 100%

 Φ : From 0.60 to 1.30

Again, note that the time to rise T_r is not the same as the watershed lag time T_p . The difference between the two is that T_r is defined as the time from the beginning of effective rainfall to the peak of the unit hydrograph, while T_L is the time from the centroid of the effective rainfall to the peak of the unit hydrograph. For the purposes of HEC-1 analyses, however, T_L may be determined simply by subtracting one-half the computation time interval from the computed value of T_r (T_R - $\Delta t/2$).

The relationship developed by Espey to compute the peak flow rate of the unit hydrograph is as follows:

6.32

$$Q_{\rm m} = 31600 {\rm A}^{0.96} T_{\rm m}^{-1.07}$$

in which:

 Q_{μ} = unit hydrograph peak discharge (cfs)

A = drainage area (square miles)

 T_r = time of rise from beginning of surface runoff to unit hydrograph peak (minutes)

Riverside County Method for Estimating Snyder Parameters Three watershed lag equations have been derived for use in rural areas of Riverside County, California by the Riverside County Flood Control and Water Conservation District [Anonymous, 1963]. These equations differ slightly from those developed at the Tulsa District of the U.S. Army Corps of Engineers in that lag is defined as the time from the beginning of rainfall to the point on the unit hydrograph corresponding to one-half of the total runoff volume.

Each equation is applicable to a different topographic region:

6.33	$T_L = 1.20 \left(\frac{L \times L_{Ca}}{\sqrt{S}} \right)^{0.38}$	(Mountain Areas)
6.34	$T_L = 0.72 \left(\frac{L \times L_{Ca}}{\sqrt{S}}\right)^{0.38}$	(Foothill Areas)
6.35	$T_L = 0.38 \left(\frac{L \times L_{ca}}{\sqrt{S}}\right)^{0.38}$	(Valley Areas)
	in which:	
	T_L = watershed lag in hours	

L = watershed length in miles

 L_{ca} = length to centroid in miles

S = watershed slope in feet per mile.

The sizes of the watersheds studied in developing these equations ranged from 2.3 square miles to 645 square miles.



	0.0171		10.34				0.1134	0.1	
	0.0071		4.52		СНС		0.0141	0.0	
	0.0213		13.62	CHD		CHD			0.0124
IFICATION	MIT MOD	PER	TIER III	INC.	OKLAHOMA,	FOR	PREPARED	MANA	
AINAGE AREAS	MENT DRA	ELOPN	POST-DEVE		REVISIONS				
IY RDF KLAHOMA	E COMMUNII COUNTY, OI	SKOGEI (OGEE	MU: MUSK						
E H-2-17	FIGURE	сом	WWW.WCGRP.						

FFSITE							
A (SQ MILES)							
0.0322							
0.1134							
0.0141							
0.0101							

HEC-1	DRAINAGE AREAS	S – ONSITE				
DRAINAGE AREA	AREA (ACRES)	AREA (SQ MILES)				
LF1	25.64	0.0401				
LF2	13.41	0.0210				
LF3	14.29	0.0223				
LF4	21.29	0.0333				
LF5	23.14	0.0362				
LF6	13.63	0.0213				
S1	0.69	0.0011				
S2	51.48	0.0804				
S3	0.62	0.0010				
S4	3.02	0.0047				
S5	3.79	0.0059				
S6	6.97	0.0109				
S7	20.69	0.0323				
S8	7.57	0.0118				
S9	4.77	0.0075				
P1	2.22	0.0035				
P2	0.60	0.0009				
P3	3.52	0.0055				
P4	0.68	0.0011				
P5	1.96	0.0031				
P6	1.34	0.0021				
P7	5.84	0.0091				
P8	45.30	0.0708				
P9	15.54	0.0243				
СНА	0.61	0.0010				
СНВ	10.94	0.0171				
СНС	4.52	0.0071				
СНД	13.62	0.0213				



<u>LEGEND</u>

EXISTING PERMIT BOUNDARY PROPOSED PERMIT BOUNDARY PERMITTED LIMITS OF WASTE PROPOSED LIMITS OF WASTE STATE PLANE COORDINATE GRID EXISTING CONTOUR BORROW AREA CONTOUR FINAL COVER CONTOUR DRAINAGE SWALE DRAINAGE CHUTE DRAINAGE CHANNEL RIPRAP/GABIONS APPROXIMATE OG&E POWER LINE EASEMENT EXISTING GROUNDWATER MONITORING WELL PROPOSED GROUNDWATER MONITORING WELL EXISTING LANDFILL GAS MONITORING PROBE PROPOSED LANDFILL GAS MONITORING PROBE DRAINAGE AREA BOUNDARY DRAINAGE AREA DESIGNATION

PROPOSED POST-DEVELOPMENT HYDROLOGIC ANALYSIS: HEC-1 OUTPUT

Includes Pages H-2-19 through H-2-67

1**	* * * * * * * * * * * * * * * * * * * *	**	***************************************					
*		*	*		*			
*	FLOOD HYDROGRAPH PACKAGE (HEC-1)	*	*	U.S. ARMY CORPS OF ENGINEERS	*			
*	JUN 1998	*	*	HYDROLOGIC ENGINEERING CENTER	*			
*	VERSION 4.1	*	*	609 SECOND STREET	*			
*		*	*	DAVIS, CALIFORNIA 95616	*			
*	RUN DATE 07MAR23 TIME 17:19:45	*	*	(916) 756-1104	*			
*		*	*		*			
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THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1GS, HEC1DB, AND HEC1KW.

THE DEFINITIONS OF VARIABLES -RTIMP- AND -RTIOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE. THE DEFINITION OF -AMSKK- ON RM-CARD WAS CHANGED WITH REVISIONS DATED 28 SEP 81. THIS IS THE FORTRAN77 VERSION NEW OPTIONS: DAMBREAK OUTFLOW SUBMERCENCE, SINCLE EVENT DAMAGE CALCULATION, DSS.WRITE STAGE FREQUENCY, DSS:READ TIME SERIES AT DESIRED CALCULATION INTERVAL LOSS RATE:GREEN AND AMPT INFILTRATION KINEMATIC WAVE: NEW FINITE DIFFERENCE ALGORITHM

1	HEC-1 INPUT	PAGE 1
LINE	ID1	
	*DIAGRAM	
1	ID MUSKOGEE COMMUNITY RDF	
2	ID TIER I PERMIT MODIFICATION	
3	ID 25-YEAR 24-HOUR STORM EVENT	
45	IT 5 0 2400 720 0 0	
6	IO 3 0 0	
	*	
7	KK P6	
8	KM POND 6	
10	PH 071 154 32 388 43 528 625 725	
11	BA 0.0021	
12	LS 0 100	
13	UD 0 *	
1.4	KK B/D6	
15	KM ROUTE THROUGH 30 INCH CMP CULVERT	
16	KO 0 0 0 7 21	
17	RS 1 ELEV 635.5	
18	SA U U.UI U.U7 U.19 U.32 U.46 SE 635 5 636 637 638 639 640	
20	SS 639 20 2.6 1.5	
21	SL 636.15 4.9087 0.8 0.5	
	*	
22	KK LF3	
23	KM SUBAREA LF3	
24	KO 0 0 0 7 21	
25	BA 0.0223	
27	UK 120 0.25 0.3 100	
28	RD 1316 0.01 0.03 TRAP 0 2 NO	
	*	
29	KK Ol	
30	KM OFFSITE AREA 1	
31		
33	LS 0 79	
34	US 0.63 0.68	
	*	
35	КК Р5	
36	KM POND 5	
38	BA 0.0031	
39	LS 0 100	
40	UD 0	
1	* HEC-1 INPUT	PAGE 2
TIME		
DINE	10	
41	кк с/4	
42	KO 0 0 0 7 21	
43	HC 4	
	x	
44	KK R/P5	
45	KM ROUTE THROUGH 30 INCH RCP	
46	KO U O O 7 21	
4 /	NO I DEV UJ2./	

48 49 50 51	SA SE SS SL *	0 632.7 639.9 633.75	0.31 633 20 4.908	0.39 634 2.6 0.8	0.47 635 1.5 0.5	0.55 636	0.64 637	0.72 638	0.82 639	0.94 640		
52 53 54 55 56 57	KK KO BA LS UD *	P4 POND 4 0.0011 0 0	0 100	0	7	21						
58 59 60	KK KO HC *	C/2 0 2	0	0	7	21						
61 62 63 64 65 66 67 68	KK KO RS SA SE SL *	R/P4 ROUTE T 0 1 0 627.7 630 628.45	HROUGH 2 0 ELEV 0.17 628 20 9.8174	-30 INCH 0 627.7 0.19 629 2.6 0.8	CMPS 7 0.21 630 1.5 0.5	21 0.36 631						
69 70 71 72 73 74	KK KM BA LS UD *	P3 POND 3 0.0055 0 0	0	0	7	21						
75 76 77	KK KO HC *	C/2 0 2	0	0	7	21						2
1 LINE	ID.	1.	2.	3.	HEC-1	INPUT	6	7	8	910	PAGE	3
78 79 80 81 82 83 84 85	KK KM KO RS SA SE SS SL	P3/DP1 ROUTE T 0 1 0 625.5 628 626.55	HROUGH 3 0 ELEV 0.06 625.6 700 4.9087	0 INCH CI 0 625.5 0.32 626 2.6 0.8	MP OFFSIT 7 0.41 627 1.5 0.5	TE 21 0.5 628	0.59 629					
86 87 88 90 91	KK KM KO BA LS UD *	P2 POND 2 0.0009 0 0	0 100	0	7	21						
92 93 94 95 96 97 98 99	KK KO RS SA SE SS SL	R/P2 ROUTE T 0 1 0 638.4 639.9 639.25	HROUGH 3 0 ELEV 0.01 640 20 4.9087	0 INCH CM 0 638.4 0.07 641 2.6 0.8	M₽ 7 1.5 0.5	21						
100 101 102 103 104 105 106	KK KM KO BA LS UK RD *	R/CHA CHANNEL 0.001 0 59 582	CHA 0 84 0.051 0.011	0 0.35 0.04	7	21 TRAP	0	2.5	YES			
107 108 109 110 111 112 113	KK KM KO BA LS UK RD *	LF1 SUBAREA 0 0.0401 0 88 1788	LF1 0 88 0.25 0.01	0 0.3 0.03	7	21 TRAP	0	2	NO			
114 115 116 117 118 119	KK KM BA LS UD *	P1 POND 1 0.0035 0 0	0 100	0	7	21						4
LINE	ID.	1.	2.		нес-1		6	7	8	910	PAGE	4

H-2-20

	120 121 122	KK KO HC	C/3 0 3	0	0	7	21						
	123 124 125 126 127 128 129 130	KK KM RS SA SE SS SL *	R/P1 ROUTE T 0 1 0 626 631 623.5	HROUGH 36 0 ELEV 0 628.6 15 7.07	5 INCH RCH 626 0.493 629 2.6 0.8	7 0.567 630 1.5 0.5	21 0.64 631	0.712 632					
	131 132 133 134 135 136 137	KK KM KO BA LS UK RD *	CHB SUBAREA 0 0.0171 0 184 2088	CHB 0 88 0.224 0.004	0 0.3 0.03	7	21 TRAP	10	2	NO			
	138 139 140 141 142 143	KK KO BA LS UK	LF2 SUBAREA 0 0.021 0 120	LF2 0 88 0.25	0.3	7	21						
	144 145 146 147	RD * KK KO HC *	1920 C/3 0 3	0.01	0.03	7	TRAP 21	0	2	NO			
	148 149 150 151 152 153 154	KK KM KO BA LS UK RD	CHC SUBAREA 0 0.0071 0 140 645	CHC 0 88 0.25 0.005	0 0.3 0.03	7	21 TRAP	10	2	YES			
	155 156 157 158 159	KK KM KO BA LS	S1 AREA S1 0.0011 0	0 79	0	7	21						
	160	UD *	0.57	0.78									
1	160	UD *	0.57	0.78	2	HEC-1	INPUT	c	-	0		PAGE	5
1	160 LINE	UD * ID.	0.57	0.78		HEC-1	INPUT	6	7	8	91	PAGE	5
1	160 LINE 161 162 163 164 165 166	UD * ID. KK KM KO BA LS UD *	0.57 1. P7 POND 7 0.0089 0 0	0.78	3	HEC-1	INPUT 5 21	6	7	8	91	PAGE	5
1	160 LINE 161 162 163 164 165 166 167 168 169	UD * ID. KK KM KO BA LS UD * KK KK KO HCC *	0.57 1. P7 POND 7 0.0089 0 0 C/3 0 3	0.78 2 0 100 0	3 0 0	HEC-1	INPUT 5 21 21	6	7	8	91	PAGE	5
1	160 LINE 161 162 163 164 165 166 166 169 170 171 172 173 174 175 176	UD * ID. KK KM KM BA LS UD * KK KK KK KK KK KK KK KK KK KK SA SE SS *	0.57 P7 P0ND 7 0.0089 0 0 0 0 0 0 0 0 0 0 0 0 0	0.78 2 0 100 0 HROUGH 36 0 ELEV 2.26 606 8189	0 0 5 INCH RCI 0 605 2.51 608 2.6	HEC-1 7 7 7 3.02 612 1.5	INPUT 21 21 21 21 3.56 616	3.83 618	4.39 622	8	91	PAGE	5
1	160 LINE 161 162 163 164 165 166 167 168 169 170 171 172 173 174 175 176 177 178 179 180 181 182	UD * ID. KK KK KM KO BA LS UD * KK KK KM KO SA SE SS * KK KK KM KO BA LS US US S*	0.57 1. P7 POND 7 0 0.0089 0 C/3 0 C/3 0 C/3 0 C/3 0 0 0 0 0 0 0 0 0 0 0 0 0	0.78 	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	HEC-1 7 7 7 3.02 612 1.5 7	INPUT 21 21 3.56 616 21	3.83 618	4.39 622	8	91	PAGE	5
1	160 LINE 161 162 163 164 165 166 167 168 169 170 171 172 173 174 175 176 177 178 179 180 181 182 183 184 185 186 187 188	UD * ID. KK KM KM KK KM KC EA SS SS SS KK KK KM KO EA SS SS SS K KK KK KM KO EA SS SS SS SS K KK KK KM KM KM KK KK KK KM KM KM KK KK	0.57 P7 P0ND 7 0.0089 0 C/3 0 C/3 0 C/3 0 0 C/3 0 0 0 0 0 0 0 0 0 0 0 0 0	0.78 0.78 0 100 0 ELEV 2.26 606 189 AREA 2 0 79 0.65 0 79 0.66	0 0 5 INCH RCI 0 605 2.51 608 2.6 0 0	HEC-1 7 7 7 3.02 612 1.5 7	INPUT 21 21 3.56 616 21 21	3.83 618	4.39 622	8	91	PAGE 10	5
1	160 LINE 161 162 163 164 165 166 167 168 169 170 171 172 173 174 175 176 177 178 179 180 181 182 183 184 185 186 187 188 189 190 191	UD * ID. KK KM KM KK KK KM KC F S S S S S S S S S S S S S S S KK KK KM KO BA S S S S S S S S S S S S S S S S S S	0.57 P7 P0ND 7 0.0089 0 C/3 0 C/3 0 C/3 0 0 C/3 0 0 0 0 0 0 0 0 0 0 0 0 0	0.78 0 100 0 100 0 ELEV 2.26 606 189 AREA 2 0 79 0.65 0 79 0.66 0	0 0 5 INCH RCI 0 605 2.51 608 2.6 0 0	HEC-1 7 7 7 3.02 612 1.5 7 7 7	INPUT 5 21 21 3.56 616 21 21 21 21 21	3.83 618	4.39 622	8	91	PAGE 10	5

194	KO	0	0	0	7	21					
195	RS	1	ELEV 0 12	617.9	3 32	5 84					
190	SE	617.9	620	622	624	626					
198	SS	622	746	2.6	1.5						
199	SL *	618.3	0.785	0.8	0.5						
1					HEC-1	INPUT					PAGE 6
LINE	TD	1	2	3	4	5	6	7	8	9 10	
	10.										
200	vv	TEG									
200	KM	SUBAREA	LF6								
202	KO	0	0	0	7	21					
203	BA	0.0213	0.0								
204 205	UK	120	0.25	0.3	100						
206	RD	1554	0.01	0.03		TRAP	0	2	NO		
	*										
207	KK	04									
208	KM	AREA 04	0	0	-	0.1					
209	RA	0 0124	U	U	/	21					
211	LS	0.0121	79								
212	US	0.3	0.7								
	*										
213	KK	C/2									
214	KO	0	0	0	7	21					
213	*	2									
216	KK	CHD	CHD								
218	KO	0	0	0	7	21					
219	BA	0.0213									
220	LS	139	88	03	100						
222	RD	2806	0.01	0.03	100	TRAP	10	3	YES		
	*										
223	кк	LF4									
224	KM	SUBAREA	LF4								
225	KO	0	0	0	7	21					
226	LS	0.0333	88								
228	UK	120	0.25	0.3	100						
229	RD	1258	0.01	0.03		TRAP	0	2	NO		
	î										
230	KK	C/2			_						
231	KO	0	0	0	7	21					
202	*	-									
222		0.2									
233	KM	CO ATTENTS	AREA 3								
235	KO	0	0	0	7	21					
236	BA	0.0141	70								
237	US	0.52	0.71								
	*										
l					HEC-1	INPUT					PAGE /
LINE	ID.	1.	2.	3.	4	5	6	7	8		
239	KK	S9									
240	KM	AREA S9	0	0	7	21					
241 242	BA	0.0075	0	U	/	21					
243	LS	0	79								
244	US *	0.59	0.72								
245	KK	P9									
246	KM	POND 9	0	0	7	21					
248	BA	0.0243	0	0	,	21					
249	LS	0	100								
250	UD *	0									
251	KK	S6									
252	KM	AREA S6	0	0	7	21					
254	BA	0.0109	0	0	,						
255	LS	0	79								
256	*	0.52	0.71								
257	KK	C/4	0	0	7	21					
259	HC	4	0	0	'	<u> </u>					
	*										
260	KK	R/P9									
261	KM	ROUTE T	HROUGH 1	x48 INCH	CMP						
262	KO	0	0	0 610 7	7	21					
263	SA	0	0.055	1.07	3.93	4.21					
265	SE	610.7	612	620	630	630.8					

H-2-22

	266 267	SS SL *	630.3 611.3	10 12.567	2.6 0.8	1.5 0.5							
	268	KK	LF5										
	269	KM	SUBAREA	LF5									
	270	KO BA	0 0362	0	0	7	21						
	272	LS	0.0002	88									
	273	UK	259	0.25	0.3	100	00 A D	0	2	NO			
	2/4	RD *	1500	0.01	0.03		TRAP	U	2	NO			
	275	KK	P8										
	270	KO	POND 8	0	0	7	21						
	278	BA	0.0708										
	279	LS	0	100									
	200	*	0										
1						HEC-1	INPUT					PAGE	8
	LINE	ID.	1.	2	3.	4.	5.	6		8	910		
	0.01												
	281	KM	AREA S8										
	283	KO	0	0	0	7	21						
	284	BA	0.0118	75									
	286	US	0.21	0.68									
		*											
	287	KK	C/4			-							
	288	KO HC	4	U	U	/	21						
		*											
	290	KK	R/P8										
	291	KM	ROUTE T	HROUGH P8	SPILLWA	ΑY							
	292	KO	0	0 FLEV	0 561	7	21						
	294	SA	Ū.	8.61	12.63	16.82	21.15	29.36					
	295	SE	561	570	580	590	600	618					
	296	*	017	200	2.0	1.5							
	297	KK	S3/DP6										
	298	KM	AREA S3	0	0	7	0.1						
	300	BA	0.001	0	U	/	21						
	301	LS	0	75									
	302	US *	0.4	0.78									
			/										
	303	KK	AREA S7										
	305	KO	0	0	0	7	21						
	306	BA	0.0323	75									
	307	US	0.86	0.68									
		*											
	309	KK	S5/DP4										
	310	KM	AREA S5	0	0	7	0.1						
	312	BA	0.0059	U	U	/	21						
	313	LS	0	75									
	314	US *	0.23	0.7									
	315 316	KM	S4/DP5 AREA S4										
	317	KO	0	0	0	7	21						
	318	BA	0.0047	75									
	320	US	0.48	0.74									
	201	*											
1	521	22											
INPUT	SCHI	EMATIC DI	AGRAM OF	STREAM N	IETWORK								
LINE	(V) ROUT	ING	(>) DIVERS	ION OR I	PUMP FLO	W						
NÖ.	(.) CONNI	SCTOR	(<	-) RETURN	OF DIVE	SKTED OR	FOMPED	F TOM					
/	VР												
14	V R/P6												
22	•	LF	3										
	•		•										
29				01									
	•		•	•									
35	•			•	PS	5							
	•		•	•									
41	C/4												
	V												
	•												

H-2-23



H-2-24

P9 .

051					0.0	
231	· · ·	· · ·	•	•		
257		. C/4 . V	· · · · · · · · · · · · · · · · · · ·	· · · · · · · · · · · · · · · · · · ·		
260		. V R/P9				
	· · ·	· · ·				
268	· ·	· ·	LF5			
275	· ·	· ·		P8		
	· · ·	· · ·		•		
281	· · ·	· · ·		•	S8	
287	· · ·	. c/4				
	· · ·	. V . V				
290	· · ·	. R/P8				
297	· · ·	· · ·	S3/DP6			
	· · ·	· · ·	•			
303	· · ·	· · ·	•	S7/DP3		
309	· · ·	· · ·	•		S5/DP4	
	· · ·	· · ·	•		•	
315					•	S4/DP5
(***) RUI 1*******	NOFF ALSO COMPUTED AT THIS I	JOCATION				*****
* * FLOOI	D HYDROGRAPH PACKAGE (HEC-1	.) *				* U.S. ARMY CORPS OF ENGINEERS *
*	JUN 1998 VERSION 4.1	*				* HYDROLOGIC ENGINEERING CENTER * * 609 SECOND STREET *
* * RUN DA	ATE 07MAR23 TIME 17:19:4	*				* DAVIS, CALIFORNIA 95616 * * (916) 756-1104 *
* ******	* * * * * * * * * * * * * * * * * * * *	*				* ************************************
	MUSKOGEE C TIER I PEF	COMMUNITY RDF RMIT MODIFICATION				
	25-YEAR 24 P:\SOLIDWA	-HOUR STORM EVENT ASTE\MUSKOGEE\TIER 1 MO	D\APPENDIX	B\HEC1\PROPO	SED.IH1	
6 IO	OUTPUT CONTROL VARI	ABLES				
	IPRNT IPLOT	3 PRINT CONTROL 0 PLOT CONTROL				
	QSCAL	U. HYDROGRAPH PLOT	SCALE			
1T	NMIN	5 MINUTES IN COMPU	TATION INTE	RVAL		
	IDATE I ITIME	0 STARTING DATE 0000 STARTING TIME				
	NQ NDDATE 4	0 ENDING DATE	RAPH ORDINA	TES		
	NDTIME ICENT	1155 ENDING TIME 19 CENTURY MARK				
	COMPUTATION INTEF TOTAL TIME F	RVAL .08 HOURS BASE 59.92 HOURS				
	ENGLISH UNITS	000355 VT150				
	PRECIPITATION DEPTH	INCHES				
	FLOW	CUBIC FEET PER SECOND)			
	SURFACE AREA	ACRES				
	IBRESKALUKE	DEGREES FARKENREIT				
*** *** :	*** *** *** *** *** ***	*** *** *** *** *** **	* *** *** *	** *** *** *	** *** ***	*** *** *** *** *** *** *** ***

7 KK	* P6 *					
	* ***********************************					
	POND 6					

			POND 6	5	
1	9	ко	OUTPUT CONTROL	VARIABLES	
			IPRNT	3	PRINT CONTROL
			IPLOT	0	PLOT CONTROL
			QSCAL	0.	HYDROGRAPH PLOT SCALE
			IPNCH	7	PUNCH COMPUTED HYDROGRAPH
			IOUT	21	SAVE HYDROGRAPH ON THIS UNIT
			ISAV1	1	FIRST ORDINATE PUNCHED OR SAVED
			ISAV2	720	LAST ORDINATE PUNCHED OR SAVED
			TIMINT	.083	TIME INTERVAL IN HOURS

H-2-25

	SUBBASI	N RUNOFF DA	ATA						
11 BA	SUBBA	SIN CHARAC TAREA	TERISTICS .00 SU	IBBASIN AREA					
	PRECI	PITATION DA	ATA						
10 PH	5-MIN .71	HYDRO-35 1 15-MIN (1.54	DEP1 50-MIN 2- 3.20 3.	HS FOR 0- HR 3-HR 88 4.30	PERCENT HY . TP-40 . 6-HR 5.28	12-HR 6.25	CAL STOP 24-HR 7.25	RM 2-DAY 4-DA .00 .0	TP-49 Y 7-DAY 10-DAY 0 .00 .00
				STOR	M AREA =	.00			
12 LS	SCS I	LOSS RATE STRTL CRVNBR RTIMP	.00 IN 100.00 CU .00 PE	IITIAL ABSTR. IRVE NUMBER RCENT IMPER	ACTION VIOUS AREA	A			
13 UD	SCS I	IMENSIONLES TLAG	SS UNITGRAPH	l \G					
						* * *			
	12.	3.	1.	5	UNIT HY END-OF-PH 0.	YDROGRAPH ERIOD ORI	H DINATES		
* * *		* * *	***	*	* *	**;	*		
		HYDROGR/	APH AT STATI	ON P6					
TOTAL RA	AINFALL =	7.25, TO	TAL LOSS =	.00, TOT.	AL EXCESS	= 7.2	25		
PEAK FLOW	TIME		6-HR	MAXIMUM AV 24-HR	ERAGE FLOW 72-HI	W R 59.	.92-HR		
+ (CFS)	(HR)	(CES)							
+ 10.	12.08	(INCHES) (AC-FT)	1. 5.278 1.	0. 7.247 1.	0 7.250 1	D	0. 7.250 1.		

CUMULATIVE AREA = .00 SQ MI

			* * * * * * * * * * *	* * *		
			*	*		
14	KK		* R/P6	*		
			*	*		
			* * * * * * * * * *	* * *		
				ROUTE	THROUGH 3) INCH CMP CULVERT
16	KO)	OUTPUT	CONTROL	VARIABLES	
				IPRNT	3	PRINT CONTROL
				IPLOT	0	PLOT CONTROL
				QSCAL	0.	HYDROGRAPH PLOT SCALE
				IPNCH	7	PUNCH COMPUTED HYDROGRAPH
				IOUT	21	SAVE HYDROGRAPH ON THIS UNIT
				ISAV1	1	FIRST ORDINATE PUNCHED OR SAVED
				ISAV2	720	LAST ORDINATE PUNCHED OR SAVED
			Т	IMINT	.083	TIME INTERVAL IN HOURS

HYDROGRAPH ROUTING DATA

17	RS	STORAGE ROUTING NSTPS ITYP RSVRIC X	; ELEV 635.50 .00	NUMBER OF TYPE OF I INITIAL C WORKING R	SUBREACH NITIAL CONDITION AND D COE	ES NDITION FFICIENT			
18	SA	AREA	.0	.0	.1	.2	.3	.5	
19	SE	ELEVATION	635.50	636.00	637.00	638.00	639.00	640.00	
21	SL	LOW-LEVEL OUTLE ELEVL CAREA COQL EXPL	CT 636.15 4.91 .80 .50	ELEVATION CROSS-SEC COEFFICIE EXPONENT	I AT CENTEL CTIONAL ARI CNT OF HEAD	R OF OUTLE: EA	г		
20	SS	SPILLWAY CREL SPWID COQW EXPW	639.00 20.00 2.60 1.50	SPILLWAY SPILLWAY WEIR COEF EXPONENT	CREST ELE WIDTH FICIENT OF HEAD	VATION			
						* * *			
				CC	MPUTED ST	ORAGE-ELEVA	ATION DATA		

STORAGE	.00	.00	.04	.16	.41	.80				
ELEVATION	635.50	636.00	637.00	638.00	639.00	640.00				
			COM	PUTED OUTF	LOW-ELEVAT	ION DATA				
OUTFLOW ELEVATION	.00	.00	34.04 637.32	35.89 637.45	37.94 637.60	40.25 637.78	42.85 638.00	45.81 638.27	49.22 638.59	53.17 639.00

H-2-26

 OUTFLOW
 54.03
 55.32
 57.46
 60.66
 65.14
 71.09
 78.72
 88.25
 99.87
 113.80

 ELEVATION
 639.04
 639.09
 639.15
 639.22
 639.31
 639.42
 639.54
 639.68
 639.83
 640.00

COMPUTED STORAGE-OUTFLOW-ELEVATION DATA

STORAGE	.00	.00	.00	.04	.06	.08	.10	.12	.16	.22
OUTFLOW	.00	.00	.00	29.04	34.04	35.89	37.94	40.25	42.84	45.81
ELEVATION	635.50	636.00	636.15	637.00	637.32	637.45	637.60	637.78	638.00	638.27
STORAGE	.30	.41	.43	.44	.46	.49	.52	.56	.61	.66
OUTFLOW	49.22	53.17	54.03	55.32	57.46	60.66	65.14	71.09	78.72	88.25
ELEVATION	638.59	639.00	639.04	639.09	639.15	639.22	639.31	639.42	639.54	639.68
STORAGE	.73	.80								

OUTFLOW 99.87 113.80 ELEVATION 639.83 640.00

*** WARNING *** MODIFIED PULS ROUTING MAY BE NUMERICALLY UNSTABLE FOR OUTFLOWS BETWEEN 0. TO 29. THE ROUTED HYDROGRAPH SHOULD BE EXAMINED FOR OSCILLATIONS OR OUTFLOWS GREATER THAN PEAK INFLOWS. THIS CAN BE CORRECTED BY DECREASING THE TIME INTERVAL OR INCREASING STORAGE (USE A LONGER REACH.)

	* * *		***	***	* * *	* * *		
			HYDROGRAI	PH AT STAT	ION R/P6			
	PEAK FLOW	TIME			MAXIMUM AVEF	RAGE FLOW		
				6-HR	24-HR	72-HR	59.92-HR	
+	(CFS)	(HR)						
			(CFS)					
+	10.	12.08		1.	0.	0.	0.	
			(INCHES)	5.278	7.218	7.218	7.218	
			(AC-FT)	1.	1.	1.	1.	
P	EAK STORAGE	TIME			MAXIMUM AVERA	GE STORAGE		
				6-HR	24-HR	72-HR	59.92-HR	
+	(AC-FT)	(HR)						
	0.	12.08		Ο.	0.	0.	0.	
	PEAK STAGE	TIME			MAXIMUM AVEF	AGE STAGE		
				6-HR	24-HR	72-HR	59.92-HR	
+	(FEET)	(HR)						
	636.44	12.08		636.19	636.16	636.16	636.16	
			CUMULATIVE	E AREA =	.00 SQ MI			

*** ***

		* *
22	KK	* LF3 *
		* **
		SUBAREA LF3
24	KO	OUTPUT CONTROL VARIABLES IPRNT 3 PRINT CONTROL IPLOT 0 PLOT CONTROL QSCAL 0. HYDROGRAPH PLOT SCALE IPNCH 7 PUNCH COMPUTED HYDROGRAPH IOUT 21 SAVE HYDROGRAPH ON THIS UNIT ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED ISAV2 720 LAST ORDINATE PUNCHED OR SAVED TIMINT .083 TIME INTERVAL IN HOURS
		SUBBASIN RUNOFF DATA
25	BA	SUBBASIN CHARACTERISTICS TAREA .02 SUBBASIN AREA
		PRECIPITATION DATA
10	РН	DEPTHS FOR 0-PERCENT HYPOTHETICAL STORM HYDRO-35 TP-40 5-MIN 15-MIN 60-MIN 2-HR 7-1 1.54 3.88 4.30 5.28 6.25 7.25 .00 .00 .00
		STORM AREA = .02
26	LS	SCS LOSS RATE STRTL .27 INITIAL ABSTRACTION CRVNBR 88.00 CURVE NUMBER RTIMP .00 PERCENT IMPERVIOUS AREA
27	UK	KINEMATIC WAVE OVERLAND-FLOW ELEMENT NO. 1 L 120. OVERLAND FLOW LENGTH S .2500 SLOPE N .000 DEVICENTED CONTRACTORY
28	RD	PA 100.0 PERCENT OF SUBASIN DXMIN 5 MINIMUM NUMBER OF DX INTERVALS MUSKINGUM-CUNGE MAIN CHANNEL
		L 1316. CHANNEL LENGTH S .0100 SLOPE N .030 CHANNEL ROUGHNESS COEFFICIENT CA .02 CONTRIBUTING AREA SHAPE TRAP CHANNEL SHAPE

WD	.00	BOTTOM	WIDTH OR DI	AMETER					
Z	2.00	SIDE SL	OPE						
RUPSTQ	NO	ROUTE U	PSTREAM HYD	ROGRAPH					
				* * *					
	COMP	UTED MUSK	INGUM-CUNGE	PARAMETER	RS				
		COMPUT	ATION TIME	STEP					
ELEMENT	ALPHA	М	DT	DX	PEAK	TIME TO	VOLUME	MAXIMUM	
						PEAK		CELERITY	
			(MIN)	(FT)	(CFS)	(MIN)	(IN)	(FPS)	
PLANE1	2.48	1.67	.76	24.00	111.28	724.75	5.84	.67	
MATN	2 31	1 33	3 22	658 00	108 88	724 80	5 64	6 81	

CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .6941E+01 OUTFLOW= .6706E+01 BASIN STORAGE= .5516E-03 PERCENT ERROR= 3.4

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

		MAI	EN	2.31	1.33	5.00		106.96	725.00	5.67
	* * *		***	***		* * *		***		
			HYDROGR!	APH AT STA	ATION	LF3				
	TOTAL F	RAINFALL =	7.25, TO:	FAL LOSS =	= 1.41,	TOTAL	EXCESS =	5.84		
	PEAK FLOW	TIME		6-HR	MAXIMU 24-	JM AVERA -HR	.GE FLOW 72-HR	59.92-HR		
+	(CFS)	(HR)	(CES)							
+	107.	12.08	(INCHES) (AC-FT)	11. 4.627 6.	5.6	3. 567 7.	1. 5.667 7.	1. 5.667 7.		
			CUMULATI	/E AREA =	.02 \$	SQ MI				

	* * * * * * * * * * * * *				
	* *				
29 KK	* * *				
	* * * * * * * * * * * * * *				
	OFFSITE AREA 1				
31 KO	OUTPUT CONTROL VARIABLE IPRNT 3 IPLOT 0 QSCAL 0. IPNCH 7 IOUT 21 ISAVI 1 ISAVI 1 ISAV2 720 TIMINT .083	S PRINT CONTROL PLOT CONTROL HYDROGRAPH PLOT SCAL PUNCH COMPUTED HYDRO SAVE HYDROGRAPH ON TI FIRST ORDINATE PUNCH LAST ORDINATE PUNCHE TIME INTERVAL IN HOU	E SRAPH HIS UNIT ED OR SAVED D OR SAVED RS		
	SUBBASIN RUNOFF DATA				
32 BA	SUBBASIN CHARACTERISTIC	S			
	TAREA .03	SUBBASIN AREA			
	PRECIPITATION DATA				
10 PH		DEPTHS FOR 0-PERCENT	HYPOTHETICAL STOP	RM	
	HYDRO-35 5-MIN 15-MIN 60-MIN .71 1.54 3.20	TP-40 2-HR 3-HR 6-HR 3.88 4.30 5.28	12-HR 24-HR 6.25 7.25	TP- 2-DAY 4-DAY .00 .00	-49 7-DAY 10-DAY .00 .00
		STORM AREA :	= .03		
33 LS	SCS LOSS RATE STRTL .53 CRVNBR 79.00 RTIMP .00	INITIAL ABSTRACTION CURVE NUMBER PERCENT IMPERVIOUS A	REA		
34 US	SNYDER UNITGRAPH				
	TP .63 CP .68	LAG PEAKING COEFFICIENT			
	SYNTHETIC ACCUMULATED-A	REA VS. TIME CURVE WIL	L BE USED		
			* * *		
		UNIT HYDR	OGRAPH PARAMETERS		
		CLARK TC= . SNYDER TP= .	74 HR, R= 63 HR, CP=	.49 HR .68	
		UNIT	HYDROGRAPH		
	1. 4. 8	36 END-OF	-PERIOD ORDINATES	23.	22. 19
	16. 14. 12.	10. 8.	7. 6.	5.	4. 3.
	3. 2. 2.	2. 1.	1. 1.	1.	1. 1.
	1. 0. 0.	υ. υ.	υ.		
* * *	*** **	* ***	* * *		

HYDROGRAPH AT STATION 01

H-2-28

TOTAL	RAINFALL =	7.25, TO	TAL LOSS =	= 2.44, TC	TAL EXCESS =	4.81	
PEAK FLOW	TIME			MAXIMUM A	VERAGE FLOW		
+ (CFS)	(HR)		6-HR	24-HR	72-HR	59.92-HR	
+ 48.	12.67	(CFS)	13.	4.	2.	2.	
		(INCHES) (AC-FT)	3.893 7.	4.790 8.	4.790 8.	4.790 8.	
		CUMULATI	VE AREA =	.03 SQ M	п		
	~ ~~~ ~~~ ~~	~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~	~~~ ~~~ ~				~~~ ~~ ~~ ~~ ~~ ~~ ~~ ~~ ~~ ~~ ~~ ~~ ~~
	********* *	****					
35 KK	* P *	5 * *					
	*******	****					
		POND 5					
37 KO	OUTPU	T CONTROL IPRNT	VARIABLES 3	PRINT CONTRO	L		
		IPLOT QSCAL	0.	PLOT CONTROL HYDROGRAPH P	LOT SCALE		
		IPNCH	7	PUNCH COMPUT	ED HYDROGRAPH	IN T T	
		ISAV1	1	FIRST ORDINA	TE PUNCHED OF	SAVED	
		TIMINT	.083	TIME INTERVA	L IN HOURS	SAVED	
	SUBBASI	N RUNOFF D	ATA				
38 BA	SUBBA	SIN CHARAC TAREA	TERISTICS	SUBBASIN ARE	A		
	PRECT	PITATION D	ата				
10 54	11001			FDTUS FOR 0	-DEDCENT UVDC		DM
10 11		HYDRO-35		····· ···· ····	TP-40	······	TP-49
	5-MIN .71	15-MIN 1.54	60-MIN 3.20	2-HR 3-HR 3.88 4.30	5.28 6	-HR 24-HR	2-DAY 4-DAY 7-DAY 10-DAY .00 .00 .00 .00
				STC	RM AREA =	.00	
39 LS	SCS I	OSS RATE					
		STRTL	.00	INITIAL ABST	RACTION		
		RTIMP	.00	PERCENT IMPE	RVIOUS AREA		
40 UD	SCS D	IMENSIONLE	SS UNITGRA	APH			
		TLAG	.00	LAG			
					* * *		
					UNIT HYDR 5 END-OF-PERI	OGRAPH OD ORDINATES	
	18.	5.	1.	0.	0.		
***		***	***		***	***	
		HYDROGR	APH AT STA	ATION P	5		
TOTAL	RAINFALL =	7.25, TO	TAL LOSS =	= .00, TC	TAL EXCESS =	7.25	
PEAK FLOW	TIME			MAXIMUM A	VERAGE FLOW		
+ (CFS)	(HR)		6-HR	24-HR	72-HR	59.92-HR	
+ 15	12 09	(CFS)	2	1	0	0	
- 10.	12.08	(INCHES)	5.278	7.247	7.250	7.250	
		(AC-FT)	1.	1.	1.	1.	
		CUMULATI	VE AREA =	.00 SQ M	II		
*** *** **	* *** *** **	* *** ***	*** *** *;	** *** *** **	* *** *** ***	*** *** ***	*** *** *** *** *** *** *** *** *** *** ***
	********	****					
41 KK	* c/	4 *					
	* ******	* * * * *					
42 KO	OUTPU	T CONTROL	VARIABLES				
	,	IPRNT	3	PRINT CONTRO	L		
		QSCAL	0.	HYDROGRAPH P	LOT SCALE		
		1PNCH IOUT	7 21	PUNCH COMPUT SAVE HYDROGR	ED HYDROGRAPH APH ON THIS U	INIT	
		ISAV1 ISAV2	1	FIRST ORDINA	TE PUNCHED OF	SAVED	
		TIMINT	.083	TIME INTERVA	L IN HOURS	GAVED	
	43 HC	HYDR	OGRAPH COMBI	NATION			
---	-----------	-------	--------------	-----------	-----------	-------------	-------------
			ICOMP	4	NUMBER OF	HYDROGRAPHS	TO COMBINE
							* * *
	* * *		***	* * *		* * *	* * *
			HYDROGRA	PH AT STA	TION	C/4	
	PEAK FLOW	TIME			MAXIMUM	AVERAGE FL	OW
				6-HR	24-H	IR 72-	HR 59.92-HR
+	(CFS)	(HR)					
			(CFS)				
+	147.	12.08		27.	9		3. 3.
			(INCHES)	4.249	5.32	2 5.3	31 5.331
			(AC-FT)	14.	17	. 1	7. 17.
			CUMULATIV	E AREA =	.06 SC	MI	

		* * * * *	******	* *
		*		*
44	KK	*	R/P5	*
		*		*

46 KO

* * ***************** ROUTE THROUGH 30 INCH RCP

	VADIADIEC	
OUIPUI CONIROL	VARIADIES	
IPRNT	3	PRINT CONTROL
IPLOT	0	PLOT CONTROL
QSCAL	0.	HYDROGRAPH PLOT SCALE
IPNCH	7	PUNCH COMPUTED HYDROGRAPH
IOUT	21	SAVE HYDROGRAPH ON THIS UNIT
ISAV1	1	FIRST ORDINATE PUNCHED OR SAVED
ISAV2	720	LAST ORDINATE PUNCHED OR SAVED
TIMINT	.083	TIME INTERVAL IN HOURS

HYDROGRAPH ROUTING DATA

47	RS	STOP	AGE ROUTIN NSTPS ITYP RSVRIC X	UTING S 1 NUMBER OF SUBREACHES P ELEV TYPE OF INITIAL CONDITIO C 632.70 INITIAL CONDITION X .00 WORKING R AND D COEFFICIE									
48	SA		AREA	.0	.3	.4	.5	.6	.6	.7	.8	.9	
49	SE	ELEV	ATION	632.70	633.00	634.00	635.00	636.00	637.00	638.00	639.00	640.00	
51	SL LOW-LEVEL OUTLET ELEVL 633.75 CAREA 4.91 COQL .80 EXPL .50				ELEVATION CROSS-SEC COEFFICIE EXPONENT	LEVATION AT CENTER OF OUTLET ROSS-SECTIONAL AREA COEFFICIENT XYPONENT OF HEAD							
50	SS	SPII	LWAY CREL SPWID COQW EXPW	639.90 20.00 2.60 1.50	SPILLWAY SPILLWAY WEIR COEF EXPONENT	CREST ELEV WIDTH FICIENT OF HEAD	ATION						
							* * *						
					со	MPUTED STO	RAGE-ELEVA	FION DATA					
		STORAGE ELEVATION	.00 632.70	.03 633.00	.38 634.00	.81 635.00	1.32 636.00	1.91 637.00	2.59 638.00	3.36 639.00	4.24 640.00		
					CO	MPUTED OUT	FLOW-ELEVA	FION DATA					
		OUTFLOW ELEVATION	.00 632.70	.00 633.75	34.93 634.98	37.92 635.20	41.48 635.48	45.77 635.86	51.05 636.38	57.71 637.11	66.37 638.19	78.09 639.90	
		OUTFLOW ELEVATION	78.22 639.91	78.31 639.92	78.42 639.92	78.57 639.93	78.75 639.94	78.98 639.95	79.25 639.96	79.57 639.97	79.94 639.99	80.37 640.00	
					COMPUT	ED STORAGE	-OUTFLOW-EI	LEVATION I	ATA				
		STORAGE OUTFLOW ELEVATION	.00 .00 632.70	.03 .00 633.00	.29 .00 633.75	.38 15.74 634.00	.80 34.93 634.98	.81 35.21 635.00	.91 37.92 635.20	1.05 41.48 635.48	1.24 45.77 635.86	1.32 47.23 636.00	
		STORAGE OUTFLOW ELEVATION	1.53 51.05 636.38	1.91 56.77 637.00	1.98 57.71 637.11	2.59 64.92 638.00	2.73 66.37 638.19	3.36 72.15 639.00	4.15 78.09 639.90	4.16 78.22 639.91	4.17 78.42 639.92	4.19 78.75 639.94	
		STORAGE OUTFLOW ELEVATION	4.20 79.25 639.96	4.22 79.57 639.97	4.23 79.94 639.99	4.24 80.37 640.00							
	* * *		* * *	***		* * *	* :	**					
			HYDROG	RAPH AT SI	ATION	R/P5							

PE.	AK FLOW	TIME		MAXIMUM AVER	AGE FLOW	
			6-HR	24-HR	72-HR	59.92-HR
+	(CFS)	(HR)				

+	64.	12.67	(CFS) (INCHES) (AC-FT)	27. 4.249 14.	8. 5.241 17.	3. 5.241 17.	3. 5.241 17.
PEA	K STORAGE	TIME			MAXIMUM AVE	RAGE STORAGE	
				6-HR	24-HR	72-HR	59.92-HR
+ (AC-FT)	(HR)					
	з.	12.67		1.	0.	0.	0.
PE	AK STAGE	TIME			MAXIMUM AV	ERAGE STAGE	
				6-HR	24-HR	72-HR	59.92-HR
+	(FEET)	(HR)					
	637.94	12.67		635.00	634.09	633.81	633.81
			CUMULATIV	e area =	.06 SQ MI		

***** * * P4 * 52 KK *********** POND 4 54 KO OUTPUT CONTROL VARIABLES ARIAELES 3 PRINT CONTROL 0 PLOT CONTROL 0. HYDROGRAPH PLOT SCALE 7 PUNCH COMPUTED HYDROGRAPH 21 SAVE HYDROGRAPH ON THIS UNIT 1 FIRST ORDINATE PUNCHED OR SAVED IPRNT IPLOT QSCAL IPNCH IOUT ISAV1 720 LAST ORDINATE PUNCHED OR SAVED .083 TIME INTERVAL IN HOURS TSAV2 TIMINT SUBBASIN RUNOFF DATA SUBBASIN CHARACTERISTICS TAREA .00 SUBBASIN AREA 55 BA PRECIPITATION DATA DEPTHS FOR 0-PERCENT HYPOTHETICAL STORM 10 PH HYDRO-35 5-MIN 15-MIN 60-MIN .71 1.54 3.20 TP-49 2-DAY 4-DAY 7-DAY 10-DAY .00 .00 .00 .00 STORM AREA = .00 56 LS SCS LOSS RATE .00 INITIAL ABSTRACTION 100.00 CURVE NUMBER .00 PERCENT IMPERVIOUS AREA STRTL CRVNBR RTIMP 57 UD SCS DIMENSIONLESS UNITGRAPH .00 LAG TLAG +++ UNIT HYDROGRAPH 5 END-OF-PERIOD ORDINATES 0. 2. Ο. 6. Ο. *** * * * * * * * * * * * * HYDROGRAPH AT STATION P4 TOTAL RAINFALL = 7.25, TOTAL LOSS = .00, TOTAL EXCESS = 7.25 MAXIMUM AVERAGE FLOW PEAK FLOW TIME 6-HR 24-HR 72-HR 59.92-HR + (CFS) (HR) (CFS) 5. 1. 0. 7.247 12.08 0. 7.250 + 0 (INCHES) 5.278 7.250 0. (AC-FT) Ο. Ο. Ο. CUMULATIVE AREA = .00 SQ MI ******* * C/2 * 58 KK + ******* OUTPUT CONTROL VARIABLES 59 KO ARIABLES 3 PRINT CONTROL 0 PLOT CONTROL 0. HYDROGRAPH PLOT SCALE 7 PUNCH COMPUTED HYDROGRAPH 21 SAVE HYDROGRAPH ON THIS UNIT 1 FIRST ORDINATE PUNCHED OR SAVED IPRNT IPLOT QSCAL

H-2-31

IPNCH IOUT ISAV1

		103110	700 **			
		TIMINT	.083 TI	AST ORDINATE P IME INTERVAL I	N HOURS	SAVED
60 HC	HYDR	OGRAPH COMBI	NATION			
		ICOMP	2 NU	JMBER OF HYDRO	GRAPHS TO C	COMBINE
					* * *	
* * *		***	* * *	***		***
		HYDROGRA	PH AT STATI	ION C/2		
PEAK FLOW	TIME			MAXIMUM AVER	AGE FLOW	
			6-HR	24-HR	72-HR	59.92-HR
+ (CFS)	(HR)	(070)				
F 65.	12.50	(CFS)	28.	9.	3.	3.
		(INCHES)	4.265	5.274	5.278	5.278
		(AC-FT)	14.	17.	17.	17.

*** ***

61 KK

+ +

* * * ****************** ROUTE THROUGH 2-30 INCH CMPS

CUMULATIVE AREA = .06 SQ MI

63	KO	OUTPUT CONTROL	VARIABLES	
		IPRNT	3	PRINT CONTROL
		IPLOT	0	PLOT CONTROL
		QSCAL	0.	HYDROGRAPH PLOT SCALE
		IPNCH	7	PUNCH COMPUTED HYDROGRAPH
		IOUT	21	SAVE HYDROGRAPH ON THIS UNIT
		ISAV1	1	FIRST ORDINATE PUNCHED OR SAVED
		ISAV2	720	LAST ORDINATE PUNCHED OR SAVED
		TIMINT	.083	TIME INTERVAL IN HOURS

HYDROGRAPH ROUTING DATA

64 I	RS	STOP	AGE ROUTIN NSTPS ITYP RSVRIC X	IG ELEV 627.70 .00	NUMBER OF TYPE OF II INITIAL C WORKING R	SUBREACHE NITIAL CON ONDITION AND D COEF	3 DITION FICIENT						
65 5	SA		AREA	.0	.2	.2	.2	.4					
66 5	SE	ELEV	ATION	627.70	628.00	629.00	630.00	631.00					
68 5	SL	LOW-	LEVEL OUTI ELEVL CAREA COQL EXPL	JET 628.45 9.82 .80 .50	ELEVATION CROSS-SEC COEFFICIE EXPONENT	ELEVATION AT CENTER OF OUTLET CROSS-SECTIONAL AREA COEFFICIENT EXPONENT OF HEAD							
67 5	3S	SPII	LWAY CREL SPWID COQW EXPW	630.00 20.00 2.60 1.50	SPILLWAY SPILLWAY WEIR COEF EXPONENT	CREST ELEV WIDTH FICIENT OF HEAD	ATION						
							* * *						
					CO	MPUTED STO	RAGE-ELEVA1	TION DATA					
		STORAGE ELEVATION	.00 627.70	.02 628.00	.20 629.00	.40 630.00	.68 631.00						
					CO	MPUTED OUT	FLOW-ELEVA	TION DATA					
		OUTFLOW ELEVATION	.00 627.70	.00 628.45	77.18 629.95	77.35 629.96	77.53 629.97	77.71 629.97	77.88 629.98	78.06 629.99	78.24 629.99	78.42 630.00	
		OUTFLOW ELEVATION	85.34 630.15	88.63 630.21	92.77 630.28	97.83 630.35	103.92 630.44	111.11 630.53	119.48 630.64	129.12 630.75	140.13 630.87	152.58 631.00	
					COMPUT	ED STORAGE	-OUTFLOW-EI	EVATION DA	.TA				
		STORAGE OUTFLOW ELEVATION	.00 .00 627.70	.02 .00 628.00	.10 .00 628.45	.20 46.71 629.00	.39 77.18 629.95	.39 77.53 629.97	.39 77.88 629.98	.40 78.24 629.99	.43 85.34 630.15	.44 88.63 630.21	
		STORAGE	.46	.48	.50	.53	.56	.59	.63	.68			

 630.28
 630.35
 630.44
 630.53
 630.64
 630.75
 630.87
 631.00
 ELEVATION *** WARNING *** MODIFIED PULS ROUTING MAY BE NUMERICALLY UNSTABLE FOR OUTFLOWS BETWEEN 0. TO 47. THE ROUTED HYDROGRAPH SHOULD BE EXAMINED FOR OSCILLATIONS OR OUTFLOWS GREATER THAN PEAK INFLOWS. THIS CAN BE CORRECTED BY DECREASING THE TIME INTERVAL OR INCREASING STORAGE (USE A LONGER REACH.)

* * * * * * * * * * * * ***

HYDROGRAPH AT STATION R/P4

PEAK FLOW		TIME			MAXIMUM AVER	RAGE FLOW	
				6-HR	24-HR	72-HR	59.92-HR
+	(CFS)	(HR)					
			(CFS)				
+	65.	12.58		28.	9.	з.	3.
			(INCHES)	4.265	5.248	5.248	5.248
			(AC-FT)	14.	17.	17.	17.
PH	EAK STORAGE	TIME			MAXIMUM AVERA	GE STORAGE	
				6-HR	24-HR	72-HR	59.92-HR
+	(AC-FT)	(HR)					
	0.	12.58		0.	0.	0.	0.
1	PEAK STAGE	TIME			MAXIMUM AVER	AGE STAGE	
				6-HR	24-HR	72-HR	59.92-HR
+	(FEET)	(HR)					
	629.57	12.58		628.85	628.57	628.43	628.43
			CUMULATIV	E AREA =	.06 SO MI		

*** ***** * * * * P3 * 69 KK ***** POND 3 OUTPUT CONTROL VARIABLES IPRNT 3 PRINT CONTROL IPLOT 0 PLOT CONTROL 71 KO 0. HUDT CONTROL 0. HYDROGRAPH PLOT SCALE 7 PUNCH COMPUTED HYDROGRAPH 21 SAVE HYDROGRAPH ON THIS UNIT 1 FIRST ORDINATE PUNCHED OR SAVED 720 LAST ORDINATE PUNCHED OR SAVED .083 TIME INTERVAL IN HOURS QSCAL IPNCH IOUT TSAV1 ISAV2 TIMINT SUBBASIN RUNOFF DATA SUBBASIN CHARACTERISTICS TAREA .01 SUBBASIN AREA 72 BA PRECIPITATION DATA 10 PH DEPTHS FOR 0-PERCENT HYPOTHETICAL STORM HYDRO-35 5-MIN 15-MIN 60-MIN .71 1.54 3.20 STORM AREA = .01 SCS LOSS RATE 73 LS .00 INITIAL ABSTRACTION 100.00 CURVE NUMBER .00 PERCENT IMPERVIOUS AREA STRTL CRVNBR RTIMP SCS DIMENSIONLESS UNITGRAPH 74 UD TLAG .00 LAG * * * UNIT HYDROGRAPH 5 END-OF-PERIOD ORDINATES 9. 32. 2. 0. 0. * * * * * * * * * * * * * * * HYDROGRAPH AT STATION P3 TOTAL RAINFALL = 7.25, TOTAL LOSS = .00, TOTAL EXCESS = 7.25 MAXIMUM AVERAGE FLOW PEAK FLOW TIME 6-HR 24-HR 59.92-HR 72-HR + (CFS) (HR) (CFS) 27. 12.08 0. 7.250 2. 0. 7.250 2. (INCHES) 5.278 7.247 (AC-FT) 2. 2. CUMULATIVE AREA = .01 SQ MI ***

		*******	* * *		
		*	*		
75	KK	* C/2	*		
		*	*		
		********	* * *		
76	KO	OUTPUT	CONTROL	VARIABLES	
			IPRNT	3	PRINT CONTROL
			IPLOT	0	PLOT CONTROL

		QSCAL IPNCH IOUT ISAV1 ISAV2 TIMINT	0. 7 21 1 720 .083	HYDROGRAP PUNCH COM SAVE HYDR FIRST ORD LAST ORDI TIME INTE	H PLOT SC PUTED HYE OGRAPH ON INATE PUN NATE PUNC RVAL IN H	CALE DROGRAPH N THIS UNIT NCHED OR SAV CHED OR SAV HOURS	VED ED					
77 HC	HYDRO	GRAPH COME ICOMP	INATION 2	NUMBER OF	HYDROGRA	APHS TO COM	BINE					
						* * *						
* * *		* * *	* * *		* * *		* * *					
		HYDROGR	APH AT ST	ATION	C/2							
PEAK FLOW	TIME			MAXIMU	M AVERAGE	E FLOW						
+ (CES)	(HP)		6-HR	24-	HR	72-HR	59.92-HR					
, (015)	10.00	(CFS)	21		0	,	4					
+ /9.	12.08	(INCHES) (AC-FT)	4.337 15.	1 5.4 1	0. 04 9.	4. 5.414 19.	4. 5.414 19.					
		CUMULATI	VE AREA =	.07 S	Q MI							
*** *** **	* *** *** **	* *** ***	*** *** *	** *** ***	*** ***	*** *** **	* *** *** *	*** *** ***	*** *** **	* *** ***	*** *** **	* *** ***
	********** *	* * * *										
78 KK	* P3/DP *	1 * *										
	*****	***	TUROTICU 3	O TNCH CMD	OFFOTTE							
80 70		NUUIE	UADIADIES	U INCH UMP	OFFOLIE							
80 KO	00190	IPRNT	VARIABLES	PRINT CON	TROL							
		IPLOT QSCAL	0.	PLOT CONT HYDROGRAP	ROL H PLOT SC	CALE						
		IPNCH IOUT	7 21	PUNCH COM SAVE HYDR	PUTED HYD OGRAPH ON	DROGRAPH N THIS UNIT						
		ISAV1	1	FIRST ORD	INATE PUN	NCHED OR SAV	VED					
		TIMINT	.083	TIME INTE	RVAL IN F	HED OK SAV.	60					
	HYDROGR	APH ROUTIN	IG DATA									
81 RS	STORA	GE ROUTING NSTPS ITYP RSVRIC X	1 ELEV 625.50 .00	NUMBER OF TYPE OF I INITIAL C WORKING R	SUBREACH NITIAL CO ONDITION AND D COB	HES DNDITION EFFICIENT						
82 SA		AREA	.0	.1	.3	.4	.5	.6				
83 SE	ELEVA	TION	625.50	625.60	626.00	627.00	628.00	629.00				
85 SL	LOW-L	EVEL OUTLE	т									
		ELEVL CAREA	626.55 4.91	ELEVATION CROSS-SEC	AT CENTE TIONAL AF	ER OF OUTLE REA	Т					
		COQL EXPL	.80	COEFFICIE	NT OF HEAD							
94 00	CDIII	DALD	.00	DAT ONDAT	OI IILAD							
04 55	SFILL	CREL	628.00	SPILLWAY	CREST ELE	EVATION						
		COQW	2.60	WEIR COEF	WIDTH FICIENT							
		EXPW	1.50	EXPONENT	OF HEAD							
						* * *						
				CO	MPUTED SI	FORAGE-ELEV.	ATION DATA					
1	STORAGE ELEVATION	.00 625.50	.00	.07	.44 627.00	1.89 628.00	1.43 629.00					
				со	MPUTED OU	JTFLOW-ELEV.	ATION DATA					
	OUTFLOW	0.0	0.0	32 98	33 61	34.26	34 93	35 63	36 37	37 13	37 92	
1	ELEVATION	625.50	626.55	627.65	627.69	627.73	627.78	627.83	627.88	627.94	628.00	
1	OUTFLOW ELEVATION	40.13 628.01	53.89 628.04	89.93 628.09	158.98 628.16	271.85 628.25	439.30 628.36	671.98 628.49	980.75 628.64	1376.23 628.81	1869.30 629.00	
				COMPUT	ED STORAG	GE-OUTFLOW-	ELEVATION I	DATA				
	STORAGE	.00	.00	.07	.26	5 .44	.72	.74	.76	.78	.81	
1	OUTFLOW ELEVATION	.00 625.50	.00 625.60	.00 626.00	.00 626.55	21.13 627.00	32.98 627.65	33.61 627.69	34.26 627.73	34.93 627.78	35.63 627.83	
	STORAGE	.83	.86	.89	.89	.91	.94	.97	1.02	1.08	1.15	
1	OUTFLOW ELEVATION	36.37 627.88	37.13 627.94	37.92 628.00	40.13 628.01	53.89 628.04	89.93 628.09	158.98 628.16	271.85 628.25	439.30 628.36	671.98 628.49	
1	STORAGE OUTFLOW ELEVATION	1.23 980.75 628.64	1.32 1376.23 628.81	1.43 1869.30 629.00								

*** WARNING *** MODIFIED PULS ROUTING MAY BE NUMERICALLY UNSTABLE FOR OUTFLOWS BETWEEN 38. TO 1869. THE ROUTED HYDROGRAPH SHOULD BE EXAMINED FOR OSCILLATIONS OR OUTFLOWS GREATER THAN PEAK INFLOWS. THIS CAN BE CORRECTED BY DECREASING THE TIME INTERVAL OR INCREASING STORAGE (USE A LONGER REACH.)

* * *	* * *	***	***		***	
	HYI	DROGRAPH AT STA	ATION P3/DP1			
PEAK FLOW	TIME	6-HP	MAXIMUM AVERA	AGE FLOW	59 92-HP	
+ (CFS)	(HR)	CFS)	24 110	72 110	55.52 m	
+ 85.	12.17 (INCH (AC-	31. HES) 4.336 -FT) 15.	10. 5.341 19.	4. 5.341 19.	4. 5.341 19.	
PEAK STORAGE	TIME	6-HR	MAXIMUM AVERAG 24-HR	E STORAGE 72-HR	59.92-HR	
+ (AC-FT) 1.	(HR) 12.17	1.	0.	Ο.	0.	
PEAK STAGE	TIME	6-HR	MAXIMUM AVERA 24-HR	AGE STAGE 72-HR	59.92-HR	
+ (FEET) 628.09	(HR) 12.17	627.32	626.78	626.58	626.58	
	CUM	JLATIVE AREA =	.07 SQ MI			
*** *** *** *	** *** *** ***	*** *** *** **	** *** *** *** ***	** *** ***	*** *** ***	*** *** *** *** *** *** *** *** *** *** *** *** ***
*	*					
*	FZ " *					
	PC	OND 2				
88 KO	OUTPUT CON IPRN QSCAI IPNC IOU ISAV ISAV TIMIN	TROL VARIABLES r 3 r 0 L 0. H 7 T 21 1 1 2 720 F <td.083< td=""></td.083<>	PRINT CONTROL PLOT CONTROL HYDROGRAPH PLOT PUNCH COMPUTED F SAVE HYDROGRAPH FIRST ORDINATE F LAST ORDINATE PU TIME INTERVAL IN	SCALE HYDROGRAPH ON THIS UN PUNCHED OR S N HOURS	IIT SAVED SAVED	
	SUBBASIN RUNG	OFF DATA				
89 BA	SUBBASIN CH TAREA	HARACTERISTICS A .00	SUBBASIN AREA			
	PRECIPITAT:	ION DATA				
10 PH	HYDRO 5-MIN 15-N .71 1.	DF-35 MIN 60-MIN .54 3.20	2-HR 3-HR 3.88 4.30 STORM P	RCENT HYPOT PP-40 6-HR 12- 5.28 6. AREA =	HETICAL STOP HR 24-HR 25 7.25	RM TP-49 2-DAY 4-DAY 7-DAY 10-DAY .00 .00 .00 .00
90 LS	SCS LOSS RÅ STRTI CRVNBI RTIMI	ATE L .00 R 100.00 P .00	INITIAL ABSTRACT CURVE NUMBER PERCENT IMPERVIC	TION DUS AREA		
91 UD	SCS DIMENS: TLAC	IONLESS UNITGRÆ G .00	APH LAG			
				***	003.00	
	5 .	1 0	5 EN	UNIT HYDRO ID-OF-PERIC	OGRAPH DD ORDINATES	
* * *	***	***	***	•	* * *	
	HYI	DROGRAPH AT STA	ATION P2			
TOTAL RAI	NFALL = 7.25	5, TOTAL LOSS =	00, TOTAL	EXCESS =	7.25	
PEAK FLOW	TIME	-	MAXIMUM AVERA	AGE FLOW		
+ (CFS)	(HR)	6-HR	24-HR	72-HR	59.92-HR	
+ 4.	(0 12.08 (INCH (AC-	CFS) 1. HES) 5.278 -FT) 0.	0. 7.247 0.	0. 7.250 0.	0. 7.250 0.	
	CUM	JLATIVE AREA =	.00 SO MI			

*** ***

		* .	*				
92	KK	* R/1	P2 *				
		*	*				
		*******	* * * * *				
			ROUTE	THROUGH 3	0 INCH CMP		
0.4	77.0	011771					
94	KO	001.50	JT CONTROL	VARIABLES	DDINE COMPOS		
			IPRNI	3	PRINI CONTROL		
			IPLOT	0	PLOT CONTROL		
			QSCAL	0.	HYDROGRAPH PL	OT SCALE	
			IPNCH	/	PUNCH COMPUTE	D HYDROGRAP	'Н
			IOUT	21	SAVE HYDROGRA	PH ON THIS	UNIT
			ISAV1	1	FIRST ORDINAT	E PUNCHED C	R SAVED
			ISAV2	720	LAST ORDINATE	PUNCHED OR	SAVED
			TIMINT	.083	TIME INTERVAL	IN HOURS	
		IIVDDOCI					
		HIDROG	KAPH ROUII	NG DAIA			
95	PS	STOR	AGE ROUTIN	G			
20	100	01010	NSTPS	1	NUMBER OF SUB	REACHES	
			TTVP	FLEV	TYPE OF INITI	AL CONDITIC	N
			REVETC	638 40	INITIAL CONDI	TTON	-14
			V	000.40	MODELING D AND	D COFFETCIE	NUT
			A	.00	WORKING K MND	D COLFFICIE	114 1
96	SA		AREA	0	0	1	
50	011					•-	
97	SE	ELEV	ATTON	638.40	640.00 641	.00	
99	SL	LOW-1	LEVEL OUTL	ET			
			ELEVL	639.25	ELEVATION AT	CENTER OF C	UTLET
			CAREA	4.91	CROSS-SECTION	AL AREA	
			COOL	. 80	COEFFICIENT		
			EXPL	50	EXPONENT OF H	EAD	
			2 1		Line on Line of th		

98	SS	SPILLWAY		
		CREL	639.90	SPILLWAY CREST ELEVATION
		SPWID	20.00	SPILLWAY WIDTH
		COQW	2.60	WEIR COEFFICIENT
		EXPW	1.50	EXPONENT OF HEAD
				* * *

COMPUTED STORAGE-ELEVATION DATA

STORAGE ELEVATION	.00 638.40	.01 640.00	.04 641.00							
			COM	PUTED OUTF	LOW-ELEVAT	ION DATA				
OUTFLOW	.00	.00	31.30	30.29	29.35	28.46	27.63	26.84	26.10	25.39
ELEVATION	638.40	639.25	640.24	640.18	640.12	640.07	640.02	639.98	639.94	639.90
OUTFLOW	29.31	32.07	35.83	40.71	46.86	54.43	63.54	74.35	87.01	101.65
ELEVATION	640.01	640.07	640.14	640.22	640.32	640.43	640.55	640.69	640.84	641.00
			COMPUTE	D STORAGE-	OUTFLOW-EL	EVATION DA	TA			
STORAGE	.00	.00	.01	.01	.01	.01	.02	.02	.03	.04
OUTFLOW	.00	.00	28.92	41.51	46.86	54.43	63.54	74.35	87.01	101.65
ELEVATION	638.40	639.25	640.00	640.24	640.32	640.43	640.55	640.69	640.84	641.00

*** WARNING *** MODIFIED PULS ROUTING MAY BE NUMERICALLY UNSTABLE FOR OUTFLOWS BETWEEN 0. TO 102. THE ROUTED HYDROGRAPH SHOULD BE EXAMINED FOR OSCILLATIONS OR OUTFLOWS GREATER THAN PEAK INFLOWS. THIS CAN BE CORRECTED BY DECREASING THE TIME INTERVAL OR INCREASING STORAGE (USE A LONGER REACH.)

	* * *		* * *	* * *	***	t	* * *	
			HYDROGRA	PH AT STAT	ION R/P2			
	PEAK FLOW	TIME			MAXIMUM AVER	RAGE FLOW		
				6-HR	24-HR	72-HR	59.92-HR	
+	(CFS)	(HR)						
			(CFS)					
+	4.	12.08		1.	0.	0.	0.	
			(INCHES)	5.278	7.233	7.233	7.233	
			(AC-FT)	0.	0.	0.	0.	
P	EAK STORAGE	TIME			MAXIMUM AVERA	AGE STORAGE		
				6-HR	24-HR	72-HR	59.92-HR	
+	(AC-FT)	(HR)						
	0.	11.83		0.	0.	0.	0.	
	PEAK STAGE	TIME			MAXIMUM AVER	RAGE STAGE		
				6-HR	24-HR	72-HR	59.92-HR	
+	(FEET)	(HR)						
	639.37	12.08		639.26	639.25	639.25	639.25	
			CUMULATIV	E AREA =	.00 SQ MI			

*** ***

		* * * *	* * * * * * * *	* *	
		*		*	
100	KK	*	R/CHA	*	
		*		*	
		* * * *	* * * * * * * *	* *	
				CHANNEL	CHA

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102 КО	OUTPUT CONTROL IPRNT IPLOT QSCAL IPNCH IOUT ISAV1 ISAV2 TIMINT	VARIABLES 3 0 0. 7 21 1 720 .083	PRINT CO PLOT COI HYDROGRI PUNCH CO SAVE HYI FIRST OI LAST ORI TIME IN	DNTROL NTROL APH PLOT SC: DMEUTED HYDI DROGRAPH ON RDINATE PUN(DINATE PUN() TERVAL IN H(ALE ROGRAPH THIS UNIT CHED OR SA HED OR SAV DURS	VED ED				
	SUBBASIN RUNOFF I	DATA								
103 BA	SUBBASIN CHARAC TAREA	CTERISTICS .00	SUBBASI	N AREA						
	PRECIPITATION I	DATA								
10 PH		D	EPTHS FO	R 0-PERCEI	NT HYPOTHE	TICAL STO	RM			
	HYDRO-35 5-MIN 15-MIN .71 1.54	60-MIN 3.20	2-HR 3.88	3-HR 6-1 4.30 5.3	40 HR 12-HR 28 6.25	24-HR 7.25	2-DAY .00	TP-4 4-DAY .00	19 7-DAY .00	10-DAY .00
				STORM ARE	A = .0	0				
104 LS	SCS LOSS RATE STRTL CRVNBR RTIMP	.38 84.00 .00	INITIAL CURVE N PERCENT	ABSTRACTIO UMBER IMPERVIOUS	N AREA					
	KINEMATIC WAVE									
105 UK	OVERLAND-FLOW L S PA DXMIN MUSKINGUM-CUNGE	W ELEMENT 59. .0510 .350 100.0 5	NO. 1 OVERLANI SLOPE ROUGHNE: PERCENT MINIMUM	D FLOW LENG SS COEFFICI OF SUBBASI NUMBER OF 1	TH ENT N DX INTERVA	LS				
106 RD	MAIN CHANNEL L S N CA SHAPE WD Z RUPSTQ	582. .0110 .040 .00 TRAP .00 2.50 YES	CHANNEL SLOPE CHANNEL CONTRIB CHANNEL BOTTOM U SIDE SLO ROUTE U	LENGTH ROUGHNESS (UTING AREA SHAPE WIDTH OR DI DPE PSTREAM HYDI	COEFFICIEN AMETER ROGRAPH	т				
		COMPU	TED MUSK	INGUM-CUNGE	*** PARAMETER	s				
	ELEMENT A	ALPHA	COMPUT: M	ATION TIME : DT	STEP DX	PEAK	TIME TO	VOLUM	ie ma	XIMUM
				(MIN)	(FT)	(CFS)	PEAK (MIN)	(IN)	CE (LERITY FPS)
	PLANE1 MAIN	.96	1.67	.75	11.80 291.00	4.64	724.86	5.3	88	.27

CONTINUITY SUMMARY (AC-FT) - INFLOW= .3472E+00 EXCESS= .2868E+00 OUTFLOW= .6326E+00 BASIN STORAGE= .2835E-03 PERCENT ERROR= .2

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

		MA	IN	1.73	1.33	5.00		7.91	725.00	6.25
	* * *		* * *	***		* * *		***		
			HYDRO	GRAPH AT ST.	ATION I	R/CHA				
	TOTAL	RAINFALL =	7.25,	TOTAL LOSS	= 1.87,	, TOTAL EX	CESS =	5.38		
	PEAK FLOW	TIME			MAXIM	JM AVERAGE	FLOW			
+	(CFS)	(HR)		6-HR	24-	-HR	72-HR	59.92-HR		
			(CFS)						
+	8.	12.08		1.		0.	Ο.	0.		
			(INCHES) 4.788	6.2	247	6.254	6.254		
			(AC-FT) 0.		1.	1.	1.		
			CUMULA	TIVE AREA =	.00 \$	SQ MI				

*** ***

***** ****** * LF1 * * * * SUBAREA LF1 107 KK OUTPUT CONTROL VARIABLES IPRNT 3 PRINT CONTROL IPLOT 0 PLOT CONTROL QSCAL 0. HYDROGRAPH PLOT SCALE IPNCH 7 PUNCH COMPUTED HYDROGRAPH IOUT 21 SAVE HYDROGRAPH ON THIS UNIT ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED 109 KO

	ISAV2 TIMINT	720 .083	LAST ORDI TIME INTE	INATE PUN ERVAL IN	CHED OR SAU HOURS	/ED					
	SUBBASIN RUNOFI	F DATA									
110 BA	SUBBASIN CHAR TAREA	RACTERISTICS	SUBBASIN	AREA							
	PRECIPITATION	I DATA									
10 PH	HYDRO-3 5-MIN 15-MIN .71 1.54	1 35 1 60-MIN 1 3.20	2-HR 3 3.88	0-PERC TP 3-HR 6 1.30 5	ENT HYPOTHE -40 -HR 12-HE .28 6.25	24-HR 7.25	2-DAY .00	TP-49 4-DAY 7-1 .00	DAY 10-DAY .00 .00		
				STORM AR	EA = .()4					
111 LS	SCS LOSS RATE STRTL CRVNBR RTIMP	2. 88.00 .00	INITIAL A CURVE NUN PERCENT 1	ABSTRACTI MBER IMPERVIOU	ON IS AREA						
112 UK	KINEMATIC WAV OVERLAND-FI S N PA DXMIN MUSKINGUM-CUN	VE LOW ELEMENT 88. .2500 .300 100.0 5 NGE	NO. 1 OVERLAND SLOPE ROUGHNESS PERCENT (MINIMUM N	FLOW LEN S COEFFIC DF SUBBAS NUMBER OF	GTH IENT IN DX INTERVA	ALS					
113 RD	MAIN CHANNE S N CA SHAPE WD Z RUPSTQ	EL 1788. .0100 .030 .04 TRAP .00 2.00 NO	CHANNEL I SLOPE CHANNEL F CONTRIBUT CHANNEL S BOTTOM WI SIDE SLOP ROUTE UPS	LENGTH ROUGHNESS FING AREA SHAPE IDTH OR D PE STREAM HY	COEFFICIEN IAMETER	νT					
		COMPL	MILCUTH	ICUM-CUNC	***						
	ELEMENT	ALPHA	COMPUTAT M	DT (MIN)	(FT)	PEAK (CFS)	TIME TO PEAK (MIN)	VOLUME (IN)	MAXIMUM CELERITY (FPS)		
	PLANE1 MAIN	2.48 2.31	1.67 1.33	.59 3.76	17.60 894.00	201.86 187.29	724.59 722.88	5.83 5.62	.59 7.92		
CONTINUITY SU	MMARY (AC-FT) - 1	INFLOW= .000	0E+00 EX0	CESS= .12	48E+02 OUTH	FLOW= .120	1E+02 BAS:	IN STORAGE=	.7302E-03 PE	RCENT ERROF	R= 3.7

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

		MAII	Ň	2.31	1.33	5.00		171.06	725.00	5.59
	* * *		* * *	***		* * *		***		
			HYDROGRA	PH AT STA	TION	LF1				
	TOTAL	RAINFALL =	7.25, TOI	AL LOSS =	1.41,	TOTAL EXC	ESS =	5.84		
1	PEAK FLOW	TIME		6-HP	MAXIMU	M AVERAGE	FLOW 2-HP	59 92-HP		
+	(CFS)	(HR)	(CES)	0 111	24	, inc	2 1110	55.52 m		
+	171.	12.08	(INCHES) (AC-FT)	20. 4.553 10.	5.5	6. 87 5 2.	2. .588 12.	2. 5.588 12.		
			CUMULATIV	E AREA =	.04 S	Q MI				

*** ***

		* * * * *	******		
		*	*		
114	KK	*	P1 *		
		*	*		
		* * * * *	* * * * * * * * *		
			POI	ND 1	
116	KO		OUTPUT CONTI	ROL VARIABLES	
			IPRNT	3	PRINT CONTROL
			IPLOT	0	PLOT CONTROL
			QSCAL	0.	HYDROGRAPH PLOT SCALE
			IPNCH	7	PUNCH COMPUTED HYDROGRAPH
			IOUT	21	SAVE HYDROGRAPH ON THIS UNIT
			ISAV1	1	FIRST ORDINATE PUNCHED OR SAVED
			ISAV2	720	LAST ORDINATE PUNCHED OR SAVED
			TIMINT	.083	TIME INTERVAL IN HOURS
		SU	JBBASIN RUNO	FF DATA	

117 BA SUBBASIN CHARACTERISTICS TAREA .00 SUBBASIN AREA

	PRECIPITATI	ON DATA							
10 PH		r	DEPTHS FOR 0-1	PERCENT HYPO	THETICAL STO	RM			
	HYDRO	-35		. TP-40			TP	-49	
	5-MIN 15-M .71 1.	IN 60-MIN 54 3.20	2-HR 3-HR 3.88 4.30	6-HR 12 5.28 6	-HR 24-HR .25 7.25	2-DAY .00	4-DAY .00	/-DAY	10-DAY .00
			STORM	1 AREA =	.00				
118 LS	SCS LOSS RA	TE							
	STRTI CRVNBF	.00	INITIAL ABSTRA CURVE NUMBER	ACTION					
	RTIME	.00	PERCENT IMPERV	/IOUS AREA					
119 UD	SCS DIMENSI	ONLESS UNITG	RAPH						
	TLAG	.00	LAG						
				* * *					
			5	UNIT HYDR	OGRAPH OD ORDINATES				
	20. 6	. 1.	0.	0.					
* * *	* * *	***	,	**	* * *				
	HYD	ROGRAPH AT ST	TATION P1						
TOTAL F	AINFALL = 7.25	, TOTAL LOSS	= .00, TOT#	AL EXCESS =	7.25				
PEAK FLOW	TIME		MAXIMUM AVE	ERAGE FLOW					
		6-HI	R 24-HR	72-HR	59.92-HR				
+ (CFS)	(HR)	FS)							
+ 17.	12.08	2	. 1.	0.	0.				
	(INCH (AC-	ыз) 5.278 FT) 1	s 7.247 . 1.	1.250	7.250				
	CUMU	LATIVE AREA =	= .UU SQ MI						
*** *** ***	*** *** *** ***	*** *** *** *	*** *** *** ***	*** *** ***	*** *** ***	*** ***	*** ***	*** ***	< *** *** *** *

120 KK	* C/3 *								
	* *								
121 KO	OUTPUT CONT	ROL VARIABLES	BRINE COMERCI						
	IPRNI IPLOI	3	PRINT CONTROL PLOT CONTROL						
	QSCAI	0.	HYDROGRAPH PLC	DT SCALE					
	IOUT	21	SAVE HYDROGRAE	PH ON THIS U	NIT				
	ISAV1	1	FIRST ORDINATE	E PUNCHED OR	SAVED				
	TIMIN1	.083	TIME INTERVAL	IN HOURS	SAVED				
122 HC	HYDROGRAPH	COMBINATION							
	ICOME	3	NUMBER OF HYDE	ROGRAPHS TO	COMBINE				

* * *	* * *	**		**	***				
	HYI	ROGRAPH AT ST	PATION C/3						
PEAK FLOW	TIME		MAXIMUM AVE	ERAGE FLOW					
+ (CFS)	(HR)	6-HF	R 24-HR	72-HR	59.92-HR				
(010)	() (0	FS)							
⊦ 196.	12.08 (TNC)	23. ES) 4 617	. 7. 5 5.737	3. 5.743	3.				
	(AC-	FT) 11	. 14.	14.	14.				
	CTIMI	LATIVE APEA -	= 05 SO MT						
	COMO	ANDA -	.00 50 MI						
*** *** ***	*** *** *** ***	*** *** *** *	*** *** *** ***	*** *** ***	*** *** ***	*** ***	*** ***	*** ***	* *** *** *** *

123 KK	* R/P1 *								
	* *								
	**********************************	UTE THROUGH :	36 INCH RCP						
105									
172 KQ	OUTPUT CONT IPRNT	KUL VARIABLES	PRINT CONTROL						
	IPLOT	0	PLOT CONTROL						
	QSCAL	0.	HYDROGRAPH PLC PUNCH COMPUTER	DT SCALE HYDROGRAPH					
	IOUI	21	SAVE HYDROGRAE	PH ON THIS U	NIT				
	ISAV1 TSAV2	1 720	FIRST ORDINATE	PUNCHED OR	SAVED SAVED				
	I OFIV Z	,20	TTME INTERVAL	TN HOUDO					

HYDROGRAPH ROUTING DATA

126 RS	STOP	RAGE ROUTING NSTPS ITYP RSVRIC X	1 ELEV 626.00	NUMBER OF TYPE OF I INITIAL O	F SUBREACH INITIAL CON CONDITION	ES NDITION						
127 SA		AREA	.0	.0	.5	.6	.6	.7				
128 SE	ELEV	ATION	626.00	628.60	629.00	630.00	631.00	632.00				
130 SL	LOW-	-LEVEL OUTLE ELEVL CAREA COQL EXPL	T 623.50 7.07 .80 .50	ELEVATION CROSS-SEC COEFFICIE EXPONENT	N AT CENTER CTIONAL ARI ENT OF HEAD	R OF OUTLET EA	ſ					
129 SS	SPII	LLWAY CREL SPWID COQW EXPW	631.00 15.00 2.60 1.50	SPILLWAY SPILLWAY WEIR COEH EXPONENT	CREST ELE' WIDTH FFICIENT OF HEAD	VATION						
						* * *						
				cc	MPUTED ST	ORAGE-ELEVA	ATION DATA					
E	STORAGE LEVATION	.00 626.00	.00 628.60	.07 629.00	.60 630.00	1.20 631.00	1.87 632.00					
				CC	MPUTED OU	FFLOW-ELEVA	ATION DATA					
E	OUTFLOW LEVATION	.00 626.00	75.26 626.25	79.16 626.55	83.48 626.89	88.31 627.29	93.73 627.77	99.86 628.35	106.85 629.05	114.88 629.91	124.23 631.00	
E	OUTFLOW LEVATION	125.11 631.05	126.24 631.10	128.02 631.16	130.61 631.24	134.15 631.33	138.79 631.43	144.67 631.55	151.95 631.69	160.76 631.84	171.25 632.00	
				COMPUT	TED STORAG	E-OUTFLOW-E	ELEVATION D	ATA				
E	STORAGE OUTFLOW LEVATION	.00 71.72 626.00	.00 75.26 626.25	.00 79.16 626.55	.00 83.48 626.89	.00 88.31 627.29	.00 93.73 627.77	.00 99.86 628.35	.00 102.44 628.60	.07 106.38 629.00	.09 106.85 629.05	
E	STORAGE OUTFLOW LEVATION	.55 114.88 629.91	.60 115.65 630.00	1.20 124.23 631.00	1.23 125.11 631.05	1.26 126.24 631.10	1.30 128.02 631.16	1.35 130.61 631.24	1.41 134.15 631.33	1.48 138.79 631.43	1.56 144.67 631.55	
E	STORAGE OUTFLOW LEVATION	1.65 151.95 631.69	1.76 160.76 631.84	1.87 171.25 632.00								
* * *		***	* * *		* * *	÷	***					
		HYDROGR	APH AT SI	ATION	R/Pl							
PEAK FLOW + (CFS)	TIME (HR)		6-HF	MAXIMU 24-	JM AVERAGE -HR	FLOW 72-HR 5	59.92-HR					
+ 120.	12.17	(CFS) (INCHES) (AC-FT)	23. 4.618 11.	5.8	7. 331 14.	3. 5.849 14.	3. 5.849 14.					
PEAK STORAG	E TIME		6-HR	MAXIMUN 24-	4 AVERAGE	STORAGE	59.92-HB					
+ (AC-FT) 1.	(HR) 12.17		0.		0.	0.	0.					
PEAK STAGE	TIME		C 110	MAXIMU	JM AVERAGE	STAGE						
+ (FEET) 630.51	(HR) 12.17		626.35	626.	-нк .09 б:	26.04	626.04					
		CUMULATI	VE AREA =	= .05 s	SQ MI							
*** *** ***	*** *** *	*** *** ***	*** *** *	** *** ***	* *** ***	*** *** **;	* *** *** *	** *** ***	*** *** **	* *** ***	*** *** ***	*** ***
131 KK	********* * * C	****** * CHB * * ****** SUBARE	A CHB									

133 KO OUTPUT CONTROL VARIABLES IPRNT 3 PRINT CONTROL IPLOT 0 PLOT CONTROL QSCAL 0. HYDROGRAPH PLOT SCALE IPNCH 7 PUNCH COMPUTED HYDROGRAPH IOUT 21 SAVE HYDROGRAPH ON THIS UNIT ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED ISAV2 720 LAST ORDINATE PUNCHED OR SAVED TIMINT .083 TIME INTERVAL IN HOURS

	SUBBASIN RUNOFF	DATA							
134 BA	SUBBASIN CHAR TAREA	ACTERISTICS	SUBBASI	N AREA					
	PRECIPITATION	DATA							
10 PH	HYDRO-3 5-MIN 15-MIN .71 1.54	5 60-MIN 3.20	2-HR 3.88	R 0-PERCE TP- 3-HR 6- 4.30 5.	NT HYPOTH 40 HR 12-H 28 6.2	ETICAL STC R 24-HR 5 7.25	DRM 2-DAY 4 .00	TP-49 . -DAY 7-D .00 .	AY 10-DAY 00 .00
				STORM ARE	A =	02			
135 LS	SCS LOSS RATE STRTL CRVNBR RTIMP	.27 88.00 .00	INITIAL CURVE N PERCENT	ABSTRACTIC UMBER IMPERVIOUS	N AREA				
	KINEMATIC WAV	Έ							
136 UK	OVERLAND-FL	OW ELEMENT	NO. 1						
	L	184.	OVERLAN	D FLOW LENG	TH				
	S	.2240	SLOPE						
	N	.300	ROUGHNE	SS COEFFICI	ENT				
	PA	100.0	PERCENT	OF SUBBASI	N				
	DXMIN	5	MINIMUM	NUMBER OF	DX INTERV	ALS			
	MUSKINGUM-CUN	GE							
137 RD	MAIN CHANNE	L							
	L	2088.	CHANNEL	LENGTH					
	S	.0040	SLOPE						
	N	.030	CHANNEL	ROUGHNESS	COEFFICIE	NT			
	CA	.02	CONTRIB	UTING AREA					
	SHAPE	TRAP	CHANNEL	SHAPE					
	WD	10.00	BOTTOM	WIDTH OR DI	AMETER				
	Z	2.00	SIDE SL	OPE					
	RUPSTQ	NO	ROUTE U	PSTREAM HYD	ROGRAPH				
					* * *				
		COMPL	ITED MUSK	TNGUM-CUNGE	PARAMETEI	RS			
			COMPUT	ATION TIME	STEP				
	ELEMENT	ALPHA	М	DT	DX	PEAK	TIME TO PEAK	VOLUME	MAXIMUM CELERITY
				(MIN)	(FT)	(CFS)	(MIN)	(IN)	(FPS)
	PLANE1	2.35	1.67	.88	36.80	80.61	725.26	5.84	.74
	MAIN	.87	1.44	5.00	696.00	67.03	725.00	5.30	4.06

CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .5322E+01 OUTFLOW= .4834E+01 BASIN STORAGE= .2292E-02 PERCENT ERROR= 9.1

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

		MAI	N	.87	1.44	5.00		67.03	725.00	5.30
	* * *		***	***		***		***		
			HYDROGRA	PH AT STA	TION	CHB				
	TOTAL RA	INFALL =	7.25, TOT	AL LOSS =	1.41	, TOTAL	EXCESS =	5.84		
1	PEAK FLOW	TIME			MAXIM	UM AVERA	GE FLOW			
				6-HR	24	-HR	72-HR	59.92-HR		
+	(CFS)	(HR)								
			(CFS)							
+	67.	12.08		8.		2.	1.	1.		
			(INCHES)	4.434	5.	299	5.300	5.300		
			(AC-FT)	4.		5.	5.	5.		
			CUMULATIV	e area =	.02	SQ MI				

	HYDRO-35	5		TI	2-40			TP-4	9	
	5-MIN 15-MIN	60-MIN	2-HR	3-HR 6	5-HR 12-H	R 24-HR	2-DAY	4-DAY	7-DAY	10-DAY
	.71 1.54	3.20	3.88	4.30 5	5.28 6.2	5 7.25	.00	.00	.00	.00
				STORM AN	REA = .	02				
142 LS	SCS LOSS RATE									
	STRTL	.27	INITIAI	L ABSTRACT	EON					
	CRVNBR	88.00	CURVE 1	JUMBER						
	RTIMP	.00	PERCENT	IMPERVIOU	JS AREA					
	WTNEWS #10 10310	-								
142 112	OVEDIAND EL	DI ETEMENIE	NO 1							
145 UK	UVERLAND-FLC	JW ELEMENI	NU. I	ID FLOW LEN	ICMU					
	L	2500	CLODE	ND FLOW LEI	NGIH					
	5	.2300	DOUCUNI	COPPET	TENID					
	IN D.D.	100 0	DEDGENI	LSS COLFFIC	L LINI					
	DVMTN	100.0	MINIMUM	A NUMBER OF	TNUEDU	A L C				
	MUCKINCUM_CUN	- -	MINIMOR	I NOMBER OI	DA INIERV	A110				
144 PD	MAIN CUANNEL									
INN RD	T.	1920	CHANNEL	LENCTH						
	c	0100	CIANNE							
	N	.0100	CHANNEL	ROUGHNESS	COFFETCIE	NT				
	CD	.050	CONTRAL	DITTIC ADE?	V CODITICID					
	CUADE	-02 TDAD	CUANNEL	CUNDE	1					
	WD	100	BOTTOM	WIDTH OR I	TAMETER					
	7	2 00	CIDE CI	ODE	JINNE I DIV					
	DIDOTO	2.00	BOUTE I	IDCTDEAM UN	/DDOCD3 DU					
	ROFSIQ	NO	KOUIL (JESIKEAM II.	DROGRAFI					
					* * *					
		COMPL	ITED MUSH	TNGUM-CUN	E PARAMETE	RS				
			COMPU	TATTON TIME	STEP					
	ELEMENT	AL.PHA	M	DT	DX	PEAK	TIME TO	VOLUM	IE M	AXTMUM
				21	211	1 20100	PEAK	10201	C	ELERITY
				(MIN)	(FT)	(CFS)	(MIN)	(IN)	-	(FPS)
	PLANE1	2.48	1.67	.76	24.00	104.79	724.75	5.8	34	.67
	MAIN	2.31	1.33	4.77	960.00	100.77	725.21	5.6	5	6.71

CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .6536E+01 OUTFLOW= .6327E+01 BASIN STORAGE= .7921E-03 PERCENT ERROR= 3.2

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

		MAI	IN	2.31	1.33	5.00	99.44	725.00	5.66
	* * *		***	***		* * *	***		
			HYDROGRA	APH AT STA	TION I	F2			
	TOTAL	RAINFALL =	7.25, TO	FAL LOSS =	1.41, T	OTAL EXCESS =	5.84		
	PEAK FLOW	I TIME			MAXIMUM	AVERAGE FLOW			
+	(CFS)	(HR)		6-HR	24-HR	/2-HR	59.92-HR		
+	99.	12.08	(CFS)	10.	3.	1.	1.		
			(INCHES)	4.624	5.661	5.662	5.662		
			(AC-FT)	5.	6.	6.	б.		
			CUMULATI	VE AREA =	.02 SQ	MI			

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		*******	* * * * *				
		*	*				
	145 KK	* C	/3 *				
		*	*				
		*******	* * * * *				
	146 KO	OUTP	UT CONTROL	VARIABLES			
			IPRNT	3	PRINT CONTROL		
			IPLOT	0	PLOT CONTROL		
			QSCAL	0.	HYDROGRAPH PLO	DT SCALE	
			IPNCH	7	PUNCH COMPUTEI) HYDROGRAPH	
			IOUT	21	SAVE HYDROGRAN	PH ON THIS UN	IIT
			ISAV1	1	FIRST ORDINATE	E PUNCHED OR	SAVED
			ISAV2	720	LAST ORDINATE	PUNCHED OR S	SAVED
			TIMINT	.083	TIME INTERVAL	IN HOURS	
	1.47 110	IIVDD	OCDADU. COM				
	14/ HC	HIDR	UGRAPH COM	SINAILON	NUMBER OF UVE		OMDINE
			ICOMP	2	NUMBER OF HIDE	COGRAPHS IO C	JOMBINE

	* * *		* * *	***	* *	**	* * *
			HYDROGI	RAPH AT STA	ATION C/3		
	DEAK DLOW						
	PEAK FLOW	TIME		6 UD	MAXIMUM AVE	SRAGE FLOW	E0 02 UD
	(070)	(110)		0-HK	24=nr	/2=nk	39.92-HK
+	(CFS)	(HK)	(070)				
	000	10.00	(CFS)	41	10	F	-
+	282.	12.08	(=)	41.	13.	5.	5.
			(INCHES)	4.582	5.669	5.689	5.689
			(AC-FT)	20.	25.	25.	25.

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148 KK $\frac{1}{100}$ SUBAREA CHC SUBAREA CHC 150 KO 00TUTUT CONTROL VARIABLES IFRNT 3 FRINT CONTROL IFRNT 3 FRINT CONTROL (SGCAL 0. HYDROGRAPH PLOT SCALE IFRNT 1 SAVE HURBORARE ON THIS BTT ISAVE HURBORARE ON THIS BTT ISAVE HURBORARE ON THIS OF SAVED ISAVED ISAVE INTERVAL IN HOURS SUBBASIN RUNOFF DATA 151 BA SUBBASIN RUNOFF DATA 151 BA SUBBASIN CHARACTERISTICS TAREA .01 SUBBASIN AREA PRECIPITATION DATA 10 PH 			****								
148 KK · CHC · · · · · · · · · · · · · · · · ·			* *								
SUBARA CH SUBARA CH IENNT 3 FRINT CONTROL IENNT 3 FRINT CONTROL IENNT 3 FRINT CONTROL GCAL 0. HIDROGRAPH FLOT SCALE I TOT 1 PLOT CONTROL GCAL 0. HIDROGRAPH FLOT SCALE I TOT 1 FIRST ORDINATE FUNCHED OR SAVED I SUBARSIN CUNNOFF DATA SUBBASIN CUNNOFF DATA SUBBASIN CUNNOFF DATA 10 FH DEPTHS FOR 0-PERCENT HYDOTHETICAL STORM HURRO-35	148	KK	* CHC *								
$\begin{tabular}{l lllllllllllllllllllllllllllllllllll$			^ ^ ^								
150 K0 OUTPUT CONTROL VARIABLES LIFLOT 0 PLOT CONTROL IFLOT 0 FLOT CONTROL OUTPUT CONTROL WARKARLES LIFLOT 0 PLOT CONTROL IFLOT 0 FLOT CONTROL IFLOT 0 PLOT CONTROL IFLOT 0 PLOT CONTROL IFLOT 0 FLOT CONTROL IFLOT 0 FLOT CONTROL WARKARLES SUBBASIN CHARACTERISTICS TAREA .01 SUBBASIN AREA PRECIPITATION DATA 10 PH DEFTNS FOR 0-PERCENT HYPOTHETICAL STORM HYDRO-35 DEFTNS FOR 0-DX INTERVALS ISTORM AREA = .01 152 LS SCS LOSS RATE STORM AREA = .01 153 UK OVERLAND FLOW ELEMENT NO.1 L 140. OVERLAND FLOW MERGEN RTHP 00 FERCENT IMPERVIOUS AREA HUBKINOUM-CUNCE MAIN (CHNNEL S .0050 SLOPE N .010.00 REGENT OF SUBBASIN MUSINGUM-CUNCE NAINO (ENNEL L 645. CHANNEL LENGTH S .0030 SLOPE NO.10.00 BOTTOM MILDER OF SUBBASIN MUSINGUM-CUNCE TRANKLI SHAPE NAINO (ENNEL S .0030 SLOPE N .010 CONTENDENTION AREA SIGNE DEFTNENCENTER NO.10.00 BOTTOM HILD SHAPE NO.10.00 BOTTOM HILD SHA			SUBAREA	CHC							
IFANT 3 PRINT CONTROL GCAL 0. HYDROGRAPH FLOT SCALE IFNOL 1. PUNCH COMPUTED HYDROGRAPH HLOT SCALE IFNOL 1. STORE HYDROGRAPH NOT THIS DE SAVED 1. SOUTH 2. SAVED 1. SAVED 1. SOUTH 2. SAVED 1. SAVED 1. SUBBASIN RUNOFF DATA SUBBASIN RUNOFF DATA 151 BA SUBBASIN CHARACTERISTICS TAREA .01 SUBBASIN AREA PRECIPITATION DATA 10 PH DEFTHS FOR 0-PERCENT HYDOTHETICAL STORM 	150	KO	OUTPUT CONTROL V	ARIABLES							
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I = O(1 + O) PUNCH COMPUTED NUDRORAPH HIS UNIT I SAVI 1 SAVE HYDORGRAPH ON THIS UNIT I SAVI 1 OTT 21 SAVE HYDORGRAPH ON THIS UNIT I SAVI 1 OTT 20 LAST ORDINATE PUNCHED OR SAVED I SAVE 1 OTT 20 LAST ORDINATE PUNCHED OR SAVED SUBBASIN CHARACTERISTICS TAREA .01 SUBBASIN AREA FRECIPITATION DATA 10 PH DEPTHS FOR 0-PERCENT HYDOTHETICAL STORM HYDRO-35 TP-40			IPLOT OSCAL	0	HYDROGRAF	H PLOT SCI	TF				
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THINT			ISAV1	1	FIRST ORD	INATE PUNC	CHED OR SA	VED			
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10 PH DEFTHS FOR 0-PERCENT HYPOTHETICAL STORM 11 PH			TAREA	.01	SUBBASIN	AREA					
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1	10	PH		D	EPTHS FOR	0-PERCEN	NT HYPOTHE	TICAL STC	RM		
<pre>152 LS</pre>			HYDRO-35 . 5-MIN 15-MIN 6	0-MIN	2-HR 3	-HR 6-F	10 IR 12-HR	24-HR	2-DAY 4	TP-49 . -DAY 7-D	AY 10-DAY
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154 RD MAIN CHANNEL L 645. CHANNEL LENGTH S .0050 SLOPE N .030 CHANNEL ROUGHNESS COEFFICIENT CA .01 CONTRIBUTING AREA SHAPE TRAPE CHANNEL SHAPE WD 10.00 BOTTOM WIDTH OR DIAMETER Z 2.00 SIDE SLOPE RUPSTQ YES ROUTE UPSTREAM HYDROGRAPH **** COMPUTED MUSKINGUM-CUNGE PARAMETERS COMPUTATION TIME STEP ELEMENT ALPHA M DT DX PEAK TIME TO VOLUME MAXIMUM CELERITY (MIN) (FT) (CFS) (MIN) (IN) (FFS) PLANE1 2.48 1.67 .72 28.00 35.22 724.95 5.84 .70			MUSKINGUM-CUNGE	C	MINIMUM N	UMBER OF L	JX INTERVA	12			
L 645. CHANNEL LENGTH S .0050 SLOPE N .030 CHANNEL ROUGHNESS COEFFICIENT CA .01 CONTRIBUTING AREA SHAPE TRAP CHANNEL SHAPE WD 10.00 BOTTOM WIDTH OR DIAMETER Z 2.00 SIDE SLOPE RUPSTQ YES ROUTE UPSTREAM HYDROGRAPH *** COMPUTED MUSKINGUM-CUNGE PARAMETERS COMPUTATION TIME STEP ELEMENT ALPHA M DT DX PEAK TIME TO VOLUME MAXIMUM CELERITY (MIN) (FT) (CFS) (MIN) (IN) (FFS) PLANE1 2.48 1.67 .72 28.00 35.22 724.95 5.84 .70	154	RD	MAIN CHANNEL								
S .0050 SLOPE N .030 CHANNEL ROUGHNESS COEFFICIENT CA .01 CONTRIBUTING AREA SHAPE TRAP CHANNEL SHAPE WD 10.00 BOTTOM WIDTH OR DIAMETER Z 2.00 SIDE SLOPE RUPSTQ YES ROUTE UPSTREAM HYDROGRAPH **** COMPUTED MUSKINGUM-CUNGE PARAMETERS COMPUTATION TIME STEP ELEMENT ALPHA M DT DX PEAK TIME TO VOLUME MAXIMUM CELERITY (MIN) (FT) (CFS) (MIN) (IN) (FPS) PLANE1 2.48 1.67 .72 2.8.00 35.22 724.95 5.84 .70			L	645.	CHANNEL L	ENGTH					
CA . 01 CONTRIBUTING AREA SHAPE TRAP CHANNEL SHAPE WD 10.00 BOTTOM WIDTH OR DIAMETER Z 2.00 SIDE SLOPE RUPSTQ YES ROUTE UPSTREAM HYDROGRAPH **** COMPUTED MUSKINGUM-CUNGE PARAMETERS COMPUTATION TIME STEP ELEMENT ALPHA M DT DX PEAK TIME TO VOLUME MAXIMUM PEAK CELERITY (MIN) (FT) (CFS) (MIN) (IN) (FPS) PLANE1 2.48 1.67 .72 28.00 35.22 724.95 5.84 .70			S	.0050	SLOPE CHANNEL P	OUCUMERS (OFFETOTEN	m			
SHAPE TRAP CHANNEL SHAPE WD 10.00 BOTTOM WIDTH OR DIAMETER Z 2.00 SIDE SLOPE RUPSTQ YES ROUTE UPSTREAM HYDROGRAPH *** COMPUTED MUSKINGUM-CUNGE PARAMETERS COMPUTATION TIME STEP ELEMENT ALPHA M DT DX PEAK TIME TO VOLUME MAXIMUM CELERITY (MIN) (FT) (CFS) (MIN) (IN) (FPS) PLANE1 2.48 1.67 .72 28.00 35.22 724.95 5.84 .70			CA	.030	CONTRIBUT	TNG AREA	OFFLUTEN	1			
WD 10.00 BOTTOM WIDTH OR DIAMETER Z 2.00 SIDE SLOPE RUPSTQ YES ROUTE UPSTREAM HYDROGRAPH *** COMPUTED MUSKINGUM-CUNGE PARAMETERS COMPUTATION TIME STEP ELEMENT ALPHA M DT DX PEAK TIME TO VOLUME MAXIMUM CELERITY (MIN) (FT) (CFS) (MIN) (IN) (FPS) PLANE1 2.48 1.67 .72 28.00 35.22 724.95 5.84 .70 PLANE1 2.48 1.67 .72 28.00 35.22 726.95 5.84 .70			SHAPE	TRAP	CHANNEL S	HAPE					
Z 2.00 SIDE SLOPE RUPSTQ YES ROUTE UPSTREAM HYDROGRAPH *** COMPUTED MUSKINGUM-CUNGE PARAMETERS COMPUTATION TIME STEP ELEMENT ALPHA M DT DX PEAK TIME TO VOLUME MAXIMUM CELERITY (MIN) (FT) (CFS) (MIN) (IN) (FPS) PLANE1 2.48 1.67 .72 28.00 35.22 724.95 5.84 .70 PLANE1 2.48 1.67 .72 28.00 35.22 726.95 5.84 .70			WD	10.00	BOTTOM WI	DTH OR DIA	AMETER				
KOFSIQ TES KOULE OFSIKEAW HIDROKERN *** COMPUTED MUSKINGUM-CUNGE PARAMETERS COMPUTATION TIME STEP ELEMENT ALPHA M DT DX PEAK TIME TO VOLUME MAXIMUM MIN DT DX PEAK CELERITY (MIN) (FT) (CFS) (MIN) (IN) (FPS) PLANE1 2.48 1.67 .72 28.00 35.22 724.95 5.84 .70			Z	2.00	SIDE SLOP	E EDEAM UVDI					
*** COMPUTED MUSKINGUM-CUNGE PARAMETERS COMPUTATION TIME STEP ELEMENT ALPHA M DT DX PEAK TIME TO VOLUME MAXIMUM PEAK CELERITY (MIN) (FT) (CFS) (MIN) (IN) (FPS) PLANE1 2.48 1.67 .72 28.00 35.22 724.95 5.84 .70 NDVN 07 1.44 1.51 64.00 30.33 736.51 5.62 710			RUPSTQ	IES	ROUTE UPS	TREAM HIDP	KOGRAPH				
COMPUTED RUSAINGUPCONGE FARMETERS COMPUTATION TIME STEP ELEMENT ALPHA M DT DX PEAK TIME TO VOLUME MAXIMUM PEAK CELERITY (MIN) (FT) (CFS) (MIN) (IN) (FPS) PLANE1 2.48 1.67 .72 28.00 35.22 724.95 5.84 .70 PLANE1 2.48 1.67 .72 28.00 35.22 724.95 5.84 .70				COMPT	TED MICKEN	CIM-CINCE	***	0			
ELEMENT ALPHA M DT DX PEAK TIME TO VOLUME MAXIMUM PEAK CELERITY (MIN) (FT) (CFS) (MIN) (IN) (FPS) PLANE1 2.48 1.67 .72 28.00 35.22 724.95 5.84 .70				COMPU	COMPUTAT	ION TIME S	FARAMETER STEP	5			
PEAK CELERITY (MIN) (FT) (CFS) (MIN) (IN) (FPS) PLANE1 2.48 1.67 .72 28.00 35.22 724.95 5.84 .70			ELEMENT AL	PHA	M	DT	DX	PEAK	TIME TO	VOLUME	MAXIMUM
(MIN) (FT) (CFS) (MIN) (IN) (FPS)						(((070)	PEAK	(=) =)	CELERITY
PLANEI 2.48 1.67 .72 28.00 35.22 724.95 5.84 .70						(MIN)	(FT)	(CFS)	(MIN)	(IN)	(FPS)
			PLANE1	2.48	1.67	.72	28.00	35.22	724.95	5.84	.70

CONTINUITY SUMMARY (AC-FT) - INFLOW= .2537E+02 EXCESS= .2210E+01 OUTFLOW= .2754E+02 BASIN STORAGE=-.2918E+00 PERCENT ERROR= 1.2

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

		MAI	N	.97	1.44	5.00		297	.78	725.00	5.70
	* * *		* * *	***		* * *		* * *			
			HYDRO	GRAPH AT ST	ATION	CHC					
	TOTAL	RAINFALL =	7.25,	TOTAL LOSS :	= 1.41,	TOTAL	EXCESS =	5.84			
	PEAK FLOW	TIME		6-HR	MAXIMU 24-	JM AVERA -HR	GE FLOW 72-HR	59.92	-HR		
+	(CFS)	(HR)	(CES	`							
+	298.	12.08	(INCHES (AC-FT	45. 4.570 22.	5.0 2	14. 675 27.	6. 5.697 28.	5.	6. 697 28.		
			CUMULA	TIVE AREA =	.09 8	SQ MI					

H-2-43

*** ***

	SUBBASIN	RUNOFF	DATA									
164 BA	SUBBAS	IN CHARA TAREA	CTERISTI .0	cs 1 subbas	IN AREA							
	PRECIP	ITATION	DATA									
10 PH				DEPTHS F	OR 0-P	ERCENT	HYPOTHET	ICAL STO	RM			
		HYDRO-35				TP-40				TP-	-49	
	5-MIN	15-MIN	60-MIN	2-HR	3-HR	6-HR	12-HR	24-HR	2-DAY	4-DAY	7-DAY	10-DAY
	.71	1.54	3.20	3.88	4.30	5.28	6.25	7.25	.00	.00	.00	.00
					STORM	AREA =	.01					

VARIABLES 3 PRINT CONTROL 0 PLOT CONTROL 0. HYDROGRAPH PLOT SCALE 7 PUNCH COMPUTED HYDROGRAPH 21 SAVE HYDROGRAPH ON THIS UNIT 1 FIRST ORDINATE PUNCHED OR SAVED 720 LAST ORDINATE PUNCHED OR SAVED .083 TIME INTERVAL IN HOURS

*** ***

	10 PH	5-MIN .71	HYDRO-35 . 15-MIN 6 1.54	DEP 0-MIN 2 3.20 3	THS FOR 0-1 -HR 3-HR .88 4.30	PERCENT HYP TP-40 6-HR 1 5.28	2-HR 6.25	CAL STOR 24-HR 7.25	M 2-DAY .00	TP- 4-DAY .00	49 7-DAY .00	10-DAY .00
					STORM	AREA =	.00					
	159 LS	SCS LO C	SS RATE STRTL RVNBR RTIMP	.53 I 79.00 C .00 P	NITIAL ABSTRA URVE NUMBER ERCENT IMPERV	ACTION /IOUS AREA						
	160 UD	SCS DI	MENSIONLES TLAG	S UNITGRAP .57 L	H AG							
						**	*					
					36	UNIT HYI	ROGRAP	H				
		0	0	0	0	1	1	1	1		1	1
		1	0.	0.	0.	 0	1. 0	÷.	1.		0	
		0	0.	0.	0	0	0	0.	0.		0	0.
		0.	0.	0.	0.	0.	0.	۰.	0.		••	۰.
	* * *	*	**	***	**	**	**	*				
			UVDDOCDA		TON C1							
			III DROGRA	EII AI SIAI	101 31							
	TOTAL RA	AINFALL =	7.25, TOT	AL LOSS =	2.44, TOT#	AL EXCESS =	4.	81				
	PEAK FLOW	TIME		6-HR	MAXIMUM AVE 24-HR	ERAGE FLOW 72-HR	59	.92-HR				
+	(CFS)	(HR)	(CES)									
+	2.	12.67	(CF3)	0.	0.	0.		0.				
			(INCHES)	3.916	4.814	4.814		4.814				
			(AC-FT)	Ο.	0.	0.		0.				
			CUMULATIV	E AREA =	.00 SQ MI							

* * * S1 * * *

158 BA

155 KK

AREA S1

TIMINT SUBBASIN RUNOFF DATA

PRECIPITATION DATA

***** * * * P7 * * *

SCS LOSS RATE

POND 7

OUTPUT CONTROL VARIABLES IPRNT 3 IPLOT 0 QSCAL 0. IPNCH 7 IOUT 21 ISAV1 1 ISAV2 720 TIMINT .083

161 KK

163 KO

165 LS

157	KO	OUTPUT CONTROL	VARIABLES	
		IPRNT	3	PRINT CONTROL
		IPLOT	0	PLOT CONTROL
		QSCAL	0.	HYDROGRAPH PLOT SCALE
		IPNCH	7	PUNCH COMPUTED HYDROGRAPH
		IOUT	21	SAVE HYDROGRAPH ON THIS UNIT
		ISAV1	1	FIRST ORDINATE PUNCHED OR SAVED
		ISAV2	720	LAST ORDINATE PUNCHED OR SAVED
		TIMINT	.083	TIME INTERVAL IN HOURS

SUBBASIN CHARACTERISTICS TAREA .00 SUBBASIN AREA

	STRTL CRVNBR RTIMP	.00 100.00 .00	INITIAL ABSTRACTION CURVE NUMBER PERCENT IMPERVIOUS AREA					
166 UD	SCS DIMENSIONLESS	S UNITGRA	PH					
	TEAG	.00	***					
			UNIT HYDR	OGRAPH				
	51. 14.	з.	5 END-OF-PERI 1. 0.	OD ORDINATES				
* * *	* * *	* * *	* * *	***				
	HYDROGRAI	PH AT STA	TION P7					
TOTAL F	AINFALL = 7.25, TOTA	AL LOSS =	.00, TOTAL EXCESS =	7.25				
PEAK FLOW	TIME		MAXIMUM AVERAGE FLOW					
+ (CFS)	(HR)	6-HR	24-HR 72-HR	59.92-HR				
+ 44.	(CFS)	5.	2. 1.	1.				
	(INCHES) (AC-FT)	5.278 3.	7.247 7.250 3. 3.	7.250 3.				
	CUMULATIV	E AREA =	.01 SQ MI					
*** *** ***	* *** *** *** *** ***	** *** **	* *** *** *** *** ***	* * * * * * * * * * * *	* *** ***	*** *** ***	*** *** *** *** *	** *** ***

167 KK	* C/3 * * *							

168 KO	OUTPUT CONTROL VA IPRNT	ARIABLES 3	PRINT CONTROL					
	IPLOT QSCAL	0.	PLOT CONTROL HYDROGRAPH PLOT SCALE					
	IPNCH IOUT	7 21	PUNCH COMPUTED HYDROGRAPH SAVE HYDROGRAPH ON THIS U	I INIT				
	ISAV1 ISAV2	1 720	FIRST ORDINATE PUNCHED OR LAST ORDINATE PUNCHED OR	SAVED SAVED				
	TIMINT	.083	TIME INTERVAL IN HOURS					
169 HC	HYDROGRAPH COMBIN	NATION						
	ICOMP	3	NUMBER OF HYDROGRAPHS TO	COMBINE				

* * *	* * *	* * *	* * *	***				
	HYDROGRAI	PH AT STA	TION C/3					
PEAK FLOW	TIME	6-HR	MAXIMUM AVERAGE FLOW 24-HR 72-HR	59.92-HR				
+ (CFS)	(HR) (CFS)							
+ 343.	12.08 (INCHES)	50. 4.620	16. 6. 5.804 5.825	6. 5.825				
	(AC-FT)	25.	31. 31.	31.				
	CUMULATIVE	E AREA =	.10 SQ MI					
*** *** ***	* *** *** *** *** ***	** *** **	* *** *** *** *** ***	* * * * * * * * * * * *	* * * * * * * *	*** *** ***	*** *** *** *** *	** *** ***
	* * * * * * * * * * * * *							
170 KK	* R/P7 * * *							
	**************************************	HROUGH 36	INCH RCP					
172 KO	OUTPUT CONTROL VA	ARTABLES						
	IPRNT TPLOT	3	PRINT CONTROL					
	QSCAL	0.7	HYDROGRAPH PLOT SCALE	r.				
	IOUT	21	SAVE HYDROGRAPH ON THIS U	INIT CAVED				
	ISAV2	720	LAST ORDINATE PUNCHED OR	SAVED				
	1 TMTN 1	.000	TINE INTERVAL IN BOOKS					
	HYDROGRAPH ROUTING	DATA						
173 RS								
	STORAGE ROUTING	1	NUMBER OF SUBPRACUES					
	STORAGE ROUTING NSTPS ITYP POUNTC	1 ELEV	NUMBER OF SUBREACHES TYPE OF INITIAL CONDITION	I				
	STORAGE ROUTING NSTPS ITYP RSVRIC X	1 ELEV 605.00 .00 W	NUMBER OF SUBREACHES TYPE OF INITIAL CONDITION INITIAL CONDITION ORKING R AND D COEFFICIEN	I				
174 SA	STORAGE ROUTING NSTPS ITYP RSVRIC X AREA	1 ELEV 605.00 .00 W .0	NUMBER OF SUBREACHES TYPE OF INITIAL CONDITION INITIAL CONDITION ORKING R AND D COEFFICIEN 2.3 2.5 3.	1 1T 0 3.6	3.8	4.4		

175 SE	ELEV	/ATION	605.00	606.00	608.00	612.00	616.00	618.00	622.00			
176 SS	SPII	CREL SPWID	621.00 189.00 2.60	SPILLWAY SPILLWAY	CREST ELE WIDTH	VATION						
		EXPW	1.50	EXPONENT	OF HEAD							

	STOPACE	0.0	75	5 52	MPUTED ST	ORAGE-ELEV	ATION DATA	53 53				
	ELEVATION	605.00	606.00	608.00	612.00	616.00	618.00	622.00				
				cc	MPUTED OU	TFLOW-ELEV	ATION DATA					
	OUTFLOW ELEVATION	.00 605.00	.00 621.00	.09 621.00	.67 621.01	2.27 621.03	5.39 621.05	10.53 621.08	18.19 621.11	28.90 621.15	43.13 621.20	
	OUTFLOW ELEVATION	61.42 621.25	84.26 621.31	112.16 621.37	145.61 621.44	185.12 621.52	231.20 621.60	284.38 621.69	345.11 621.79	413.96 621.89	491.40 622.00	
				COMPUI	ED STORAG	E-OUTFLOW-	ELEVATION I	DATA				
	STORAGE OUTFLOW ELEVATION	.00 .00 605.00	.75 .00 606.00	5.52 .00 608.00	16.57 .00 612.00	29.71 .00 616.00	37.10 .00 618.00	49.21 .00 621.00	49.26 .67 621.01	49.33 2.27 621.03	49.42 5.39 621.05	
	STORAGE OUTFLOW ELEVATION	49.54 10.53 621.08	49.68 18.19 621.11	49.85 28.90 621.15	50.05 43.13 621.20	50.27 61.42 621.25	50.53 84.26 621.31	50.80 112.16 621.37	51.11 145.61 621.44	51.44 185.12 621.52	51.80 231.20 621.60	
	STORAGE OUTFLOW ELEVATION	52.19 284.38 621.69	52.61 345.11 621.79	53.05 413.96 621.89	53.53 491.40 622.00							
***		* * *	***		* * *		***					
		HYDROG	RAPH AT ST	ATION	R/P7							
PEAK FLOW	I TIME		6-HR	MAXIMU 24-	M AVERAGE HR	FLOW 72-HR	59.92-HR					
+ (CFS)	(HR)	(CFS)	0		0	0	0					
- 0.	.00	(INCHES) (AC-FT)	.000	.0	0.	.000	.000					
PEAK STORA	AGE TIME		C 110	MAXIMUN	AVERAGE	STORAGE	50.00 VID					
+ (AC-FT) 31.	(HR) 37.08		о-нк 31.	. 24-	нк 1.	25.	25.					
PEAK STAG	GE TIME			MAXIMU	IM AVERAGE	STAGE						
+ (FEET) 616.43	(HR) 44.75		6-HR	. 24- 616.	HR 42 6	72-HR 14.19	59.92-HR 614.19					
		CUMULAT	IVE AREA =	.10 \$	I DI							
*** *** **	* * * * * * *	*** *** ***	*** *** *	** *** ***	*** ***	*** *** **	* *** *** *	*** *** ***	* *** *** **	** *** ***	*** *** ***	*** *
	*******	* * * * *										
177 KK	*	°2 *										
	* *******	*										
179 KO	OUTE	OFFSI	VARIABLES									
175 10	0011	IPRNT IPROT	3	PRINT CON	TROL							
		QSCAL	0.	HYDROGRAE	PH PLOT SC.	ALE						
		IOUT	21	SAVE HYDR	OGRAPH ON	THIS UNIT	,					
		ISAV1 ISAV2	1 720	FIRST ORD LAST ORDI	NATE PUNC	CHED OR SA HED OR SAV	VED ED					
		TIMINT	.083	TIME INTE	RVAL IN H	OURS						
	SUBBAS	SIN RUNOFF	DATA									
180 BA	SUBE	BASIN CHARA TAREA	CTERISTICS .11	SUBBASIN	AREA							
	PREC	CIPITATION	DATA									
10 PH		UVDDO 35	D	EPTHS FOR	0-PERCE	NT HYPOTHE	TICAL STORM	1	mp_40			
	5-MI	IN 15-MIN 1 1.54	60-MIN 3.20	2-HR 3	-HR 6-	HR 12-HF 28 6.25	24-HR 7.25	2-DAY 4-	-DAY 7-DAY	2 10-DAY		
		1.01	2.20		STORM ARE	A = .1	1					
181 LS	SCS	LOSS RATE										
		STRTL CRVNBR	.53 79.00	INITIAL A CURVE NUM	BER	N						

		RTIMP	.00 F	PERCENT IM	IPERVIOUS ARE	EA					
182 US	SNYDE	R UNITGRAPH									
		TP CP	.71 I .65 P	AG PEAKING CC	EFFICIENT						
	SYNTH	ETIC ACCUMUL#	ATED-AREA	VS. TIME	CURVE WILL	BE USED					
						* * *					
				CLARK SNYDER	UNIT HYDROO TC= .82 TP= .71	GRAPH PARA 2 HR, 1 HR,	R= . CP= .	60 HR 65			
					UNIT H	HYDROGRAPH					
	з.	10.	19.	30.	44 END-OF-1 42.	53.	61.	66.	68.	66.	
	59. 15.	52. 13.	45. 11.	39. 10.	34. 9.	30. 7.	26. 7.	23. 6.	20. 5.	17.	
	4.	3.	3.	2.	2.	2.	2.	1.	1.	1.	
		±•		± •							
~~~		~ ~ ~	~ ~ ~		~ ~ ~						
		HYDROGRAPH	H AT STAT	ION	02						
TOTAL R.	AINFALL =	7.25, TOTAI	LOSS =	2.44,	TOTAL EXCESS	5 = 4.8	1				
PEAK FLOW	TIME		6-HP	MAXIMUM 24-H	AVERAGE FLO	)W 1P 59	92-HP				
+ (CFS)	(HR)	(070)	0 1110	24 1			52 m				
+ 150.	12.75	(CFS)	47.	15		5.	6.				
		(INCHES) (AC-FT)	23.	4.79	0 4.79 . 29	90 9.	4.790 29.				
		CUMULATIVE	AREA =	.11 SQ	MI						
*** *** ***	*** *** **	* *** *** ***	* * * * * * *	* * * * * * *	*** *** ***	*** *** *	** *** *	** *** *** **	* *** **	* *** *** ***	* *** *** *** ***
	*	*									
183 KK	* S *	2 *									
	*******	**** AREA S2									
185 KO		T CONTROL VAL	TABLES								
105 110	00110	IPRNT	3 E	RINT CONT	ROL						
		QSCAL	0. H	YDROGRAPH	OL PLOT SCALE						
		IPNCH IOUT	7 E 21 S	ONCH COMP	UTED HYDROGH GRAPH ON THI	RAPH IS UNIT					
		ISAV1	1 F	TIRST ORDI	NATE PUNCHEI	OR SAVEL					
		TIMINT	.083 1	TIME INTER	VAL IN HOURS	OR SAVED					
	SUBBASI	N RUNOFF DATA	Ŧ								
186 BA	SUBBA	SIN CHARACTEF TAREA	NISTICS	SUBBASIN A	REA						
	PRECT	DITATION DATE	4								
10	INDOI	IIINIION DAIF			0 00000000	WDOMUDETC					
IO PH		HYDRO-35		· · · · · · · · · · · · · · · · · · ·	TP-40			T	P-49		
	5-MIN .71	15-MIN 60- 1.54 3	-MIN 2 3.20 3	2-HR 3- 3.88 4.	HR 6-HR 30 5.28	12-HR 6.25	24-HR 2 7.25	2-DAY 4-DAY .00 .00	.00	10-DAY .00	
				S	TORM AREA =	.08					
187 LS	SCS L	OSS RATE									
		STRTL	.53 I	NITIAL AB	STRACTION						
		RTIMP	.00 F	PERCENT IM	IPERVIOUS ARE	EA					
188 US	SNYDE	R UNITGRAPH									
		TP CP	.76 I .66 E	LAG PEAKING CC	EFFICIENT						
	SYNTH	ETIC ACCUMULA	ATED-AREA	VS. TIME	CURVE WILL	BE USED					
						***					
					UNITE UNDOO		MEREDO				
				CLARK SNYDER	TC= .85 TP= .76	5 HR, 5 HR, 5 HR,	R= . CP= .	64 HR 66			
					UNIT H	HYDROGRAPH	TNATEC				
	2.	6.	12.	19.	27.	34.	40.	44.	46.	46.	
	42. 12.	37. 10.	33. 9.	29. 8.	25. 7.	22. 6.	19. 5.	17. 5.	15. 4.	13.	
	3. 1.	3. 1.	2. 1.	2. 1.	2. 1.	2. 0.	1.	1.	1.	1.	
* * *		***	***	-	***	***					
		HYDDOODADI	4 AT CTT	TON	\$2						
		ILL DRUGRAPH	I MI STAT	UIN	20						

	TOTAL RA	AINFALL =	7.25, TOT#	AL LOSS =	2.44, TOTAL	EXCESS =	4.81
	PEAK FLOW	TIME			MAXIMUM AVER	AGE FLOW	
				6-HR	24-HR	72-HR	59.92-HR
+	(CFS)	(HR)					
			(CFS)				
+	103.	12.83		34.	10.	4.	4.
			(INCHES)	3.883	4.791	4.791	4.791
			(AC-FT)	17.	21.	21.	21.
			CUMULATIVE	AREA =	.08 SQ MI		

*** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** ***

		* * * *	* * * * * * * * * *		
		*	*		
189	KK	*	DP2 *		
		*	*		
		****	* * * * * * * * *		
190	KO		OUTPUT CONTROL	VARIABLES	
			IPRNT	3	PRINT CONTROL
			IPLOT	0	PLOT CONTROL
			QSCAL	0.	HYDROGRAPH PLOT SCALE
			IPNCH	7	PUNCH COMPUTED HYDROGRAPH
			IOUT	21	SAVE HYDROGRAPH ON THIS UNIT
			ISAV1	1	FIRST ORDINATE PUNCHED OR SAVED
			ISAV2	720	LAST ORDINATE PUNCHED OR SAVED
			TIMINT	.083	TIME INTERVAL IN HOURS

191	HC	HYDROGRAPH ICOMH	COMBINATION	1 3	NUMBER	OF	HYDROGRAPHS	то	COMBINE
								* * *	ć
	* * *	* * *	*	**			* * *		***

HYDROGRAPH AT STATION DP2

	PEAK FLOW	TIME			MAXIMUM AVER	RAGE FLOW	
+	(CFS)	(HR)	(CES)	6-HR	24-HR	72-HR	59.92-HR
+	253.	12.75	(INCHES)	81. 2.556	25. 3.152	10. 3.152	10. 3.152
			(AC-FT)	40.	50.	50.	50.

CUMULATIVE AREA = .29 SQ MI

*** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** ***

		* * * *	******	* *				
		*		*				
192	KK	*	S2/DP2	*				
		*		*				
		* * * *	* * * * * * * * *	* *				
				ROUTE	THROUGH	12	INCH	CMP

194	KO	OUTPUT CONTROL	VARIABLES	
		IPRNT	3	PRINT CONTROL
		IPLOT	0	PLOT CONTROL
		QSCAL	0.	HYDROGRAPH PLOT SCALE
		IPNCH	7	PUNCH COMPUTED HYDROGRAPH
		IOUT	21	SAVE HYDROGRAPH ON THIS UNIT
		ISAV1	1	FIRST ORDINATE PUNCHED OR SAVED
		ISAV2	720	LAST ORDINATE PUNCHED OR SAVED
		TIMINT	.083	TIME INTERVAL IN HOURS

#### HYDROGRAPH ROUTING DATA

195	RS	STORAGE ROUTIN	G				
		NSTPS	1	NUMBER OF	SUBREACH	ES	
		ITYP	ELEV	TYPE OF I	NITIAL CON	NDITION	
		RSVRIC	617.90	INITIAL C	ONDITION		
		Х	.00	WORKING R	AND D COEI	FFICIENT	
196	SA	AREA	.0	.1	1.6	3.3	5.8
197	SE	ELEVATION	617.90	620.00	622.00	624.00	626.00
199	SL	LOW-LEVEL OUTL	ET				
		ELEVL	618.30	ELEVATION	I AT CENTER	R OF OUTLET	2
		CAREA	.79	CROSS-SEC	TIONAL ARE	EA	
		COQL	.80	COEFFICIE	INT		
		EXPL	.50	EXPONENT	OF HEAD		
198	SS	SPILLWAY					
		CREL	622.00	SPILLWAY	CREST ELEV	/ATION	
		SPWID	746.00	SPILLWAY	WIDTH		
		COQW	2.60	WEIR COEF	FICIENT		
		EXPW	1.50	EXPONENT	OF HEAD		

* * *

COMPUTED S	TORAGE-ELEVATION	DATA
------------	------------------	------

STORAGE ELEVATION	.00 617.90	.08 620.00	1.53 622.00	6.36 624.00	15.40 626.00					
			CO	MPUTED OUT	FLOW-ELEVA	FION DATA				
OUTFLOW	.00	.00	3.57	3.92	4.36	4.89	5.59	6.50	7.78	9.69
ELEVATION	617.90	618.30	618.80	618.91	619.05	619.24	619.53	619.97	620.69	622.00
OUTFLOW	25.35	134.35	429.72	1004.60	1951.76	3364.55	5335.83	7958.61	11325.92	15530.78
ELEVATION	622.04	622.16	622.36	622.64	623.00	623.44	623.96	624.56	625.24	626.00
			COMPUTI	ED STORAGE	-OUTFLOW-E	LEVATION D	ATA			
STORAGE	.00	.00	.01	.01	.01	.02	.04	.08	.08	.27
OUTFLOW	.00	.00	3.57	3.92	4.36	4.89	5.59	6.50	6.57	7.78
ELEVATION	617.90	618.30	618.80	618.91	619.05	619.24	619.53	619.97	620.00	620.69
STORAGE	1.53	1.60	1.80	2.16	2.71	3.52	4.65	6.23	6.36	8.39
OUTFLOW	9.69	25.35	134.35	429.72	1004.60	1951.76	3364.55	5335.83	5498.04	7958.61
ELEVATION	622.00	622.04	622.16	622.36	622.64	623.00	623.44	623.96	624.00	624.56
STORAGE	11.36	15.40								
OUTFLOW	11325.92	15530.78								
ELEVATION	625.24	626.00								

* * *

*** WARNING *** MODIFIED PULS ROUTING MAY BE NUMERICALLY UNSTABLE FOR OUTFLOWS BETWEEN 0. TO 15531. THE ROUTED HYDROGRAPH SHOULD BE EXAMINED FOR OSCILLATIONS OR OUTFLOWS GREATER THAN PEAK INFLOWS. THIS CAN BE CORRECTED BY DECREASING THE TIME INTERVAL OR INCREASING STORAGE (USE A LONGER REACH.)

# *** *** ***

			HYDROGRAPH	H AT STAT	ION S2/DP2		
	PEAK FLOW	TIME		6-HP	MAXIMUM AVER	AGE FLOW	59 92-HP
+	(CFS)	(HR)	(CFS)	0		, <u> </u>	00102
+	253.	12.83	(INCHES) (AC-FT)	80. 2.530 40.	25. 3.152 50.	10. 3.152 50.	10. 3.152 50.
1	PEAK STORAGE	TIME		6-HR	MAXIMUM AVERA	GE STORAGE 72-HR	59.92-HR
+	(AC-FT) 2.	(HR) 12.75		2.	1.	0.	0.
	PEAK STAGE	TIME		6-HR	MAXIMUM AVERA 24-HR	AGE STAGE 72-HR	59.92-HR
+	(FEET) 622.24	(HR) 12.83		622.09	620.79	619.26	619.26
			CUMULATIVE	AREA =	.29 SQ MI		

---- --- --- --- --- --- --- --- --- --- --- --- --- --- --- --- --- --- --- --- --- --- --- --- --- --- --- --- --- --- ---

		*****
		* *
200	KK	* LF6 *
		* *
		*******
		SUBAREA LF6
202	ко	OUTPUT CONTROL VARIABLES         IPRNT       3       PRINT CONTROL         IPLOT       0       PLOT CONTROL         QSCAL       0.       HVDROGRAPH PLOT SCALE         IPNCH       7       PUNCH COMPUTED HYDROGRAPH         IOUT       21       SAVE HYDROGRAPH ON THIS UNIT         ISAV1       1       FIRST ORDINATE PUNCHED OR SAVED         ISAV2       720       LAST ORDINATE PUNCHED OR SAVED         TIMINT       .083       TIME INTERVAL IN HOURS
		SUBBASIN RUNOFF DATA
203	BA	SUBBASIN CHARACTERISTICS TAREA .02 SUBBASIN AREA
		PRECIPITATION DATA
10	PH	DEPTHS FOR 0-PERCENT HYPOTHETICAL STORM
		HYDRO-35 TP-40 TP-49
		5-MIN 15-MIN 60-MIN 2-HR 3-HR 6-HR 12-HR 24-HR 2-DAY 4-DAY 7-DAY 10-DAY .71 1.54 3.20 3.88 4.30 5.28 6.25 7.25 .00 .00 .00 .00
		STORM AREA = .02
204	LS	SCS LOSS RATE STRTL .27 INITIAL ABSTRACTION CRVNBR 88.00 CURVE NUMBER RTIMP .00 PERCENT IMPERVIOUS AREA
205	UK	KINEMATIC WAVE OVERLAND-FLOW ELEMENT NO. 1 L 120. OVERLAND FLOW LENGTH S .2500 SLOPE

	N	.300	ROUGHNES	SS COEFFIC	IENT							
	PA	100.0	PERCENT	OF SUBBAS	IN							
	DXMIN	5	MINIMUM	NUMBER OF	DX INTERVA	ALS						
	MUSKINGUM-CUN	GE										
206 RD	MAIN CHANNE:	L										
	L	1554.	CHANNEL	LENGTH								
	S	.0100	SLOPE									
	N	.030	CHANNEL	ROUGHNESS	COEFFICIEN	1T						
	CA	.02	CONTRIBU	JTING AREA								
	SHAPE	TRAP	CHANNEL	SHAPE								
	WD	.00	BOTTOM N	VIDTH OR D	IAMETER							
	Z	2.00	SIDE SLO	DPE								
	RUPSTQ	NO	ROUTE UI	STREAM HY	DROGRAPH							
	***											
	COMPUTED MUSKINGUM-CUNGE PARAMETERS											
			COMPUTA	ATION TIME	STEP							
	ELEMENT	ALPHA	М	DT	DX	PEAK	TIME TO	VOLUME	MAXIMUM			
							PEAK		CELERITY			
				(MIN)	(FT)	(CFS)	(MIN)	(IN)	(FPS)			
	PLANE1	2.48	1.67	.76	24.00	106.29	724.75	5.84	.67			
	MAIN	2.31	1.33	3.85	///.00	99.08	/23.41	5.63	6./3			

CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .6630E+01 OUTFLOW= .6390E+01 BASIN STORAGE= .6431E-03 PERCENT ERROR= 3.6

				II	TERPOLATED TO	) SPECIFIED	COMPUTATION	INTERVAL				
		MA	IN	2.31 1		00	91.97	725.00	5.61			
	* * *		* * *	* * *	***	÷	***					
			HYDROGR#	APH AT STAT	ION LF6							
	TOTAL RA	INFALL =	7.25, TOI	AL LOSS =	1.41, TOTAI	L EXCESS =	5.84					
	PEAK FLOW	TIME		6-UD	MAXIMUM AVE	RAGE FLOW	50 02_UD					
+	(CFS)	(HR)	(CES)	0-11K	24-11	72-116	59.92-nk					
+	92.	12.08	(TNOURC)	10.	3.	1.	1.					
			(AC-FT)	4.373	6.	6.	5.011					
			CUMULATIV	/E AREA =	.02 SQ MI							
		*** *** *			*** *** ***			*** *** ***		*** *** *	 	

	******
	* *
207 KK	* 04 *
	· · · · · · · · · · · · · · · · · · ·
	AREA 04
209 KO	OUTPUT CONTROL VARIABLES IRRNT 3 PRINT CONTROL IPLOT 0 PLOT CONTROL QSCAL 0. HYDROGRAPH PLOT SCALE IPNCH 7 PUNCH COMPUTED HYDROGRAPH IOUT 21 SAVE HYDROGRAPH ON THIS UNIT ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED ISAV2 720 LAST ORDINATE PUNCHED OR SAVED TIMINT .083 TIME INTERVAL IN HOURS
	SUBBASIN RUNOFF DATA
210 BA	SUBBASIN CHARACTERISTICS TAREA .01 SUBBASIN AREA
	PRECIPITATION DATA
10 PH	DEPTHS FOR         0-PERCENT HYPOTHETICAL STORM            TP-40            5-MIN         15-MIN         0-MIN           2-HR         3-HR         6-HR            71         1.54           3.88         4.30         5.28           6.25         7.25         .00         .00
	STORM AREA = .01
211 LS	SCS LOSS RATE STRTL .53 INITIAL ABSTRACTION CRVNBR 79.00 CURVE NUMBER RTIMP .00 PERCENT IMPERVIOUS AREA
212 US	SNYDER UNITGRAPH TP .30 LAG CP .70 PEAKING COEFFICIENT
	SYNTHETIC ACCUMULATED-AREA VS. TIME CURVE WILL BE USED
	***
	UNIT HYDROGRAPH PARAMETERS CLARK TC= .36 HR, R= .22 HR SNYDER TP= .30 HR, CP= .69

					UNIT HYDR	OGRAPH				
	2.	8.	15.	17 E 18. 1	IND-OF-PERI	OD ORDINATES 1. 8.	5.	4.	2.	
***	2.	1.	1.	1.	0.	0. 0. ***				
		HYDROGRAP	H AT STA	TION 04						
TOTAL R	AINFALL =	7.25, TOTA	L LOSS =	2.44, TOTAL	EXCESS =	4.81				
PEAK FLOW	TIME	, .		MAXIMUM AVER	RAGE FLOW					
+ (CFS)	(HR)		6-HR	24-HR	72-HR	59.92-HR				
+ 28.	12.33	(CFS)	5.	2.	1.	1.				
		(INCHES) (AC-FT)	3.907 3.	4.795 3.	4.795 3.	4.795 3.				
		CUMULATIVE	AREA =	.01 SQ MI						
*** *** ***	*** *** **					*** *** ***				
	*********	****								
213 KK	* C/	2 *								
	******	* * * *								
214 KO	OUTPU	T CONTROL VA	RIABLES	CONTROL						
		IPLOT	0	PLOT CONTROL						
		IPNCH	7	PUNCH COMPUTED	HYDROGRAPH					
		IOUT ISAV1	21	SAVE HYDROGRAPH FIRST ORDINATE	I ON THIS U PUNCHED OR	SAVED				
		ISAV2 TIMINT	720 .083	LAST ORDINATE F FIME INTERVAL I	PUNCHED OR	SAVED				
215 HC	HYDRO	GRAPH COMBIN ICOMP	ATION 2 1	NUMBER OF HYDRC	GRAPHS TO	COMBINE				
					***					
***		***	***	***	÷	***				
		HYDROGRAP	H AT STA	TION C/2						
PEAK FLOW	TIME		6 U.D.	MAXIMUM AVER	RAGE FLOW	50.00.00				
+ (CFS)	(HR)	(070)	6-HR	24-HR	/2-HR	59.92-HR				
+ 106.	12.08	(CFS)	16.	5.	2.	2.				
		(AC-FT)	4.317 8.	10.	5.311	5.311				
		CUMULATIVE	AREA =	.03 SQ MI						
*** *** ***	*** *** **	* *** *** **	* *** **	* *** *** *** *	*** *** ***	*** *** ***	*** *** *** :	*** *** *** *	** *** *** *** *	** *** ***
	*********	****								
216 KK	* CH	D * *								
	*****	**** SUBAREA	CHD							
218 KO	OUTPU	T CONTROL VA	RTABLES							
		IPRNT	3	PRINT CONTROL						
		QSCAL	0.1	HYDROGRAPH PLOT	SCALE					
		IOUT	21	PUNCH COMPUTED SAVE HYDROGRAPH	HYDROGRAPH I ON THIS U	NIT				
		ISAV1 ISAV2	1 720	FIRST ORDINATE LAST ORDINATE P	PUNCHED OR PUNCHED OR	SAVED SAVED				
		TIMINT	.083	TIME INTERVAL I	IN HOURS					
	SUBBASI	N RUNOFF DAT	A							
219 BA	SUBBA	SIN CHARACTE	RISTICS	CURRACIN AREA						
	PRECT	PITATION DAT	.uz :	JUDDADIN ARLA						
10 PH	INDEL	IIINIION DAI	DE	PTHS FOR 0-PF	RCENT HYPO	THETICAL STOR	м			
10 111	5-MTM	HYDRO-35	-MIN		TP-40		2-DAY 4-D	TP-49	0-DAY	
	.71	1.54	3.20	3.88 4.30	5.28 6	.25 7.25	.00 .0	00.00	.00	
				STORM	AREA =	.02				
220 LS	SCS L	OSS RATE	07	INTETNI ADOEDO	TON .					
		CRVNBR	.2/	CURVE NUMBER						

		RTIMP	.00	PERCENT IMPERVIOUS AREA
		WINDWART O WAND		
		KINEMATIC WAVE		
221	UK	OVERLAND-FLOW	ELEMENT	NO. 1
		L	139.	OVERLAND FLOW LENGTH
		S	.2500	SLOPE
		N	.300	ROUGHNESS COEFFICIENT
		PA	100.0	PERCENT OF SUBBASIN
		DXMIN	5	MINIMUM NUMBER OF DX INTERVALS
		MUSKINGUM-CUNGE		
222	RD	MAIN CHANNEL		
		L	2806.	CHANNEL LENGTH
		S	.0100	SLOPE
		N	.030	CHANNEL ROUGHNESS COEFFICIENT
		CA	.02	CONTRIBUTING AREA
		SHAPE	TRAP	CHANNEL SHAPE
		WD	10.00	BOTTOM WIDTH OR DIAMETER
		Z	3.00	SIDE SLOPE
		RUPSTQ	YES	ROUTE UPSTREAM HYDROGRAPH
				* * *

*** FDKRUT - NEWTON RAPHSON FAILEDFIXED POINT ITERATION USED - ITERATION= 1

	COMP	UTED MUSK COMPUT	INGUM-CUNGE ATION TIME	PARAMETER	RS			
ELEMENT	ALPHA	М	DT	DX	PEAK	TIME TO	VOLUME	MAXIMUM
			(MIN)	(FT)	(CFS)	PEAK (MIN)	(IN)	CELERITY (FPS)
PLANE1 MAIN	2.48 1.38	1.67 1.42	.71 5.00	27.80 935.33	104.84 168.74	725.08 730.00	5.84 5.48	.70 6.99

CONTINUITY SUMMARY (AC-FT) - INFLOW= .9545E+01 EXCESS= .6630E+01 OUTFLOW= .1608E+02 BASIN STORAGE= .2067E-02 PERCENT ERROR= .6

### INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

		MAI	N	1.38	1.42	5.00	)	168.74	730.00	5.48
	* * *		* * *	***		* * *		* * *		
			HYDROGR.	APH AT STA	ATION	CHD				
	TOTAL RA	INFALL =	7.25, TO	FAL LOSS =	= 1.41	, TOTAL	EXCESS =	5.84		
	PEAK FLOW	TIME		6-HR	MAXIMU 24-	JM AVERA	AGE FLOW	59 92-HR		
+	(CFS)	(HR)	(CES)	0 1110			, <u> </u>	00.02 m		
+	169.	12.17	(INCHES) (AC-FT)	26. 4.448 13.	5.	8. 481 16.	3. 5.482 16.	3. 5.482 16.		
			CUMULATI	VE AREA =	.05 \$	SQ MI				

*** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** ***

		****
		* *
223	KK	* LF4 *
		* *
		*****
		SUBAREA LF4
225	KO	OUTPUT CONTROL VARIABLES
		IPRNT 3 PRINT CONTROL
		IPLOT 0 PLOT CONTROL
		QSCAL 0. HYDROGRAPH PLOT SCALE
		IPNCH 7 PUNCH COMPUTED HYDROGRAPH
		IOUT 21 SAVE HYDROGRAPH ON THIS UNIT
		ISAVI 1 FIRST ORDINATE PUNCHED OR SAVED
		ISAV2 720 LAST ORDINATE PUNCHED OR SAVED
		TIMINT .083 TIME INTERVAL IN HOURS
		SUBBASIN RUNOFF DATA
226	BA	SUBBASIN CHARACTERISTICS
220	211	TAREA .03 SUBBASIN AREA
		PRECIPITATION DATA
10	PH	DEPTHS FOR 0-PERCENT HYPOTHETICAL STORM
		HYDRO-35 TP-40 TP-40
		5-MIN 15-MIN 60-MIN 2-HR 3-HR 6-HR 12-HR 24-HR 2-DAY 4-DAY 7-DAY 10-DAY
		.71 1.54 3.20 3.88 4.30 5.28 6.25 7.25 .00 .00 .00 .00
		STORM AREA = .03
227	LS	SCS LOSS RATE
		STRTL .27 INITIAL ABSTRACTION
		CRVNBR 88.00 CURVE NUMBER

KINEMATIC WAVE

228	UK	OVERLAND-FLOW	ELEMENT	NO. 1						
		L	120.	OVERLANI	D FLOW LENG	TH				
		S	.2500	SLOPE						
		N	.300	ROUGHNES	SS COEFFICI	ENT				
		PA	100.0	PERCENT	OF SUBBASI	N				
		DXMIN	5	MINIMUM	NUMBER OF	DX INTERVA	ALS			
		MUSKINGUM-CUNGE								
229	RD	MAIN CHANNEL								
		L	1258.	CHANNEL	LENGTH					
		S	.0100	SLOPE						
		N	.030	CHANNEL	ROUGHNESS	COEFFICIEN	T			
		CA	.03	CONTRIBU	UTING AREA					
		SHAPE	TRAP	CHANNEL	SHAPE					
		WD	.00	BOTTOM V	WIDTH OR DI	AMETER				
		Z	2.00	SIDE SLO	OPE					
		RUPSTQ	NO	ROUTE UI	PSTREAM HYD	ROGRAPH				
						* * *				
			COMPI	TED MUSK	INGUM-CUNGE	PARAMETER	25			
			00112	COMPUT	ATTON TIME	STEP				
		ELEMENT A	I.PHA	M	DT	DX	PEAK	TIME TO	VOLUME	MAXIMUM
					51	211	1 2000	PEAK	VOLOTIL	CELEBITY
					(MIN)	(FT)	(CFS)	(MIN)	(IN)	(FPS)
		PLANE1	2.48	1.67	.76	24.00	166.15	724.75	5.84	.67
		MAIN	2.31	1.33	2.78	629.00	160.98	724.07	5.64	7.53

CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .1036E+02 OUTFLOW= .1002E+02 BASIN STORAGE= .5353E-03 PERCENT ERROR= 3.3

INTERPOLATED	TO	SPECIFIED	COMPUTATION	INTERVAL	

		MAI	N	2.31	1.33	5.00		152.01	725.00	5.64
	* * *		***	***		* * *		***		
			HYDROGRA	APH AT STA	TION	LF4				
	TOTAL RAI	INFALL =	7.25, TOI	AL LOSS =	1.41,	TOTAL EX	ICESS =	5.84		
	PEAK FLOW	TIME		6-HR	MAXIMU 24-1	M AVERAGE	FLOW	59.92-HB		
+	(CFS)	(HR)	(070)							
+	152.	12.08	(INCHES) (AC-FT)	16. 4.596 8.	5.6	5. 35 0.	2. 5.636 10.	2. 5.636 10.		
			CUMULATIV	/E AREA =	.03 S	2 MI				

*** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** ***

230 KK C/2 * 230 KK C/2 * 231 KO OUTPUT CONTROL VARIABLES IPENT 3 PRINT CONTROL IPLOT 0 PLOT CONTROL QSCAL 0. HYDROGRAPH PLOT SCALE IPNCH 7 PUNCH COMPUTED HYDROGRAPH IOUT 21 SAVE HYDROGRAPH ON THIS UNIT ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED ISAV2 720 LAST ORDINATE PUNCHE	2	30 KK 31 KO	********** * C * *	***** /2 * *				
230 KK C/2 + 231 KO OUTPUT CONTROL VARIABLES IPENT 3 PRINT CONTROL IPLOT 0 PLOT CONTROL IPLOT 0 PLOT CONTROL IPLOT 1 PLOT CONTROL IPLOT 0 PLOT CONTROL ON IPLOT 0 PLOT 0 PLOT CONTROL ON IPLOT 0 PLOT 0 PLOT CONTROL ON IPLOT 0 PLOT 0 PLO	2	30 KK 31 KO	* C * ****	/2 * *				
230 KK * C/2 * * * * * * * * * * * * * * * * * * *	2	30 KK 31 KO	* C * ****	*****				
231 KO 231 KO 231 KO 231 KO 231 KO 231 KO 231 KO 232 KD 232 KD	2	31 ко	* ******	*				
231 KO 231 KO OUTPUT CONTROL VARIABLES IPRNT IPLOT QSCAL O HYDROGRAPH PLOT CONTROL QSCAL IPLOT QSCAL O HYDROGRAPH PLOT SCALE IFNCH TOUT 21 SAVE HYDROGRAPH ON THIS UNIT ISAV1 IFIRST ORDINATE PUNCHED OR SAVED ISAV2 TIMINT .083 TIME INTERVAL IN HOURS 232 HC HYDROGRAPH COMBINATION ICOMP 2 NUMBER OF HYDROGRAPHS TO COMBINE *** *** *** *** *** *** *** *	2	31 KO	*******	* * * * *				
231 K0 UTPUT CONTROL VARIABLES IPRNT 3 PRINT CONTROL IPLOT 0 PLOT CONTROL QSCAL 0. HYDROGRAPH PLOT SCALE IPNCH 7 FUNCH COMPUTED HYDROGRAPH IOUT 21 SAVE HYDROGRAPH ON THIS UNIT ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED ISAV2 720 LAST ORDINATE PUNCHED OR SAVED TIMINT .083 TIME INTERVAL IN HOURS 232 HC HYDROGRAPH COMBINATION ICOMP 2 NUMBER OF HYDROGRAPHS TO COMBINE *** *** *** *** *** *** *** *	2	31 KO						
231 RO GOTFOT TOWNED VARIABLES IPRNT 3 PRINT CONTROL IPLOT 0 PLOT CONTROL IPLOT 0 PLOT CONTROL IPLOT 0 PLOT CONTROL IPNCH 7 PUNCHCOMPUTED HYDROGRAPH IOUT 21 SAVE HYDROGRAPH ON THIS UNIT ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED ISAV2 720 LAST ORDINATE PUNCHED OR SAVED ISAVE 1000000000000000000000000000000000000	2	JI NO		UT CONTROL U	ADTADTEC			
11100       0       PLOT CONTROL         QSCAL       0.       HYDROGRAPH PLOT SCALE         1FNCH       7       PUNCH COMPUTED HYDROGRAPH         10UT       21       SAVE HYDROGRAPH ON THIS UNIT         1SAV1       1       FIRST ORDINATE PUNCHED OR SAVED         1SAV2       720       LAST ORDINATE PUNCHED OR SAVED         1COMP       2       NUMBER OF HYDROGRAPHS TO COMBINE         ****       ****       ****         ****       ****       ****         HYDROGRAPH AT STATION       C/2         PEAK FLOW       TIME       MAXIMUM AVERAGE FLOW         (CFS)       (HR)       4.5			0011	TPRNT	2 SUCCESSION	PRINT CONTROL		
QSCAL 0. HYDROGRAPH PLOT SCALE IPNCH 7 FUNCH COMPUTED HYDROGRAPH IOUT 21 SAVE HYDROGRAPH ON THIS UNIT ISAVI 1 FIRST ORDINATE PUNCHED OR SAVED ISAV2 720 LAST ORDINATE PUNCHED OR SAVED TIMINT .083 TIME INTERVAL IN HOURS 232 HC HYDROGRAPH COMBINATION ICOMP 2 NUMBER OF HYDROGRAPHS TO COMBINE **** **** **** **** **** **** HYDROGRAPH AT STATION C/2 PEAK FLOW TIME MAXIMUM AVERAGE FLOW 6-HR 24-HR 72-HR 59.92-HR + (CFS) (HR) (CFS) + 317. 12.08 43. 13. 5. 5.				TPLOT	0	PLOT CONTROL		
IPACH     7     FUNCH COMPUTED HYDROGRAPH       IOUT     21     SAVE HYDROGRAPH ON THIS UNIT       ISAV2     720     LAST ORDINATE PUNCHED OR SAVED       ICOMP     2     NUMBER OF HYDROGRAPHS TO COMBINE       ****       ****       ****       ****       ****       ****       ****       ****       ****       ****       ****       ****       *****       ****				OSCAL	0	HYDROGRAPH PLOT	SCALE	
1 Iout     21 SAVE HYDROGRAPH ON THIS UNIT       ISAVE     1 FIRST ORDINATE PUNCHED OR SAVED       ISAVE     720       ISAVE     74				TPNCH		PUNCH COMPUTED	AADBUCEFFE	
ISAVI 1 FIRST ORDINATE PUNCHED OR SAVED ISAV2 720 LAST ORDINATE PUNCHED OR SAVED TIMINT .083 TIME INTERVAL IN HOURS 232 HC HYDROGRAPH COMBINATION ICOMP 2 NUMBER OF HYDROGRAPHS TO COMBINE *** *** *** *** *** *** HYDROGRAPH AT STATION C/2 PEAK FLOW TIME MAXIMUM AVERAGE FLOW 6-HR 24-HR 72-HR 59.92-HR + (CFS) (HR) (CFS) + 317. 12.08 43. 13. 5. 5.				TOUT	21	SAVE HYDROGRAPH	ON THIS UN	TT
ISAV2       720       LAST ORDINATE PUNCHED OR SAVED         TIMINT       .083       TIME INTERVAL IN HOURS         232 HC       HYDROGRAPH COMBINATION       ICOMP         ICOMP       2       NUMBER OF HYDROGRAPHS TO COMBINE         ***         ***       ***         ***       ***         HYDROGRAPH AT STATION       C/2         PEAK FLOW       TIME         MAXIMUM AVERAGE FLOW         +       6-HR         24-HR       72-HR         59.92-HR         +       (CFS)         +       317.         12.08       43.         13.       5.         5.540				TSAV1	1	FIRST ORDINATE	PUNCHED OR	SAVED
TIMINT       .083       TIME INTERVAL IN HOURS         232 HC       HYDROGRAPH COMBINATION ICOMP       2       NUMBER OF HYDROGRAPHS TO COMBINE         ***         ***         ***         ***         ***         ***         MAXIMUM AVERAGE FLOW         PEAK FLOW       TIME         MAXIMUM AVERAGE FLOW         +         (CFS)         +         +         +         (INCHES)         +         +         +         +         +         +         +         +         +         +         +         +         +         +         +         +         +         +         +         +         <				ISAV2	720	LAST ORDINATE P	JNCHED OR S	SAVED
232 HC       HYDROGRAPH COMBINATION ICOMP       2 NUMBER OF HYDROGRAPHS TO COMBINE         ****       ****       ****         ****       ****       ****         ****       ****       ****         HYDROGRAPH AT STATION       C/2         PEAK FLOW       TIME       MAXIMUM AVERAGE FLOW 6-HR         +       (CFS)         +       317.       12.08         (INCHES)       4.502       5.539         5.540       5.540				TIMINT	.083	TIME INTERVAL II	N HOURS	
***         ***         ***         ***           HYDROGRAPH AT STATION         C/2           PEAK FLOW         TIME         MAXIMUM AVERAGE FLOW           6-HR         24-HR         72-HR           +         (CFS)         (HR)           (CFS)         43.         13.         5.           +         317.         12.08         43.         13.         5.         5.	2		mibit	ICOMP	2	NUMBER OF HYDRO	GRAPHS TO (	COMBINE
HYDROGRAPH AT STATION C/2 PEAK FLOW TIME MAXIMUM AVERAGE FLOW 6-HR 24-HR 72-HR 59.92-HR + (CFS) (HR) (CFS) + 317. 12.08 43. 13. 5. 5. (INCHES) 4.502 5.539 5.540 5.540		* * *		***	***	***		* * *
PEAK FLOW         TIME         MAXIMUM AVERAGE FLOW           6-HR         24-HR         72-HR           + (CFS)         (HR)           (CFS)         (HR)           (CFS)         (13.           (CFS)         43.           (INCHES)         4.502				HYDROGRA	APH AT STA	ATION C/2		
6-HR 24-HR 72-HR 59.92-HR + (CFS) (HR) (CFS) + 317. 12.08 43. 13. 5. 5. (INCHES) 4.502 5.539 5.540 5.540	P	EAK FLOW	TIME			MAXIMUM AVER	AGE FLOW	
+ (CFS) (HR) + 317. 12.08 43. 13. 5. 5. (INCHES) 4.502 5.539 5.540 5.540					6-HR	24-HR	72-HR	59.92-HR
(CFS) + 317. 12.08 43. 13. 5. 5. (INCHES) 4.502 5.539 5.540 5.540	+	(CFS)	(HR)					
+ 317. 12.08 43. 13. 5. 5. (INCHES) 4.502 5.539 5.540 5.540				(CFS)				
(INCHES) 4,502 5,539 5,540 5,540	+	317.	12.08		43.	13.	5.	5.
(,				(INCHES)	4.502	5.539	5.540	5.540
(AC-FT) 21. 26. 26. 26.				(AC-FT)	21.	26.	26.	26.
				CUMULATIV	'E AREA =	.09 SQ MI		

233 KK * 03 *

#### STORM AREA = .01

10 PH				DEPTHS F	OR 0-P	ERCENT	HYPOTHETI	CAL STOR	М			
		HYDRO-35				TP-40				TP-	-49	
5	-MIN	15-MIN	60-MIN	2-HR	3-HR	6-HR	12-HR	24-HR	2-DAY	4-DAY	7-DAY	10-DAY
	.71	1.54	3.20	3.88	4.30	5.28	6.25	7.25	.00	.00	.00	.00

PRECIPITATION DATA

# SUBBASIN CHARACTERISTICS TAREA .01 SUBBASIN AREA

242 BA

SUBBASIN RUNOFF DATA

		*	*		
239	KK	* 5	s9 *		
		*	*		
		*******	* * * *		
			AREA S	59	
241	KO	OUTPU	JT CONTROL	VARIABLES	
			IPRNT	3	PRINT CONTROL
			IPLOT	0	PLOT CONTROL
			QSCAL	0.	HYDROGRAPH PLOT SCALE
			IPNCH	7	PUNCH COMPUTED HYDROGRAPH
			IOUT	21	SAVE HYDROGRAPH ON THIS UNIT
			ISAV1	1	FIRST ORDINATE PUNCHED OR SAVED
			ISAV2	720	LAST ORDINATE PUNCHED OR SAVED
			TIMINT	.083	TIME INTERVAL IN HOURS

*** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** ***

			HYDROGRA	PH AT STAT	ION 03		
	TOTAL RA	INFALL =	7.25, TOT	AL LOSS =	2.44, TOTAL	EXCESS =	4.81
	PEAK FLOW	TIME		6 110	MAXIMUM AVER	AGE FLOW	50.00.00
+	(CFS)	(HR)		6-HR	24-HR	/2-HR	59.92-HR
			(CFS)				
+	24.	12.58		6.	2.	1.	1.
			(INCHES)	3.899	4.791	4.791	4.791
			(AC-FT)	з.	4.	4.	4.
			CUMULATIV	E AREA =	.01 SQ MI		

***

#### UNIT HYDROGRAPH ONIT HIDROGRAPH 27 END-OF-PERIOD ORDINATES 11. 12. 2. 2. 0. 0. 5. 4. 0. 8. 3. 0. 11. 1.

* * *

# HR,

* * *

9. 1.

7. 1.

CCUMULATED-AREA	VS. TIM	CURVE	WILL BE	USED			
			* * *	÷			
		UNIT	HYDROGRAI	PH PARAMETERS			
	CLAR	( TC=	.64 HI	R, R=	.36	HR	
	SNYDER	2 TP=	52 HF	CP=	71		

236	BA	SUBBASIN CHARACTERISTICS TAREA .01 SUBBASIN AREA
		PRECIPITATION DATA
10	РН	DEPTHS FOR         0-PERCENT HYPOTHETICAL STORM           5-MIN         15-MIN         60-MIN         2-HR         3-HR         6-HR         12-HR         2-HR         2-DAY         4-DAY         7-DAY         10-DAY           .71         1.54         3.20         3.88         4.30         5.28         6.25         7.25         .00         .00         .00
		STORM AREA = .01
237	LS	SCS LOSS RATE STRTL .53 INITIAL ABSTRACTION CRVNBR 79.00 CURVE NUMBER RTIMP .00 PERCENT IMPERVIOUS AREA
238	US	SNYDER UNITGRAPH TP .52 LAG CP .71 PEAKING COEFFICIENT
		SYNTHETIC ACCUMULATED-AREA VS. TIME CURVE WILL BE USED

# SUBBASIN RUNOFF DATA

1. 6. 1.

****

* * *

3. 5. 0.

* * *

235 КО

ISAV1 ISAV1 TIMINT

- T CONTR IPRNT IPLOT QSCAL IPNCH IOUT
- **** OFFSITE AREA 3 OUTPUT CONTROL VARIABLES

	243 LS	SCS LOSS RATE STRTL	.53 I	NITIAL ABSTRACTION						
		RTIMP	.00 P	URVE NUMBER ERCENT IMPERVIOUS	AREA					
	244 US	SNYDER UNITGRAPH TP CP	1 .59 L .72 P	AG EAKING COEFFICIENI						
		SYNTHETIC ACCUMU	JLATED-AREA	VS. TIME CURVE WI	LL BE U	SED				
					***					
				UNIT HYD CLARK TC= SNYDER TP=	ROGRAPH .70 HR,	PARAMETERS R= CP=	.40 HR			
				UNI	T HYDRO	GRAPH				
		0. 1.	2.	31 END-C 3. 5.	F-PERIO 6	D ORDINATES 6.	6.	5.	4.	
		4. 3. 0. 0. 0.	2. 0.	2. 2. 0. 0.	1 0	. 1. . 0.	1. 0.	1. 0.	1. 0.	
	***	***	* * *	***		***				
		HYDROGRA	APH AT STAT	ION S9						
	TOTAL F	AINFALL = 7.25, TOT	TAL LOSS =	2.44, TOTAL EXC	ESS =	4.81				
	PEAK FLOW	TIME	6_UD	MAXIMUM AVERAGE	FLOW	50 02-110				
+	(CFS)	(HR)	0-11K	24-m	2-116	39.92-nk				
+	12.	12.67 (INCHES)	3.	1. 4.793 4	0. .793	0. 4.793				
		(AC-FT)	2.	2.	2.	2.				
		CUMULATIV	/E AREA =	.01 SQ MI						
,	*** *** ***	* *** *** *** *** *** *	*** *** ***	*** *** *** *** *	** ***	*** *** ***	*** *** *** *	** *** ***	*** *** ***	*** *** *** ***
	0.45	* *								
	243 KK	* P9 *								
		POND 9								
	247 КО	OUTPUT CONTROL V	ARIABLES	DINE COMPOS						
		IPLOT	0 P	LOT CONTROL	TE					
		IPNCH	7 P	UNCH COMPUTED HYDR	.OGRAPH	T				
		ISAV1	1 F	IRST ORDINATE PUNC	HED OR	SAVED				
		TIMINT	.083 T	IME INTERVAL IN HO	URS	AVED				
		SUBBASIN RUNOFF DA	ATA							
	248 BA	SUBBASIN CHARACT	TERISTICS	IDDACTN ADDA						
		DESCIPITATION D	.02 5	UDDASIN AREA						
	10 PH	110011111111011 01	DEP	THS FOR 0-PERCEN	т нурот	HETICAL STOP	RM			
		HYDRO-35 . 5-MIN 15-MIN 6	50-MIN 2	-HR 3-HR 6-H	0 R 12-	HR 24-HR	2-DAY 4-DA	CP-49 7-DAY	10-DAY	
		.71 1.54	3.20 3	.88 4.30 5.2	8 6.	25 7.25	.00 .00	.00	.00	
				STORM AREA	=	.02				
	249 LS	SCS LOSS RATE STRTL	.00 I	NITIAL ABSTRACTION						
		CRVNBR RTIMP	100.00 C .00 P	URVE NUMBER ERCENT IMPERVIOUS	AREA					
	250 UD	SCS DIMENSIONLES TLAG	S UNITGRAP .00 L	H AG						
					* * *					
				UNI	T HYDRO	GRAPH				
		140. 39.	8.	5 END-C 2. 0.	F-PERIO	D ORDINATES				
	* * *	* * *	***	* * *		* * *				
		HYDROGRA	APH AT STAT	ION P9						
	TOTAL F	AINFALL = 7.25, TOT	TAL LOSS =	.00, TOTAL EXC	ESS =	7.25				
	PEAK FLOW	TIME	c	MAXIMUM AVERAGE	FLOW	F0 00 ····				
+	(CFS)	(HR)	6-HR	24-HR 7	∠=HR	59.92-HR				
+	121.	(CFS) 12.08	14.	5.	2.	2.				

(INCHES)	5.277	7.247	7.250	7.250
(AC-FT)	7.	9.	9.	9.
CUMULATIVE	AREA =	.02 SQ MI		

*** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** ***

	***********	*									
251 KK	* S6 *	*									
	********	***									
		AREA S6									
253 KO	OUTPUT I C I I I I I I I I I I I	CONTROL VA PRNT PLOT SCAL IOUT SAV1 SAV2 MINT	RIABLES 3 I 0 I 0. I 7 I 21 S 1 I 720 I .083 2	PRINT CONTROL PLOT CONTROL HYDROGRAPH E PUNCH COMPUT SAVE HYDROGR FIRST ORDINAT FIME INTERVA	L LOT SCALE ED HYDROGRA APH ON THIS TE PUNCHED C E PUNCHED C L IN HOURS	PH UNIT OR SAVED PR SAVED					
	SUBBASIN	RUNOFF DAT	A								
254 BA	SUBBASI	IN CHARACTE	RISTICS .01 S	SUBBASIN ARE	A						
	PRECIPI	TATION DAT	A								
10 PH			DEI	PTHS FOR 0	-PERCENT HY	POTHETIC	L STORM				
	H	HYDRO-35 15-MIN 60	-MTN 2	2-HR 3-HR	TP-40 6-HR	12-HR 2	4-HR 2	-DAY 4-DAY	CP-49	10-DAY	
	.71	1.54	3.20 3	3.88 4.30	5.28	6.25	7.25	.00 .00	.00	.00	
				STC	RM AREA =	.01					
255 LS	SCS LOS S CF F	S RATE STRTL RVNBR RTIMP	.53 1 79.00 0	INITIAL ABST CURVE NUMBER PERCENT IMPE	RACTION RVIOUS AREA	L					
256 US	SNYDER	UNITGRAPH									
		CP	.52 1	LAG PEAKING COEF	FICIENT						
	SYNTHEI	IC ACCUMUL	ATED-ARE/	A VS. TIME C	URVE WILL B	E USED					
					*	**					
				υ	NIT HYDROGR	APH PARAN	IETERS				
				CLARK	TC= .64	HR,	R= .3	6 HR 1			
				01110211	11 .02	DROCRADU		<b>a</b>			
				2	7 END-OF-PE	RIOD ORDI	NATES		-	<i>.</i>	
	5.	4.	4. 3.	2.	2.	9.	1.	9.	1.	6. 1.	
	0.	0.	0.	0.	0.	0.	0.				
***	**	*	***		***	***					
		HYDROGRAP	H AT STAT	FION S	6						
TOTAL	RAINFALL =	7.25, TOTA	L LOSS =	2.44, TC	TAL EXCESS	= 4.81	-				
PEAK FLOW	TIME		6 UD	MAXIMUM A	VERAGE FLOW	1					
+ (CFS)	(HR)		0-HK	24=nk	/2=ns	. 59.5	2-nk				
+ 19.	12.58	(INCHES)	5. 3.899	1. 4.791	1. 4.791	. 4	1. 1.791				
		CIMULATIVE	ARFA =	01 SO M	т.		5.				
		COMULATIVE	ANEA -	.01 30 1	.1						
*** *** **	* *** *** ***	*** *** **	* *** ***	* *** *** **	* *** *** *	** *** **	* * * * * *	* *** *** *;	** *** ***	*** *** ***	*** *** *** ***
	*	*									
257 KK	* C/4 *	*									
	*********	***									
258 КО	OUTPUT I C I I I I	CONTROL VA PRNT PLOT SCAL PNCH IOUT SAV1 SAV2	RIABLES 3 I 0 I 0. I 21 S 1 I 720 1	PRINT CONTRO PLOT CONTROL HYDROGRAPH P PUNCH COMPUT SAVE HYDROGR FIRST ORDINA LAST ORDINA	L LOT SCALE ED HYDROGRA APH ON THIS TE PUNCHED O E PUNCHED O	PH UNIT OR SAVED R SAVED					
	TI	MINT	.083	TIME INTERVA	L IN HOURS						

259 HC HYDROGRAPH COMBINATION ICOMP 4 NUMBER OF HYDROGRAPHS TO COMBINE

	* * *		* * *	* * *	* * *		* * *
			HYDROGRA	PH AT STATION	N C/4		
1	PEAK FLOW	TIME		1	MAXIMUM AVER	AGE FLOW	
	(070)	(110)		6-HR	24-HR	72-HR	59.92-HR
+	(CFS)	(HK)	(CFS)				
+	140.	12.08		27.	9.	4.	4.
			(INCHES)	4.425	5.823	5.843	5.843
			(AC-FT)	13.	18.	18.	18.
			CUMULATIV	E AREA =	.06 SQ MI		

*** *** ;	*** *** *** *** ***	*** *** ***	*** *** ***	*** *** **	** *** ***	*** *** **	** *** ***	*** *** **	* *** ***	*** *** ***	* * * * * * *
	*****										
	* *										
260 KK	* R/P9 *										
	* *										
	F	OUTE THROUGH	lx48 INCH C	MP							
262 KO	OUTPUT CON	TROL VARIABLE	5								
	IPRN	IT 3	PRINT CON	ITROL							
	1PLC	.T. 0	HYDROGRAE	ROL	.F						
	IPNC	H 7	PUNCH COM	IPUTED HYDRO	JGRAPH						
	IOU	T 21	SAVE HYDR	OGRAPH ON ?	THIS UNIT						
	ISAV	1 1	FIRST ORD	INATE PUNCH	HED OR SAV	ED					
	ISAV	2 720	LAST ORDI	NATE PUNCHE	ED OR SAVE	D					
	IIMIN	.005	IIME INTE	RVAL IN HOU	JKS						
	HYDROGRAPH F	OUTING DATA									
263 RS	STORAGE RC	UTING									
	NSTE	S 1	NUMBER OF	SUBREACHES	S						
	RSVRT	C 610.70	INTTIAL C	ONDITION	DIIION						
		x .00	WORKING R	AND D COEFI	FICIENT						
264 SA	AREA	.0	.1	1.1	3.9	4.2					
265 SE	ELEVATION	610.70	612.00	620.00	630.00	630.80					
267 SL	LOW-LEVEL	OUTLET									
	ELEV	L 611.30	ELEVATION	I AT CENTER	OF OUTLET						
	CARE	A 12.57	CROSS-SEC	TIONAL ARE	Ą						
	EXF	L .50	EXPONENT	OF HEAD							
266 SS	SPILLWAY										
	CRE	L 630.30	SPILLWAY	CREST ELEVA	ATION						
	SPWI	D 10.00	SPILLWAY	WIDTH							
	EXP	W 2.60 W 1.50	EXPONENT	OF HEAD							
					* * *						
			сс	MPUTED STOP	RAGE-ELEVA	TION DATA					
	STORAGE	.00 .02	3.67	27.17	30.43						
	ELEVATION 610	.70 612.00	620.00	630.00	630.80						
			cc	MPUTED OUTH	FLOW-ELEVA	TION DATA					
	OUTFLOW	.00 .00	114.93	127.15	142.29	161.51	186.74	221.32	271.60	351.46	
	ELEVATION 610	.70 611.30	613.33	613.79	614.41	615.31	616.66	618.83	622.65	630.30	
	OUTFLOW 352	.80 353.41	354.18	355.12	356.25	357.58	359.13	360.91	362.94	365.24	
	ELEVATION 630	.38 630.41	630.44	630.48	630.52	630.57	630.62	630.68	630.74	630.80	
			COMPUT	ED STORAGE	-OUTFLOW-E	LEVATION DA	ATA				
	STORAGE	.00 .00	.02	.15	.22	.34	.58	1.13	2.56	3.67	
	OUTFLOW	.00 .00	67.46	114.93	127.15	142.29	161.51	186.74	221.32	237.82	
	ELEVATION 610	.70 611.30	612.00	613.33	613.79	614.41	615.31	616.66	618.83	620.00	
	STORAGE 7	.25 27.17	28.37	28.69	28.81	28.95	29.10	29.28	29.47	29.68	
	OUTFLOW 271	.60 348.67	351.46	352.80	353.41	354.18	355.12	356.25	357.58	359.13	
	ELEVATION 622	.65 630.00	630.30	630.38	630.41	630.44	630.48	630.52	630.57	630.62	
	STORAGE 29	.91 30.16	30.43								
	OUTFLOW 360	.91 362.94	365.24								
	ELEVATION 630	.68 630.74	630.80								

* * *

*** WARNING *** MODIFIED PULS ROUTING MAY BE NUMERICALLY UNSTABLE FOR OUTFLOWS BETWEEN 0. TO 115. THE ROUTED HYDROGRAPH SHOULD BE EXAMINED FOR OSCILLATIONS OR OUTFLOWS GREATER THAN PEAK INFLOWS. THIS CAN BE CORRECTED BY DECREASING THE TIME INTERVAL OR INCREASING STORAGE (USE A LONGER REACH.)

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#### HYDROGRAPH AT STATION R/P9

PEAK FLOW	TIME		6-40	MAXIMUM AVER	AGE FLOW	50 02_UD
+ (CFS)	(HR)		0-IIK	24-m	/2-nr	39.92-nk
	. ,	(CFS)				
+ 122.	12.17		27.	9.	4.	4.
		(INCHES)	4.425	5.823	5.842	5.842
		(AC-FT)	13.	18.	18.	18.
PEAK STORAGE	TIME			MAXIMUM AVERA	GE STORAGE	
			6-HR	24-HR	72-HR	59.92-HR
+ (AC-FT)	(HR)					
0.	12.17		0.	0.	0.	0.
PEAK STAGE	TIME			MAXIMUM AVER	AGE STAGE	
			6-HR	24-HR	72-HR	59.92-HR
+ (FEET)	(HR)					
613.61	12.17		611.63	611.41	611.35	611.35
		CUMULATIV	e area =	.06 SQ MI		

*** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** ***

***** * * 268 KK * LF5 * * * SUBAREA LF5

270	KO	OUTPUT CONTROL	VARIABLES	
		IPRNT	3	PRINT CONTROL
		IPLOT	0	PLOT CONTROL
		QSCAL	0.	HYDROGRAPH PLOT SCALE
		IPNCH	7	PUNCH COMPUTED HYDROGRAPH
		IOUT	21	SAVE HYDROGRAPH ON THIS UNIT
		ISAV1	1	FIRST ORDINATE PUNCHED OR SAVED
		ISAV2	720	LAST ORDINATE PUNCHED OR SAVED
		TIMINT	.083	TIME INTERVAL IN HOURS

#### SUBBASIN RUNOFF DATA

PLANE1 MAIN

271	BA	SUBBASIN CH TAREA	ARACTERISTICS	SUBBASI	N AREA							
		PRECIPITATI	ON DATA									
10	PH	HYDRO	-35I	DEPTHS FC	R 0-PE	RCENT I TP-40	IYPOTHET]	ICAL STO	RM 	TP-4	19	
		5-MIN 15-M .71 1.	IN 60-MIN 54 3.20	2-HR 3.88	3-HR 4.30	6-HR 5.28	12-HR 6.25	24-HR 7.25	2-DAY .00	4-DAY .00	7-DAY .00	10-DAY .00
					STORM	AREA =	.04					
272	LS	SCS LOSS RA STRTL CRVNBR RTIMP	TE .27 88.00 .00	INITIAL CURVE N PERCENT	ABSTRAC UMBER IMPERVI	TION	CA.					
273	UK	KINEMATIC W OVERLAND-	AVE FLOW ELEMENT	NO. 1	DELOW	FNCTU						
		S N PA	.2500 .300 100.0	SLOPE ROUGHNE PERCENT	SS COEFF OF SUBE	TICIENT BASIN	NTERVAL	3				
		MUSKINGUM-C	UNGE	1111111101	NONDER	OI DA .	INTERVIER.	,				
274	RD	MAIN CHAN	NEL									
		L	1500.	CHANNEL	LENGTH							
		S	.0100	CHANNET	DOLICUME	COF	FTOTENT					
		CA	.030	CONTRIB	UTING AR	SS COEI	LICIPHI					
		SHAPE	TRAP	CHANNEL	SHAPE							
		WD	.00	BOTTOM	WIDTH OF	DIAME'	ER					
		Z	2.00	SIDE SI	OPE							
		RUPSTQ	NO	ROUTE U	PSTREAM	HYDROGI	APH					
							* * *					
			COMPL	JTED MUSK COMPUT	INGUM-CU ATION TI	INGE PAI	AMETERS					
		ELEMENT	ALPHA	М	DT	1	X	PEAK	TIME TO PEAK	VOLUM	IE M	AXIMUM ELERITY
					(MIN)	(1	T)	(CFS)	(MIN)	(IN)		(FPS)

CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .1127E+02 OUTFLOW= .1091E+02 BASIN STORAGE= .7211E-03 PERCENT ERROR= 3.1

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

	MAIN	2.31	1.33	5.00	151.79	725.00	5.64
* * *	* * *	***		* * *	***		

2.481.671.1251.80165.862.311.333.34750.00152.95

H-2-58

724.83 5.84 .87 723.70 5.65 7.50

		HYDROGI	RAPH AT STA	TION LF5				
TOTAL F	RAINFALL =	7.25, T	DTAL LOSS =	1.41, TOTA	L EXCESS =	5.84		
PEAK FLOW	TIME			MAXIMUM AVE	RAGE FLOW			
+ (CFS)	(HR)		6-HR	24-HR	72-HR	59.92-HR		
+ 152.	12.08	(CFS)	18.	5.	2.	2.		
		(INCHES) (AC-FT)	4.591 9.	5.638 11.	5.639 11.	5.639 11.		
		CUMULAT	IVE AREA =	.04 SO MT				
		CONCLASS		101 0g III				
*** *** ***	* *** *** **	* *** ***	*** *** **	* *** *** ***	*** *** ***	*** *** ***	*** *** *** *** *** *** *** *** *** ***	* ***
	*********	* * * *						
275 KK	* E *	* 8						
	********	**** POND	3					
277 80	OUMDI		VADIADIEC					
277 KU	OUIPC	IPRNT	VARIABLES 3	PRINT CONTROL				
		IPLOT QSCAL	0.	PLOT CONTROL HYDROGRAPH PLC	T SCALE			
		IPNCH IOUT	7 21	PUNCH COMPUTED SAVE HYDROGRAF	HYDROGRAPH H ON THIS U	NIT		
		ISAV1	1	FIRST ORDINATE	PUNCHED OR	SAVED		
		TIMINT	.083	TIME INTERVAL	IN HOURS	UNVED		
	SUBBASI	.N RUNOFF I	JATA					
278 BA	SUBBA	TAREA	.07	SUBBASIN AREA				
	PRECI	PITATION N	DATA					
10 PH			DE	PTHS FOR 0-E	ERCENT HYPO	THETICAL STOP	М	
		HYDRO-35	60-MTN	2_40 3_40	TP-40		TP-49	
	.71	1.54	3.20	3.88 4.30	5.28 6	.25 7.25	.00 .00 .00 .00	
				STORM	1 AREA =	.07		
279 LS	SCS I	OSS RATE						
		STRTL CRVNBR	.00 100.00	INITIAL ABSTRA CURVE NUMBER	CTION			
		RTIMP	.00	PERCENT IMPERV	IOUS AREA			
280 UD	SCS I	IMENSIONL	ESS UNITGRA	PH				
		TING	.00	LING				
				5	END-OF-PERI	OGRAPH OD ORDINATES		
	407.	114.	22.	4.	0.			
***		***	***	**	*	***		
		HYDROGI	RAPH AT STA	TION P8				
TOTAL F	RAINFALL =	7.25, T	OTAL LOSS =	.00, TOTA	L EXCESS =	7.25		
PEAK FLOW	TIME		6 HD	MAXIMUM AVE	RAGE FLOW	50.00 UD		
+ (CFS)	(HR)		6-HR	24-HR	/2-HR	59.92-HK		
+ 353.	12.08	(CFS)	40.	14.	6.	6.		
		(INCHES) (AC-FT)	5.277 20.	7.246	7.249 27.	7.249 27.		
		CUMULAT	IVE AREA =	.07 SO MI				
*** *** ***	* *** *** **	* *** ***	*** *** **	* *** *** ***	*** *** ***	*** *** ***	*** *** *** *** *** *** *** *** *** ***	* ***
	*********	*						
281 KK	* S	* *						
	********	**** AREA !	38					
202 20	OTHERS		WADTADIES					
203 KU	OUT'PU	IPRNT	VARIABLES	PRINT CONTROL				
		IPLOT QSCAL	0.	PLOT CONTROL HYDROGRAPH PLC	T SCALE			
		IPNCH IOUT	7 21	PUNCH COMPUTED	HYDROGRAPH	NIT		
		ISAV1	1	FIRST ORDINATE	PUNCHED OR	SAVED		
		ISAV2 TIMINT	720 .083	LAST ORDINATE TIME INTERVAL	FUNCHED OR IN HOURS	SAVED		

	SUBBAS	IN RUNOFF D	DATA						
284 BA	SUBB	ASIN CHARAC TAREA	TERISTICS .01 S	UBBASIN AREA					
	PREC	IPITATION D	ATA						
10 PH			DEP	THS FOR 0-PE	RCENT HYPO	THETICAL STO	RM		
	5-MII .7	. HYDRO-35 N 15-MIN I 1.54	60-MIN 2 3.20 3	-HR 3-HR .88 4.30 STORM	TP-40 6-HR 12 5.28 6 AREA =	-HR 24-HR .25 7.25	2-DAY 4-DAY .00 .00	49 7-DAY 10-D. .00 .	 AY 00
295 TC	000	LOCC DATE							
200 10	505	STRTL CRVNBR RTIMP	.67 II 75.00 CI .00 PI	NITIAL ABSTRAC URVE NUMBER ERCENT IMPERVI	CTION				
286 US	SNYD	ER UNITGRAF TP CP	21 L	AG EAKING COEFFIC	CIENT				
	SYNTI	HETIC ACCUM	IULATED-AREA	VS. TIME CURV	YE WILL BE	USED			
					***				
				UNIT CLARK TC SNYDER TF	HYDROGRAP = .25 HR = .21 HR	H PARAMETERS , R= , CP=	.16 HR .68		
					UNIT HYDR	OGRAPH			
	5. 0.	17. 0.	24.	12 E 19. 1	ND-OF-PERI	OD ORDINATES 7. 4.	2.	1. 1	
***		* * *	***	* * *		* * *			
		HYDROGR	APH AT STAT	ION S8					
TOTAL F	RAINFALL =	7.25, TC	TAL LOSS =	2.88, TOTAL	EXCESS =	4.37			
PEAK FLOW	TIME			MAXIMUM AVER	AGE FLOW				
+ (CFS)	(HR)		6-HR	24-HR	72-HR	59.92-HR			
+ 28.	12.25	(CFS) (INCHES)	5. 3.573	1. 4.352	1. 4.352	1. 4.352			
		(AC-FT)	2.	3.	3.	3.			
		CUMULATI	VE AREA =	.01 SQ MI					
*** *** *** 287 KK	* *** *** *: ********** * * *	** *** *** ***** /4 * *	*** *** ***	*** *** *** *	** *** ***	*** *** ***	*** *** *** ***	*** *** ***	
	*******	* * * * *							
288 KO	OUTPI	UT CONTROL IPRNT IPLOT QSCAL IPNCH IOUT ISAV1 ISAV2 TIMINT	VARIABLES 3 Pi 0 Pi 0. Hi 7 Pi 21 S. 1 F 720 Li .083 T	RINT CONTROL LOT CONTROL YDROGRAPH PLOT UNCH COMPUTED AVE HYDROGRAPH IRST ORDINATE F IME INTERVAL I	' SCALE HYDROGRAPH I ON THIS U PUNCHED OR PUNCHED OR IN HOURS	NIT SAVED SAVED			
289 HC	HYDRO	OGRAPH COME ICOMP	INATION 4 N	UMBER OF HYDRC	GRAPHS TO	COMBINE			
					***				
* * *		* * *	* * *	* * *		* * *			
		HYDROGR	RAPH AT STAT	ION C/4					
PEAK FLOW	TIME		6-HR	MAXIMUM AVER 24-HR	AGE FLOW 72-HR	59.92-HR			
+ (CFS)	(HR)	(CFS)	5						
+ 642.	12.08	(INCHES) (AC-FT)	89. 4.723 44.	30. 6.253 59.	12. 6.268 59.	12. 6.268 59.			
		CUMULATI	VE AREA =	.18 SQ MI					
*** *** ***	* *** *** *	** *** ***	*** *** ***	*** *** *** *	** *** ***	*** *** ***	*** *** *** ***	*** *** ***	*** *** *** *** ***

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290 KK	* R,	/P8 *										
	*	*										
		ROUTE	THROUGH P	8 SPILLWAY								
292 KO	OUTI	PUT CONTROL IPRNT IPLOT QSCAL IPNCH IOUT ISAV1 ISAV2 TIMINT	VARIABLES 3 0. 7 21 1 720 .083	PRINT CONT PLOT CONT HYDROGRAPH PUNCH COM SAVE HYDRO FIRST ORDI LAST ORDIN TIME INTER	TROL ROL H PLOT SC PUTED HYE DGRAPH ON NATE PUNC RVAL IN H	CALE DROGRAPH I THIS UNI ICHED OR SA CHED OR SA IOURS	T SAVED VED					
	HYDRO	RAPH ROUTT	NG DATA									
			~									
293 KS	510	NSTPS ITYP RSVRIC X	1 ELEV 561.00 .00	NUMBER OF TYPE OF IN INITIAL CO WORKING R #	SUBREACH NITIAL CO ONDITION AND D COE	HES DNDITION EFFICIENT						
294 SA		AREA	.0	8.6	12.6	16.8	21.1	29.4				
295 SE	ELEV	VATION	561.00	570.00	580.00	590.00	600.00	618.00				
296 SS	SPII	CREL SPWID COQW EXPW	617.00 200.00 2.60 1.50	SPILLWAY ( SPILLWAY W WEIR COEFF EXPONENT (	CREST ELE VIDTH FICIENT DF HEAD	VATION						
						***						
				COM	APUTED SI	ORAGE-ELE	VATION DATA					
	STORAGE ELEVATION	.00 561.00	25.83 570.00	131.39 580.00	278.14 590.00	467.5 600.0	920.15 618.00					
				COM	APUTED OU	JTFLOW-ELE	VATION DATA					
	OUTFLOW ELEVATION	.00 561.00	.00 617.00	.09 617.00	.71 617.01	2.4	1 5.71 3 617.05	11.14 617.08	19.25 617.11	30.59 617.15	45.64 617.20	
	OUTFLOW ELEVATION	65.00 617.25	89.17 617.31	118.68 617.37	154.08 617.44	195.8 617.5	244.65 2 617.60	300.93 617.69	365.20 617.79	438.05 617.89	520.00 618.00	
				COMPUTE	ED STORAG	E-OUTFLOW	-ELEVATION D	ATA				
	0700300	0.0	05 00	101 00	070 14	467.5	0 001 04	0.01 20	001 04	000 47	002 07	
	OUTFLOW ELEVATION	.00 .00 561.00	25.83 .00 570.00	.00 580.00	278.14 .00 590.00	467.5 .0 .0	0 .00 0 617.00	.71 617.01	2.41 617.03	5.71 617.05	11.14 617.08	
	STORAGE OUTFLOW ELEVATION	894.25 19.25 617.11	895.41 30.59 617.15	896.75 45.64 617.20	898.27 65.00 617.25	899.9 89.1 617.3	97 901.85 7 118.68 1 617.37	903.92 154.08 617.44	906.16 195.89 617.52	908.59 244.65 617.60	911.21 300.93 617.69	
	STORAGE OUTFLOW	914.00 365.20 617.79	916.98 438.05 617 89	920.15 520.00								
+++	DELVATION	+++		010.00	***		+++					
		WEDDOO										
		HYDROG.	RAPH AT ST.	ATION F	R/ P8							
PEAK FLO + (CFS)	W TIME (HR)		6-HR	MAXIMUN 24-F	4 AVERAGE IR	FLOW 72-HR	59.92-HR					
+ 0.	.00	(CFS) (INCHES) (AC-FT)	0. .000	. OC	). )0	0. .000 0.	0.					
PEAK STOR	AGE TIME	,	6-HR	MAXIMUM 24-H	AVERAGE IR	STORAGE 72-HR	59.92-HR					
+ (AC-FT) 59.	(HR) 40.42		59.	59	9.	46.	46.					
PEAK STA	GE TIME			MAXIMUN	4 AVERAGE	STAGE						
+ (FEET)	(HR)		6-HR	24-H	łR	72-HR	59.92-HR					
573.11	38.25		573.11	573.1	L1 5	570.82	570.82					
		CUMULAT	IVE AREA =	.18 SÇ	2 MI							
*** *** *	** *** *** :	*** *** ***	*** *** *	** *** ***	*** ***	*** *** *	** *** *** *	** *** ***	*** *** **	* *** ***	*** *** **	* *** ***
	*******	*****										
	*	*										
297 KK	* S3/I *	DP6 * *										
	* * * * * * * * *	***** ARFA	\$3									
		nun										

299 KO OUTPUT CONTROL VARIABLES IPRNT 3 PRINT CONTROL IPLOT 0 PLOT CONTROL

	QSCAL0.HYDROGRAPH PLOT SCALEIPNCH7PUNCH COMPUTED HYDROGRAPHIOUT21SAVE HYDROGRAPH ON THIS UNITISAVI1FIRST ORDINATE PUNCHED OR SAVEDISAV2720LAST ORDINATE PUNCHED OR SAVEDTIMINT.083TIME INTERVAL IN HOURS
	SUBBASIN RUNOFF DATA
300 BA	SUBBASIN CHARACTERISTICS TAREA .00 SUBBASIN AREA
	PRECIPITATION DATA
10 PH	DEPTHS FOR         0-PERCENT HYPOTHETICAL STORM            HYDRO-35          TP-40           5-MIN         15-MIN         60-MIN         2-HR         3-HR           7.1         1.54         3.20         3.88         4.30         5.28         6.25         7.25         .00         .00         .00
301 LS	SIGKE AREA00
	STRTL .67 INITIAL ABSTRACTION CRVNBR 75.00 CURVE NUMBER RTIMP .00 PERCENT IMPERVIOUS AREA
302 US	SNYDER UNITGRAPH TP .40 LAG CP .78 PEAKING COEFFICIENT
	SYNTHETIC ACCUMULATED-AREA VS. TIME CURVE WILL BE USED
	***
	UNIT HYDROGRAPH PARAMETERS CLARK TC= .50 HR, R= .22 HR
	SNYDER TP= .40 HR, CP= .77
	0811 HIDROGRAPH 18 END-OF-PERIOD ORDINATES 0. 0. 1. 1. 1. 1. 1. 0. 0.
	0. 0. 0. 0. 0. 0. 0. 0.
***	*** *** ***
momat p	HYDROGRAPH AT STATION S3/DP6
PEAK FLOW	TIME MAXIMIM AURRAGE FLOW
+ (CFS)	6-HR 24-HR 72-HR 59.92-HR (HR)
+ 2.	(CFS) 12.42 0. 0. 0. 0. (INCHES) 3.574 4.354 4.354 (AC-FT) 0. 0. 0.
	CUMULATIVE AREA = .00 SQ MI
*** *** ***	*** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** ***
202 ***	***************************************
303 KK	^ 5// UP3 ^ * * * ******************************
305 KO	OUTPUT CONTROL VARIABLES         IPRNT       3         IPLOT       OPLOT CONTROL         IPLOT       0         QSCAL       0.         HYDROGRAPH PLOT SCALE         IPNCH       7         PUNCH COMPUTED HYDROGRAPH         IOUT       21         SAVE HYDROGRAPH         ISAV1       1         ISAV2       720         LAST ORDINATE PUNCHED OR SAVED         TIMINT       .083
	SUBBASIN RUNOFF DATA
306 BA	SUBBASIN CHARACTERISTICS TAREA .03 SUBBASIN AREA
	PRECIPITATION DATA
10 PH	DEPTHS FOR         0-PERCENT HYPOTHETICAL STORM           HYDRO-35         TP-40           5-MIN         15-MIN           60-MIN         2-HR           3.88         4.30           5.28         6.25           7.1         1.54           3.20         3.88           4.30         5.28           6.25         7.25           .00         .00
307 J.S	SIGNI AREA03
50, 10	STRTL .67 INITIAL ABSTRACTION CRVNBR 75.00 CURVE NUMBER RTIMP .00 PERCENT IMPERVIOUS AREA
308 US	SNYDER UNITGRAPH

TP .86 LAG CP .68 PEAKING COEFFICIENT

SYNTHETIC ACCUMULATED-AREA VS. TIME CURVE WILL BE USED

					***					
				UN	NIT HYDROGRAP	H PARAMETER:	67 HP			
				SNYDER	TP= .85 HR	, CP=	.68			
				50	UNIT HYDR	OGRAPH	3			
	1.	2.	4. 14	6. 13	8. 1 11 1	1. 13	. 15.	16.	17.	
	5.	5.	4.	4.	3.	3. 3 1 1	. 2.	2.	2.	
	0.	0.	0.	0.	0.	0. 0	. 0.	0.	0.	
***		***	***	,	***	***				
		HYDROGR#	APH AT STAT	ION S7/DP3	3					
TOTAL RA	INFALL =	7.25, TOI	AL LOSS =	2.88, TO1	TAL EXCESS =	4.37				
PEAK FLOW	TIME		6-HR	MAXIMUM AV 24-HR	/ERAGE FLOW 72-HR	59.92-HR				
(CFS)	(HR)	(CFS)								
36.	12.92	(INCHES) (AC-FT)	12. 3.548 6.	4. 4.351 7.	2. 4.351 7.	2. 4.351 7.				
		CUMULATIV	/E AREA =	.03 SQ MI	I.					
*** *** ***	*** *** **	** *** *** *	** *** ***	*** *** ***	* *** *** ***	*** *** ***	* *** *** ***	*** *** ***	* *** *** *** **	** *** *** ***
	* * * * * * * * * *	****								
309 KK	* * S5/DE	* ?4 *								
	* * * * * * * * * * * * * *	*								
		AREA S5								
311 KO	OUTPU	IPRNT	ARIABLES 3 P	RINT CONTROL						
		QSCAL	0. H	YDROGRAPH PI	LOT SCALE					
		IOUT	21 S	AVE HYDROGRA	APH ON THIS U	NIT				
		ISAVI ISAV2	720 I	AST ORDINATE	PUNCHED OR	SAVED				
		IIMINI	.065 1	IME INIERVAI	IN HOURS					
	SUBBASI	IN RUNOFF DA	TA							
312 BA	SUBBA	ASIN CHARACI TAREA	ERISTICS .01 S	UBBASIN AREA	ł					
	PRECI	IPITATION DA	TA							
10 PH		HYDRO-35	DEF	THS FOR 0-	-PERCENT HYPO	THETICAL STO	ORM	TP-49		
	5-MIN	N 15-MIN 6	50-MIN 2 3.20 3	-HR 3-HR	6-HR 12 5.28 6	-HR 24-HR	2-DAY 4-D	DAY 7-DAY	10-DAY	
				STOP	M AREA =	.01				
313 LS	SCS I	LOSS RATE								
		STRTL CRVNBR	.67 I 75.00 C	NITIAL ABSTR URVE NUMBER	RACTION					
		RTIMP	.00 P	ERCENT IMPER	RVIOUS AREA					
314 US	SNYDE	ER UNITGRAPH TP	I .23 I	AG						
		CP	.70 P	EAKING COEFI	FICIENT					
	SYNTH	HETIC ACCUMU	ILATED-AREA	VS. TIME CU	JRVE WILL BE	USED				
					* * *					
				CLARK	TC= .28 HR	H PARAMETERS	3 .16 HR			
				SNYDER	TP= .23 HR	, CP=	.70			
	2	7		12	UNIT HYDR 2 END-OF-PERI	OGRAPH OD ORDINATE:	3	1	0	
	2.	0.	11.	10.	ь.	4. 2	. 1.	1.	υ.	
* * *		* * *	* * *	÷	* * *	* * *				
		HYDROGRA	PH AT STAT	ION S5/DP4	1					
TOTAL RA	INFALL =	7.25, TOI	AL LOSS =	2.88, TO	TAL EXCESS =	4.37				
PEAK FLOW	TIME		6-HP	MAXIMUM AV	/ERAGE FLOW	59 92-PD				
· (CFS)	(HR)		0-HK	∠4-HR	/Z-HK	39.92-HR				

			(CFS)									
+	14.	12.25	(INCHES) (AC-FT)	2. 3.573 1.	1. 4.352 1.	0. 4.352 1.	0. 4.352 1.					
			CUMULATIVE	AREA =	.01 SQ MI							
**	* *** ***	*** *** **	* *** *** **	* *** ***	*** *** ***	*** *** **	* *** *** ***	*** *** ***	* * * * * * * * *	* *** *** *	** *** *** ***	***
		* * * * * * * * * * *	* * * *									
3	315 KK	* * S4/DP	* 5 *									
		* *******	*									
			AREA S4									
3	317 КО	OUTPU	T CONTROL VA IPRNT	RIABLES 3 PI	RINT CONTROL							
			IPLOT QSCAL	0 P1 0. H1	LOT CONTROL YDROGRAPH PLO	OT SCALE						
			IOUT	21 SI	UNCH COMPUTEI AVE HYDROGRAI	D HYDROGRAP PH ON THIS	'H UNIT					
		,	ISAVI ISAV2	720 Li	AST ORDINATE	PUNCHED OR	SAVED					
			1141111	.005 1.	IME INTERVAL	IN HOURS						
		SUBBASI	N RUNOFF DAT	A								
1	318 BA	SUBBA:	SIN CHARACTE TAREA	RISTICS .00 ST	UBBASIN AREA							
		PRECI	PITATION DAT	A								
	10 PH		HYDRO-35	DEP:	THS FOR 0-1	PERCENT HYP . TP-40	OTHETICAL STO	RM	. TP-49			
		5-MIN .71	15-MIN 60 1.54	-MIN 2- 3.20 3	-HR 3-HR .88 4.30	6-HR 1 5.28	2-HR 24-HR 6.25 7.25	2-DAY 4- .00	-DAY 7-DAY .00 .00	10-DAY .00		
					STOR	M AREA =	.00					
3	319 LS	SCS LO	OSS RATE	67 7	NITERAL ADORD	A CHI T ON						
		(	CRVNBR RTIMP	75.00 Ct .00 Pl	URVE NUMBER ERCENT IMPER	VIOUS AREA						
1	320 US	SNYDEI	R UNITGRAPH TP	.48 Li	AG							
			CP	.74 PI	EAKING COEFF:	ICIENT						
		SYNTH	ETIC ACCUMUL	ATED-AREA	VS. TIME CUI	RVE WILL BE	USED					
					UN	IT HYDROGRA	PH PARAMETERS					
					CLARK SNYDER	ГС= .58 H ГР= .48 H	IR, R= IR, CP=	.31 HR .74				
		-			24	UNIT HYD END-OF-PER	ROGRAPH		-	-		
		0. 2.	1.	2.	3.	4. 1.	5. 4. 0. 0.	4. 0.	3.	2.		
	* * *	υ.	∪.	v. ***	U. +-	* *	* * *					
			HYDROGRAP	H AT STAT	ION S4/DP5							
	TOTAL R	AINFALL =	7.25, TOTA	L LOSS =	2.88, TOTA	AL EXCESS =	4.37					
I	PEAK FLOW	TIME			MAXIMUM AVI	ERAGE FLOW						
+	(CFS)	(HR)		6-HR	24-HR	72-HR	59.92-HR					
+	8.	12.50	(CFS)	2.	1.	0.	0.					
			(AC-FT)	3.570	4.353	4.353	4.353					
			CUMULATIVE	AREA =	.00 SQ MI							
1						RUNOFF S	UMMARY					
					FLOW I TIME IN H	IN CUBIC FE HOURS, ARE	ET PER SECOND A IN SQUARE M	ILES				
				PEAK	TIME OF	AVERAGE F	LOW FOR MAXIM	UM PERIOD	BASIN	MAXIMUM	TIME OF	
+	OPI	ERATION	STATION	FLOW	PEAK	6-HOUR	24-HOUR	72-HOUR	AREA	STAGE	MAX STAGE	
	HYI	DROGRAPH AT	50	10	12.00	1	0	0	0.0			
Ŧ	DO	TTED TO	Ьp	10.	12.08	1.	υ.	υ.	.00			
+	KOI	ATT TO	R/P6	10.	12.08	1.	0.	0.	.00	636 44	12 08	
	HYI	DROGRAPH AT								000.11	12.00	
+			LF3	107.	12.08	11.	з.	1.	.02			
	HYI	DROGRAPH AT										

+		01	48.	12.67	13.	4.	2.	.03						
+	HYDROGRAPH AT	P5	15.	12.08	2.	1.	0.	.00						
+	4 COMBINED AT	C/4	147.	12.08	27.	9.	3.	.06						
+ +	ROUTED TO	R/P5	64.	12.67	27.	8.	з.	.06	637.94	12.67				
+	HYDROGRAPH AT	P4	5.	12.08	1.	0.	0.	.00						
+	2 COMBINED AT	C/2	65.	12.50	28.	9.	3.	.06						
+++	ROUTED TO	R/P4	65.	12.58	28.	9.	3.	.06	629.57	12.58				
+	HYDROGRAPH AT	P3	27.	12.08	з.	1.	0.	.01						
+	2 COMBINED AT	C/2	79.	12.08	31.	10.	4.	.07						
+ +	ROUTED TO	P3/DP1	85.	12.17	31.	10.	4.	.07	628.09	12.17				
+	HYDROGRAPH AT	P2	4.	12.08	1.	0.	0.	.00						
+ +	ROUTED TO	R/P2	4.	12.08	1.	0.	0.	.00	639.37	12.08				
+	HYDROGRAPH AT	R/CHA	8.	12.08	1.	0.	0.	.00						
+	HYDROGRAPH AT	LF1	171.	12.08	20.	6.	2.	.04						
+	HYDROGRAPH AT	Pl	17.	12.08	2.	1.	0.	.00						
+	3 COMBINED AT	C/3	196.	12.08	23.	7.	з.	.05						
+ +	ROUTED TO	R/Pl	120.	12.17	23.	7.	з.	.05	630.51	12.17				
+	HYDROGRAPH AT	CHB	67.	12.08	8.	2.	1.	.02						
+	HYDROGRAPH AT	LF2	99.	12.08	10.	3.	1.	.02						
+	3 COMBINED AT	C/3	282.	12.08	41.	13.	5.	.08						
+	HYDROGRAPH AT	CHC	298.	12.08	45.	14.	6.	.09						
+	HYDROGRAPH AT	S1	2.	12.67	0.	0.	0.	.00						
+	HYDROGRAPH AT	P7	44.	12.08	5.	2.	1.	.01						
+	3 COMBINED AT	C/3	343.	12.08	50.	16.	6.	.10						
+ +	ROUTED TO	R/P7	0.	.00	0.	0.	0.	.10	616.43	44.75				
+	HYDROGRAPH AT	02	150.	12.75	47.	15.	6.	.11						
+	HYDROGRAPH AT	S2	103.	12.83	34.	10.	4.	.08						
+	3 COMBINED AT	DP2	253.	12.75	81.	25.	10.	.29						
+ +	ROUTED TO	S2/DP2	253.	12.83	80.	25.	10.	.29	622.24	12.83				
+	HYDROGRAPH AT	LF6	92.	12.08	10.	з.	1.	.02						
+	HYDROGRAPH AT	04	28.	12.33	5.	2.	1.	.01						
+	2 COMBINED AT	C/2	106.	12.08	16.	5.	2.	.03						
+	HYDROGRAPH AT	CHD	169.	12.17	26.	8.	3.	.05						
+	HYDROGRAPH	AT	LF4		152.	12.08		16.	5.	2.	.03			
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+	2 COMBINED	AT	C/2		317.	12.08		43.	13.	5.	.09			
+	HYDROGRAPH	AT	03		24.	12.58		6.	2.	1.	.01			
+	HYDROGRAPH	AT	S9		12.	12.67		з.	1.	0.	.01			
+	HYDROGRAPH	AT	P9		121.	12.08		14.	5.	2.	.02			
+	HYDROGRAPH	AT	S6		19.	12.58		5.	1.	1.	.01			
+	4 COMBINED	AT	C/4		140.	12.08		27.	9.	4.	.06			
+	ROUTED TO		R/P9		122.	12.17		27.	9.	4.	.06			
+	HYDROGRAPH	AT										613.61	12.17	
+		2.00	LF5		152.	12.08		18.	5.	2.	.04			
+	IIIDKOGKAFII	A1	P8		353.	12.08		40.	14.	6.	.07			
+	HYDROGRAPH	AT	S8		28.	12.25		5.	1.	1.	.01			
+	4 COMBINED	AT	C/4		642.	12.08		89.	30.	12.	.18			
+ +	ROUTED TO		R/P8		0.	.00		0.	0.	0.	.18	573.11	38.25	
+	HYDROGRAPH	AT	S3/DP6		2.	12.42		0.	0.	0.	.00			
+	HYDROGRAPH	AT	S7/DP3		36.	12.92		12.	4.	2.	.03			
+	HYDROGRAPH	AT	S5/DP4		14.	12.25		2.	1.	0.	.01			
	HYDROGRAPH	AT	S4/DP5		8.	12.50		2.	1.	0.	.00			
+														
1					SUM	MARY OF (FLOW I	KINEMATI S DIRECT	C WAVE -	MUSKINGUM- ITHOUT BAS	CUNGE ROUT E FLOW)	ING			
1	ISTAQ	ELEMEN	IT	DT	SUM	MARY OF (FLOW I (TIM F	KINEMATI S DIRECT E TO EAK	C WAVE - C RUNOFF W VOLUME	MUSKINGUM- ITHOUT BAS C DT	CUNGE ROUT E FLOW) INTERPOL OMPUTATION PEAK	ING ATED TO INTERVAL TIME TO PEAK	VOLUME		
1	ISTAQ	ELEMEN	IT	DT (MIN)	SUM PEAI (CFS	MARY OF (FLOW I (TIM E	KINEMATI S DIRECT E TO EAK (MIN)	C WAVE - RUNOFF W VOLUME (IN)	MUSKINGUM- ITHOUT BAS C DT (MIN)	CUNGE ROUT E FLOW) INTERPOL COMPUTATION PEAK (CFS)	ING ATED TO INTERVAL TIME TO PEAK (MIN)	VOLUME (IN)		
1	ISTAQ LF3	ELEMEN MANE	IT	DT (MIN) 3.22	SUM PEAN (CFS 108.8	MARY OF (FLOW I (TIM E 3) 38 72	KINEMATI S DIRECT E TO EAK (MIN) 4.80	C WAVE C RUNOFF W VOLUME (IN) 5.64	MUSKINGUM- ITHOUT BAS DT (MIN) 5.00	CUNGE ROUT E FLOW) INTERPOL OMPUTATION PEAK (CFS) 106.96	ING ATED TO INTERVAL TIME TO PEAK (MIN) 725.00	VOLUME (IN) 5.67		
CONTINUIT	ISTAQ LF3 Y SUMMARY	ELEMEN MANE (AC-FT)	T - INF	DT (MIN) 3.22 LOW= .	SUM PEAN (CFS 108.8	MARY OF (FLOW I (TIM B 3) 38 72 ) EXCESS	KINEMATI S DIRECT EAK (MIN) 4.80 = .6941E	C WAVE - RUNOFF W VOLUME (IN) 5.64 HO1 OUTFL	MUSKINGUM- ITHOUT BAS DT (MIN) 5.00 OW= .6706E	CUNGE ROUT E FLOW) INTERPOL OMPUTATION PEAK (CFS) 106.96 +01 BASIN	ING ATED TO INTERVAL TIME TO PEAK (MIN) 725.00 STORAGE=	VOLUME (IN) 5.67 .5516E-03 PERCENT	ERROR=	3.4
CONTINUIT	ISTAQ LF3 Y SUMMARY R/CHA	ELEMEN MANE (AC-FT) MANE	T - INFI	DT (MIN) 3.22 LOW= . 3.31	SUM PEAI (CF: 108.1 .0000E+01	MARY OF (FLOW I (TIM E 3) 38 72 38 72 0 EXCESS	KINEMATI S DIRECT E TO EAK (MIN) 4.80 = .6941E 5.55	C WAVE - C RUNOFF W VOLUME (IN) 5.64 C+01 OUTFL 6.24	MUSKINGUM- ITHOUT BAS DT (MIN) 5.00 OW= .6706E 5.00	CUNGE ROUT E FLOW) INTERPOL OMPUTATION PEAK (CFS) 106.96 +01 BASIN 7.91	ING ATED TO INTERVAL TIME TO PEAK (MIN) 725.00 STORAGE= 725.00	VOLUME (IN) 5.67 .5516E-03 PERCENT 6.25	ERROR=	3.4
CONTINUIT	ISTAQ LF3 Y SUMMARY R/CHA Y SUMMARY	ELEMEN MANE (AC-FT) MANE (AC-FT)	T – INFI – INFI	DT (MIN) 3.22 LOW= . 3.31 LOW= .	SUM PEAI (CF: 108.( .0000E+0( 8.: 3472E+0(	MARY OF (FLOW I TIM E 3) 38 72 ) EXCESS 16 72 ) EXCESS	KINEMATI S DIRECT E TO EAK (MIN) 4.80 = .6941F 5.55 = .2868F	C WAVE - RUNOFF W VOLUME (IN) 5.64 :+01 OUTFL 6.24 :+00 OUTFL	MUSKINGUM- ITHOUT BAS DT (MIN) 5.00 OW= .6706E 5.00 OW= .6326E	CUNGE ROUT E FLOW) INTERPOL OMPUTATION PEAK (CFS) 106.96 +01 BASIN 7.91	ING ATED TO INTERVAL TIME TO PEAK (MIN) 725.00 STORAGE= STORAGE=	VOLUME (IN) 5.67 .5516E-03 PERCENT 6.25 .2835E-03 PERCENT	ERROR=	3.4
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CONTINUIT CONTINUIT CONTINUIT	ISTAQ LF3 Y SUMMARY R/CHA Y SUMMARY LF1 Y SUMMARY	ELEMEN MANE (AC-FT) MANE (AC-FT) MANE (AC-FT)	T - INF: - INF: - INF:	DT (MIN) 3.22 LOW= . 3.31 LOW= . 3.76 LOW= .	SUM PEAI (CF 108.4 .0000E+00 8.1 .3472E+00 187.2	MARY OF I (FLOW I (FLOW I F 3) 38 72 38 72 38 72 9 40 EXCESS 29 72 9 9 EXCESS	KINEMATI S DIRECT EAK (MIN) 4.80 = .6941E 5.55 = .2868E 2.88 = .1248E	C WAVE - P RUNOFF W VOLUME (IN) 5.64 :+01 OUTFL 6.24 :+00 OUTFL 5.62 :+02 OUTFL	MUSKINGUM- ITHOUT BAS DT (MIN) 5.00 OW= .6706E 5.00 OW= .6326E 5.00 OW= .1201E	CUNGE ROUT E FLOW) INTERPOL OMPUTATION PEAK (CFS) 106.96 +01 BASIN 7.91 +00 BASIN 171.06 +02 BASIN	ING ATED TO INTERVAL TIME TO PEAK (MIN) 725.00 STORAGE= 725.00 STORAGE=	VOLUME (IN) 5.67 .5516E-03 PERCENT 6.25 .2835E-03 PERCENT 5.59 .7302E-03 PERCENT	ERROR= ERROR=	3.4 .2 3.7
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## H-2-66

 CHD
 MANE
 5.00
 168.74
 730.00
 5.48
 5.00
 168.74
 730.00
 5.48

 CONTINUITY SUMMARY
 (AC-FT)
 INFLOW=
 .9545E+01
 EXCESSE
 .6630E+01
 OUTFLOW=
 .1608E+02
 BASIN
 STORAGE
 .2067E-02
 PERCENT
 ERRORE
 .6

 LF4
 MANE
 2.78
 160.98
 724.07
 5.64
 5.00
 152.01
 725.00
 5.64

 CONTINUITY SUMMARY
 (AC-FT)
 INFLOW=
 .000E+00
 EXCESSE
 .103E+02
 OUTFLOW=
 .1002E+02
 BASIN
 STORAGE
 .5353E-03
 PERCENT
 ERRORE
 3.3

 LF5
 MANE
 3.34
 152.95
 723.70
 5.65
 5.00
 151.79
 725.00
 5.64

 CONTINUITY SUMMARY
 (AC-FT)
 INFLOW=
 .0000E+00
 EXCESSE
 .1127E+02
 OUTFLOW=
 .1091E+02
 BASIN
 STORAGE
 .7211E-03
 PERCENT
 FRORE
 3.1

*** NORMAL END OF HEC-1 ***

## **APPENDIX H-3**

## POST-DEVELOPMENT FINAL COVER EROSION CONTROL STRUCTURE DESIGN

Includes Pages H-3-1 through H-3-21



## CONTENTS

### **DRAINAGE SWALE DESIGN**

H-3-1

### **GEOMEMBRANE LINED CHUTE DESIGN**

H-3-7



## DRAINAGE SWALE DESIGN

## Landfill Cover Swales

- Landfill cover drainage swale layout is shown on Figure H-1-2. A swale detail is provided on Figure H-1-6.
- Landfill Cover Swale Design Summary:
  - The swale drainage areas analyzed are shown on Figure H-3-6.
  - Swale peak flow calculations are summarized on page H-3-2.
  - Swale design calculations are provided on pages H-3-2 through H-3-5.
     Maximum normal depth is 1.69 feet. Swale design depth is 2 feet.
  - Maximum flow velocity is 4.27 fps. Given that the velocity is less than 5 fps, the swale will be lined with vegetation.
  - The swales are adequately designed to convey the 25-year, 24-hour frequency storm run-off.

Prep By: MB Date: 10/11/2023

### MUSKOGEE COMMUNITY RDF 0086-364-11-19 APPENDIX H SWALE DESIGN

<u>Required:</u>	Analyze the critical swales for each letdown to determine the adequacy of the swale design.							
<u>Method:</u>	<ol> <li>Determine the 25-year frequency flow rates for the swale drainage areas by the Wright-McLaughlin Method.</li> <li>Determine the size of the swales.</li> </ol>							
<u>Reference:</u>	<ol> <li>State of Oklahoma, Department of Transportation, Roadway Drainage, November, 2014, Chapter 7.</li> </ol>							
<u>Solution:</u>	1. Determine the 25-year frequency flow rates using the Wright-McLaughlin Method. $Q = C \cdot C_f \cdot I \cdot A$							
	Where:	$C = 0.7$ $C_{f} = 1.1$ $I = \text{ intensity in/}$ $A = \text{ drainage are}$ $I = \frac{a}{(t_{d} + b)^{c}}$	(runoff coefficient) (runoff correction factor, from Ref 1 for 25-yr design storm frequency) hr ea, ac					
		a = 67 b = 10 c = 0.75	From Ref 1, for Muskogee County (Zone 2) 25-year storm event					

t_d is assumed to be 10 min. for all cases

			Normal Depth for
Swale	Area	Flow Rate	Design Flow
	(ac)	(cfs)	(ft)
SW1	3.33	18.17	1.30
SW2	4.23	23.07	1.42
SW3	6.74	36.77	1.69
SW4	1.49	8.13	0.58

Channel	Flow Rate	Bottom		Side Slope ²	Side Slope ²	Bottom	Normal	Flow Vel.	Froude	Velocity	Energy	Flow Area	Top Width
	(cfs)	Slope (ft/ft) ¹	n-value	(left)	(right)	Width (ft)	Depth (ft)	(fps)	No.	Head (ft)	Head (ft)	(sq. ft.)	of Flow (ft)
SW1	18.17	0.01	0.03	2	4	0	1.30	3.57	0.781	0.20	1.50	5.09	7.81
SW2	23.07	0.01	0.03	2	4	0	1.42	3.79	0.792	0.22	1.65	6.08	8.54
SW3	36.77	0.01	0.03	2	4	0	1.69	4.27	0.818	0.28	1.98	8.61	10.17
SW4	8.13	0.01	0.03	2	20	0	0.58	2.17	0.708	0.07	0.66	3.75	12.84

¹ Swales will have a minimum 1.0 percent slope.

² Swale side slopes are 2 Horizontal(H) to 1 Vertical(V) and 4H:1V on landfill side slopes, and 20H:1V on top deck.

³ Calculations were performed using the HYDROCALC HYDRAULICS program developed by Dodson and Associates (Version 1.2a, 1996).

Maximum flow depth is 1.69 ft < 2.0 ft (channel depth)

Design is acceptable.

#### Example Calculation: Calculate the normal depth for the channel for drainage area SW1

List of Symbols

- $Q_d$  = design flow rate for channel, cfs
- R = hydraulic radius, ft
- n = Manning's roughness coefficient
- S = channel slope, ft/ft
- b = bottom width of channel, ft
- $z_r$  = z-ratio (ratio of run to rise for channel sideslope) for right sideslope of channel
- $z_l = z$ -ratio (ratio of run to rise for channel sideslope) for left sideslope of channel
- $A_f = flow area, sf$
- $g = gravitational acceleration = 32.2 \text{ ft/s}^2$
- T = top width of flow, ft
- $d= \ normal \ depth \ of \ channel, \ ft$

The program uses an iterative process to calculate the normal depth of the channel to satisfy Manning's Equation

Q =	1.486	$A R^{0.67} S^{0.5}$	
	n		
	0	10.15	
Design Inputs:	$Q_d =$	18.17	cfs (from page B-4-2)
	S =	0.01	ft/ft
	b =	0	ft
	$z_r =$	2	(H):1(V)
	$z_l =$	4	(H):1(V)
	n =	0.03	

Step 1 - Based on the geometry of the channel cross-section, solve for R and Af

R =	$bd + 1/2d^2(z_r + z_l)$							
	b + d((z	$(z_l^2 + 1)^{0.5} + (z_r^2 + 1)^{0.5})$						
$A_f = bd$	$1 + 1/2d^2(z)$	$(r + z_l)$						
assume:	d =	= <u>1.30</u> ft						
R =	0.614	ft						
$A_{\rm f}$ =	5.09	sf						
solve for Q	:	Q = 18.17						



cfs

Step 2 - solve for velocity, T (wet perimeter), Froude number, velocity head, and energy head

$$Q = VA \Longrightarrow \qquad V = Q/A$$

$$V = 3.57 \quad \text{ft/s}$$

$$T = b + d(z_1 + z_r)$$

$$T = 7.81 \quad \text{ft}$$

$$F_r = \frac{V}{(gA/T)^{0.5}}$$

$$F_r = 0.781$$

Velocity Head =  $\frac{V^2}{2g}$ Velocity Head = 0.2 ft

Energy Head = water elevation + velocity head

Energy Head = 1.50 ft





#### NOTES:

1. EXISTING CONTOURS AND ELEVATIONS PROVIDED BY HYDREX ENVIRONMENTAL COMPILED FROM AERIAL PHOTOGRAPHY FLOWN 01-25-2023.

SWALE DRAINAG	E AREA INFORMATION
DRAINAGE AREA	AREA (ACRES)
SW1	3.33
SW2	4.23
SW3	6.74
SW4	1.49



	PREPARED FOR					
MANAG	EMENT OF OKLAHOMA, INC.	TIER III PERMIT MODIFICATIO				
	REVISIONS	SWALE DRAINAGE AREAS				
DATE	DESCRIPTION					
		MUSKOGE MUSKOGEE	E COMMUNITY RDF COUNTY, OKLAHOMA			
		WWW.WCGRP.COM	FIGURE H-3-6			

## **GEOMEMBRANE LINED CHUTE DESIGN**

The letdown structures are designed using a geomembrane as a lining. The geomembrane is placed along the entire chute to protect the chute bottom and the final cover from erosion due to potential erosive velocities. Tumbling flow energy dissipaters will be placed at the bottom of the letdown structure to dissipate excess energy present in the water as it travels down the chutes.

- Chute layout is shown on Figure H-1-1. Chute details are provided on Figures H-1-5 through H-1-8.
- Chute Design Summary:
  - Chute drainage areas are shown on Sheet H-3-21.
  - Peak flow calculations are presented on page H-3-8.
  - As shown on pages H-3-9 and H-3-10, the maximum normal depth is 0.43 feet. The chutes have a minimum of two feet of depth.
  - As shown on pages H-3-9 and H-3-10, the maximum flow velocity is 39.55 fps.

### **Flowrate Data**

Flowrate data are the result of hydrologic modeling of the drainage areas by HEC-1 software.

**Reference:** 

1. State of Oklahoma, Department of Transportation, <u>Roadway Drainage</u>, November, 2014, Chapter 7.

2. U.S. Army Corps of Engineers, Hydrologic Engineering Center. 2013. HEC-HMS Hydrologic Modeling System, User's Manual, Version 4.0, CPD-74A. Hydrologic Engineering Center, Davis, CA.

Swale	Area ¹	Flow Rate ²
	(ac)	(cfs)
LD4	21.29	152.0
LD5	23.14	152.0
LD6	13.63	92.0

¹ The letdown drainage areas are

shown on Sheet H-3-21.

¹ Flow rates are calculated with HEC-1.

### 2. Uniform flow design for the upper portion of the geomembrane lined chutes above the dissapator.

Letdown	Flow Rate	Bottom	Manning's	Side Slope	Side Slope	Bottom	Normal	Flow Vel.	Froude	Velocity	Energy	Flow Area	Flow Top
	(cfs)	Slope (ft/ft)	n	(left)	(right)	Width (ft)	Depth (ft)	(fps)	Number	Head (ft)	Head (ft)	(sf)	Width (ft)
LD4	152.0	0.25	0.01	2.00	2.00	8.00	0.43	39.55	11.098	24.31	24.75	3.84	9.73
LD5	152.0	0.25	0.01	2.00	2.00	8.00	0.43	39.55	11.098	24.31	24.75	3.84	9.73
LD6	92.0	0.25	0.01	2.00	2.00	8.00	0.32	32.98	10.610	16.90	17.22	2.79	9.29

1. Calculations were performed using the HYDROCALC Hydraulics for Windows program developed by Dodson and Associates (Version 1.2a, 1996). Conclusions: Maximum normal depth is 0.43 feet. Chute design depth is 2.0 feet.

Maximum flow velocity for the upper portion of the geomembrane-lined chute is 39.55 fps.

### 3. Chute flow design for the lower portion of the geomembraned lined chutes (dissapator width).

Letdown	Flow Rate	Bottom	Manning's	Side Slope	Side Slope	Bottom	Normal	Flow Vel.	Froude	Velocity	Energy	Flow Area	Flow Top
	(cfs)	Slope (ft/ft)	n	(left)	(right)	Width (ft)	Depth (ft)	(fps)	Number	Head (ft)	Head (ft)	(sf)	Width (ft)
LD4	152.0	0.25	0.01	2	2	16	0.29	31.65	10.547	15.57	15.86	4.80	17.16
LD5	152.0	0.25	0.01	2	2	16	0.29	30.65	10.547	15.57	15.86	7.80	17.16
LD6	92.0	0.25	0.01	2	2	10	0.28	30.73	10.448	14.68	14.96	2.99	11.13

1. Calculations were performed using the HYDROCALC Hydraulics for Windows program developed by Dodson and Associates (Version 1.2a, 1996).

2. The bottom width of the geomembrane chutes are designed to accommodate the energy dissipater design given in Section 4 of these calculations.

Conclusions: Maximum normal depth is 0.29 feet. Chute design depth is 2.0 feet.

Maximum flow velocity for the lower portion of the geomembrane-lined chute is 31.65 fps.

<u>Required</u>	Determine the hydraulic properties for the grouted ripraps as energy letdown structures (chutes).
<u>Method:</u>	<ol> <li>Calculate the design flow rate of the chute section.</li> <li>Estimate the normal and flow velocity from Hydrocalc using calculated design flow rate.</li> <li>Calculate the critical depth and critical flow velocity.</li> <li>Calculate the height of the roughness element and spacing between the rows of the roughness elements.</li> <li>Calculate the total length of roughness elements.</li> </ol>
<u>Referenc</u>	<ol> <li>Henry M. Morris, <i>Hydraulic Dissipation in Steep, Rough Channels</i>, Bulletin19, Research Division, Virginia Polytechnic Institute, 1968.</li> <li>"Open Channel Hydraulics" by V.T. Chow.</li> <li>"Hydraulic Design of Energy Dissipators for Culverts and Channels", FHWA Hydraulics Engineering Circular Number 14, Third Edition.</li> <li>"Hydraulic Considerations for Corrugated Plastic Pipes" Plastic Pipe Institute.</li> <li>"Reclamation Managing Water in the West" Erosion and Sedimentation Manual. US Department of the Interior Bureau of Reclamation, November 2006.</li> </ol>
<u>Solution:</u>	The design of energy dissipators for the 25 percent sideslope is based on tumbling flow in the chute. Tumbling flow consists of a series of hydraulic jumps on overfalls that maintain the critical velocity in the chute.
	1. Calculate the design flow rate of the chute section.
	According to the definition of the unit flow rate, q=Q/b where: Q = flowrate, cfs (from Page H-3-8) = 152 cfs b = bottom width of chute, ft = 8 ft q = unit flowrate, cfs/ft of chute width
	q = 152 cfs/8 ft
	<ul> <li>2. Estimate the normal and flow velocity from Hydrocalc using flow rate and appropriate</li> <li>Manning's coefficient.</li> <li>Q = 152 cfs</li> </ul>
	Manning's coefficient selection (Reference 2, Page 14, Table 2) <b>FML:</b> n = 0.01 bottom slope = 0.25 ft/ft side slope = 2 ft/ft bottom width = 8ft
	From Hydrocalc

Normal depth = 0.43 ft Flow Velocity = 39.55 ft/sec

#### **For Chute LD4 (For the Lower Portion of the Chute): <u>1. Calculate the design flow rate of the chute section.</u>**

According to the definition of the unit flow rate, q=Q/b

where:

$$\begin{split} &Q = flowrate, cfs = 152 \ cfs \\ &b = bottom \ width \ of \ chute, \ ft = 16 \ ft \\ &q = unit \ flowrate, \ cfs/ft \ of \ chute \ width \end{split}$$

q = 152 cfs / 16 ft= 9.50 cfs/ft

#### 2. Estimate the normal and flow velocity from Hydrocalc using flow rate and appropriate Manning's coefficient.

Q = flow	vrate, $cfs = 152 cfs$	(Equation 5-12, Reference 2)						
roughness coeffic	ient, $n = (n_0 + n_1 + n_2 + n_3 + n_4) m_5$							
where,		(Reference 2, Page 109, Table 5-5)						
$n_0 = basic n$	value for straight, uniform, smooth	h channel						
based or	n material $= 0.025$	(Reference 2, Page 109, Table 5-5)						
$n_1 = value ad$	ded for surface irregularities $= 0.0$	01 (Reference 2, Page 109, Table 5-5)						
$n_2 = value ad$	ded for variation in channel cross	section= 0.0 (Reference 2, Page 109, Table 5-5)						
$n_3 = value action$	ded for obstructions $= 0.015$	(Reference 2, Page 109, Table 5-5)						
$n_4 = value ad$	ded for vegetation and flow condi	itions = $0.005$ (Reference 2, Page 109, Table 5-5)						
$m_5 = corrections$	on factor for meandering of chann	el =1.0						
n = (0.0)	25+.01+0.0+.015+.005)*1.0							
Therefore, n =	0.055							
bottom	slope = 0.25  ft/ft							
side slo	pe = 3  ft/ft							
bottom	width = $16$ ft							
From Hydrocalc								
Normal	depth = $0.79$ ft							
Flow Ve	elocity = 10.54 ft/sec							
	5							
3. Calculate the	critical depth and critical flow v	elocity.						
	1/2							
$Y_c = (q^2)$	/g) ^{1/3} (Reference 3, Equatio	n 7.1)						
$V_c = (gc$	(Reference 3) (Reference 3)							
where:								
$Y_c = cri$	tical depth, ft							
q = unit flowrate, cfs/ft of chute width								
g = acce	$g = acceleration due to gravity = 32.2 \text{ ft/s}^2$							
$V_c = cri$	$V_c = critical$ (also outlet) velocity, ft/s							
Y _c	$Y_c = (9.50^2)/32.2)^{1/3}$							
	= 1.41 ft							
Vc	= (32.2*9.5) ^{1/3}							

=

6.74 ft/s

# 4. Calculate the height of the roughness element and spacing between the rows of the roughness elements.

 $h = Y_{c/}((3-3.7S)^{(2/3)})$  (Reference 3, Equation 7.2)

where:

$$\begin{split} \mathbf{Y}_{c} &= \text{critical depth, ft} \\ \mathbf{S} &= \text{slope in percent} \\ \mathbf{h} &= \text{element height, ft} \\ \\ \mathbf{h} &= 1.41/(3\text{-}3.7\text{*}0.25)^{2/3} \\ &= 0.86 \text{ ft} \\ &= 10.4 \text{ in} \end{split}$$

h_{provided} = 12.00 in

 $h_{provided} > h$ , so the design is adequate.

#### 5. Calculate the total length of roughness elements.

L = spacing between the roughness elements, ft L  $_{Total}$ = Total length of roughened section, ft (Reference 3) L/h = 8.5 to 10 (L/h = 9.25 average) L = 9.25 h = 9.25 * 0.85 = 8.0 ft

The spacing and height of the roughness elements are designed based on 5 rows of roughness elements. (reference 3)

So,  $L_{\text{total (recommended)}} = 5L$ = 5*7.9 ft = 39.9 ft

 $L_{total(provided)} = 40 \text{ ft}$ 

 $L_{total(provided)} > L_{total(recommended)}$ , so the design is adequate.

The following table summarizes the calculations for FML chutes.

Upper Portion of Chutes

Chute	¹ Q	W _{Design}	q	n-value	Bottom Slope	Side Slope	Normal Depth	Flow Velocity
	(cfs)	(ft)	(cfs/ft)		(ft/ft)	(ft/ft)	(ft)	(ft/sec)
LD4	152.0	8	19.00	0.01	0.250	2	0.43	39.55
LD5	152.0	8	19.00	0.01	0.250	2	0.43	39.55
LD6	92.0	8	11.50	0.01	0.250	2	0.32	32.98

Lower Portion of Chutes

Chute	¹ Q	W _{Design}	q	n-value	Bottom Slope	Side Slope	Normal Depth	Flow Velocity	Y _c	V _c	h	L (=9.25h)	h _{Design}	² L _{Total} (Recommended)	W _{Provided}	h _{Provided}	L _{Total} (Provided)
	(cfs)	(ft)	(cfs/ft)		(ft/ft)	(ft/ft)	(ft)	(ft/sec)	(ft)	(fps)	(ft)	(ft)	(in)	(ft)	(ft)	(in)	(ft)
LF4	152.0	16	9.50	0.055	0.250	3	0.79	10.54	1.41	6.74	0.86	8.0	10.4	39.9	16	12.0	40.0
LF5	152.0	16	9.50	0.055	0.250	3	0.79	10.54	1.41	6.74	0.86	8.0	10.4	39.9	16	12.0	40.0
LF6	92.0	10	9.20	0.055	0.250	3	0.59	8.84	1.38	6.67	0.85	7.8	10.1	39.1	10	12.0	40.0

1. The flowrates were reproduced from Appendix H-2.

2. Total length of the roughened section was calculated based on FHWA recommendation of 5 rows of roughened elements.

### 5. Side Anchor Trench Design



Shear force pulling on geomembrane due to water:

The shear force acting on the geomembrane per square foot of water in the chute:

$T = \gamma_w x D x S$	where:	$\gamma_{\rm w}$ = unit weight of water (lb/cf)
		D = maximum water depth (ft)
		S = hydraulic gradient (ft/ft)

Shear force acting on the geomembrane per foot of anchor trench:

 $F_{s1} = T \times P$ 

where:

$$P = \text{ wetted perimeter of the chute} = (W + 2 \text{ x } (a^2 + D^2)^{1/2})$$
  

$$a = h \text{ x } D = \text{ horizontal distance from bottom of chute to the depth}$$
  
submerged on the sideslopes  

$$h = \text{ Slope of sidewalls} = 2 \qquad (H) : 1 (V)$$
  

$$W = \text{ Minimum bottom width of flow} = 8 \qquad ft$$

Conservatively, the maximum calculated water depth in the chutes will be used to verify the design. Thus, the water depth in the narrowest part of the chute with the highest depth will be used.

Letdown	Maximum	Hydraulic			
	Water Depth	Gradient	Т	а	F _{s1}
	$(\mathrm{ft})^1$	(ft/ft)	(lb/sf)	(ft)	(lb/ft)
LD4	0.43	0.25	6.71	0.86	67
LD5	0.43	0.25	6.71	0.86	67
LD6	0.32	0.25	4.99	0.64	47

¹ See design depths on page H-3-9.

Pullout Resistance from Edges, Fatl

Assuming pullout only opposed by trench (conservative assumption)



 $F_{at} = 2[\{K_{o}\gamma(D/2)\}\{tan\zeta\}\{D\} + \{\gamma D\}\{tan\zeta\}\{w\}]$ 

where: $\zeta = \text{interface fric}$ $K_o = 1 - \sin \zeta$ $\gamma = \text{unit weight of}$ D = depth of ancl w = bottom width	) ft) trench (ft)	(Ref 3)	
soil friction angle =	15	degrees	
soil/geomembrane friction angle =	15	degrees	

unit weight =	120	lb/ft ³
depth of anchor trench =	1	ft
bottom width of anchor trench =	1	ft

²See detail D6 - Anchor Trench Type 2 on Figure H-1-6 for dimensions.

$$K_{o} = 0.74$$

$$F_{at1} = 88 \quad lb/ft \text{ width on one side}$$

$$Factor of Safety = 2F_{at1}/F_{s1} = \frac{176}{67} \quad FS = 2.6$$

### 6. Upstream End Anchor Trench Design

Shear force pulling on geomembrane due to water:

$$F_{s2} = T x A$$

where:	T = Maximum shear force acting on the geomembrane per square foot of water in the chute (lb/sf)
	A = area of geomembrane at the top of the chute ( $ft^2$ )

Area of geomembrane at top of chute = 116 ft x 17 ft = 1,972 sf

Conservatively, use the maximum shear force per square foot calculated in Part 2

Pullout resistance of upstream end,  $F_{at2}^{3}$ 



³See detail D6 - Anchor Trench Type 1 on Figure H-1-6 for dimensions.



Factor of Safety = $F_{pr}/F_{s2}$ =	14,851	FS =	1.1
	13,228		

⁴See detail D5 on Figure H-1-5 for total of anchor trench length ( $L_T$ =[14+117+14]feet) for Pullout resistance of upstream end.

### **Summary of Results**

Side Anchor Trench Pullout resistance:

$$FS = \frac{2F_{AT2}}{F_{S1}} \implies FS = 2.6$$

Upstream End Anchor Trench Pullout resistance:

$$FS = \underbrace{F_{pr}}_{F_{s2}} = FS = 1.1$$

As it is stated on page 557 of Reference two, the typical factors of safety for the proposed anchor trenches are between 0.7 to 5.0. Therefore, the design is acceptable.

#### Example Calculation: Calculate the normal depth of the chute for drainage area LD4.

List of Symbols

- $Q_d$  = design flow rate for channel, cfs
- R = hydraulic radius, ft
- n = Manning's roughness coefficient
- S = channel slope, ft/ft
- b = bottom width of channel, ft
- z = z-ratio (ratio of run to rise for channel sideslope)
- $A_f = flow area, sf$
- $g = gravitational acceleration = 32.2 \text{ ft/s}^2$
- T = top width of flow, ft
- d = normal depth of chute, ft

The program uses an iterative process to calculate the normal depth of the chute to satisfy Manning's Equation

$$Q = \underbrace{1.486}_{n} A R^{0.67} S^{0.5}$$

Design Inputs:

$Q_d =$	152.0	cfs
S =	0.25	ft/ft
b =	8	ft
z =	2	(H):1(V)
n =	0.01	

Step 1 - Based on the geometry of the chute cross-section, solve for R and  $A_{\rm f}$ 

$$R = \frac{bd + zd^2}{b + 2d(z^2 + 1)^{0.5}}$$

$$A_f = bd + zd^2$$
assume:
$$d = \frac{0.43}{ft}$$

$$R = 0.387$$

$$ft$$

$$A_f = 3.84$$

$$solve for Q:$$

$$Q = 152.0$$

$$cfs$$

if Q is not equal to Q_d, select a new d and repeat calculations

Step 2 - solve for velocity, T, Froude number, velocity head, and energy head

Q = VA => V = Q/A  
V = 39.55 ft/s  
T = b + 2(z x d)  
T = 9.73 ft  

$$F_r = \frac{V}{(gA/T)^{0.5}}$$
  
 $F_r = 11.098$ 

Velocity Head = 
$$\frac{V^2}{2g}$$

Velocity Head = 24.31 ft

Energy Head = water elevation + velocity head

Energy Head = 
$$24.75$$
 ft



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#### NOTES:

1. EXISTING CONTOURS AND ELEVATIONS PROVIDED BY HYDREX ENVIRONMENTAL COMPILED FROM AERIAL PHOTOGRAPHY FLOWN 01-25-2023.

CHUTE DRAINAG	E AREA INFORMATION
DRAINAGE AREA	AREA (ACRES)
LD4	21.29
LD5	23.14
LD6	13.63



PREPARED FOR	TIER III PERMIT MODIFICATION					
MANAGEMENT OF OKLAHOMA, INC.						
REVISIONS	CHUIE DRAINAGE AREAS					
ATE DESCRIPTION						
	MUSKOGE MUSKOGEE	E COMMUNITY RDF COUNTY, OKLAHOMA				
	WWW.WCGRP.COM	FIGURE H-3-21				

## **APPENDIX H-4**

## POST-DEVELOPMENT PERIMETER CHANNEL, STORMWATER DETENTION POND, AND CULVERT DESIGN

Includes Pages H-4-1 through H-4-10



## CONTENTS

PERIMETER CHANNEL DESIGN		H-4-1
PERIMETER CHANNEL EROSION CONTROL D	ESIGN	H-4-4
CULVERT DESIGN		H-4-5
STORMWATER DETENTION POND DESIGN	Charles R. Marsh 24599 10/13/2023	H-4-7

## PERIMETER CHANNEL DESIGN

This appendix evaluates the adequacy of the perimeter channels proposed at the site. The perimeter channels are designed to carry run-off from the landfill area and run-on from the offsite area to the stormwater ponds and discharge points.

- Perimeter Channel Design Summary:
  - Channel drainage area analyzed is shown on Figure H-1-1. Details of the channel are shown on Figure H-4-2.
  - Perimeter channel design information and calculations are presented on page H-4-3.
  - The maximum normal water depth and channel bottom width is provided on page H-4-3.
  - Calculations on page H-4-3 show the maximum flow velocity in the perimeter channel.
  - The perimeter channel was designed to convey the run-off generated by the 25-year, 24-hour frequency storm event.
  - Perimeter Channel Erosion Control Design is included on page H-4-4.





#### NOTES:

1. EXISTING CONTOURS AND ELEVATIONS PROVIDED BY HYDREX ENVIRONMENTAL COMPILED FROM AERIAL PHOTOGRAPHY FLOWN 01-25-2023.



TIER III PEI	RMIT MODIFICATION
] PERIMETER	R DRAINAGE PLAN
MUSKOGE MUSKOGEE	E COMMUNITY RDF COUNTY, OKLAHOMA
WWW.WCGRP.COM	FIGURE H-4-2
	TIER III PEI PERIMETEF MUSKOGE MUSKOGEE WWW.WCGRP.COM

#### MUSKOGEE COMMUNITY RDF 0086-364-11-19 APPENDIX H PERIMETER CHANNEL DESIGN HYDRAULIC ANALYSIS

Channel / Pond	Stat	tion	Flow Rate ³ (cfs)	Bottom Slope (ft/ft)	Bottom Width (ft)	Side Slopes (ft/ft)	Manning's N-Value	Normal Depth (ft)	Flow Vel. (fps)	Froude No.	Vel. Head (ft)	Energy Head (ft)	Flow Area (sq.ft.)	Top Width of Flow (ft)
CHD-1	30+45	28+44	169	0.006	10	3.0	0.04	2.43	4.03	0.543	0.25	2.68	41.97	24.57
CHD-2	28+44	03+71	169	0.01	10	3.0	0.04	2.13	4.84	0.689	0.36	2.49	34.92	22.78
CHD-3	03+71	01+77	317	0.008	10	3.0	0.04	3.10	5.31	0.647	0.44	3.53	59.73	28.58
CHD-4	01+77	00+00	317	0.23	10	3.0	0.04	1.29	17.66	3.097	4.85	6.14	17.95	17.76

Note:

1. Calculations were performed using the HYDROCALC Computer Program developed by Dodson and Associates (Version 1.2a, 1996).

2. n = 0.04 (Manning's coefficient) is used for the calculations.

3. Flow rate obtained from HEC-1 analysis included in Appendix H-2.

4. Geometries listed in the table are average values approximated along the length of the channel.

## PERIMETER CHANNEL EROSION CONTROL DESIGN

Channel erosion controls have been designed for flow velocities resulting from the 25-year, 24-hour storm frequency flow rates. As shown on page H-4-3, the maximum velocity for channel CHD is 17.61 fps for the 25-year storm event. The channel lining needed to protect against erosive velocities is detailed below.

Typical channel lining material needed to protect against erosive velocities is indicated below.

- Vegetation used in all areas where velocities are less than 5 fps for channels.
- Turf Reinforcement Matting used in channels for velocities between 5 fps and 10 fps.
- Riprap used in channel sections with velocities more than 10 fps will be lined with riprap. 18-inch rock riprap will be used at locations where chutes discharge into the channels and stormwater ponds.

## **CULVERT DESIGN**

This section evaluates the adequacy of the proposed stormwater culvert at the site as shown on Figure H-4-2. HEC-1 was utilized to design and analyze the culverts. HEC-1 output is included in Appendix H-2.

#### Required: Design culverts to convey the flow.

Method: Use HYDROCALC Hydraulics for Windows computer program to determine number and size of the culverts. Use total 25-year frequency storm event flow estimated by HEC-1 included in Appendix H-2.

For proposed 1 X 48" CMP culvert at downstream end of "POND 9", Culvert "1"

Total Flow=	122 cfs
No. of Culverts=	1
Culvert Span=	inches
Culvert Rise=	inches
Culvert Diameter=	48 inches

Culvert ID	Culvert Span	Culvert Span	FHWA Chart Number	FHWA Scale Number	Culvert Diameter	Manning's Coefficient	Entrance Loss Coefficient	Culvert Length	Downstream Invert Elevation	Upstream Invert Elevation	Flow Rate	Tailwater Depth ²	Headwater Inlet Control	Headwater Outlet Control	Normal Depth	Critical Depth	Depth at Outlet	Outlet Velocity
	(ft)	(ft)			(ft)			(ft)	(ft msl)	(ft msl)	(cfs)	(ft)	(ft)	(ft)	(ft)	(ft)	(ft)	(fps)
1			2	2	4	0.024	0.8	175.00	609.30	610.70	140.00	1.91	7.39	7.75	4.00	3.32	3.32	10.94

1. Calculations were performed using the HYDROCALC Hydraulics for Windows program developed by Dodson and Associates (Version 2.0, 1996-2010).

2. Tailwater depth is assumed to be the 25-year, 24-hour normal depth in the channel right after the culvert.



## STORMWATER DETENTION POND DESIGN

The stormwater detention ponds have been analyzed using HEC-1 storage routing method. The input parameters for the models are presented in Appendix H-2. A summary of the HEC-1 results are presented on page H-4-8. As shown on page H-4-8, during the 25-year storm event, Ponds 8 and 9 are not expected to have flow over their spillways.

#### MUSKOGEE COMMUNITY RDF 0086-364-11-19 APPENDIX H STORMWATER DETENTION POND DESIGN

- **Purpose:** Demonstrate that the detention pond outlet structure designs are adequate to convey runoff from the various subbasins to their discharge points.
- Method: 1. Use the 25-year, 24-hour flow rates and water surface elevations for the drainage areas that will discharge to each detention pond from the HEC-1 analysis (see Appendix H-2).
  - 2. Use the Weir Equation to calculate the flow rate over the spillways as appropriate.

#### Solution:

25-Year 24-Hour Storm	Pond 7	Pond 8	Pond 9
Bottom ELEV, ft	605.0	561.0	610.7
Spillway ELEV, ft	621.0	617.0	630.3
Spillway Length, ft	189	200	10
Top of Road/Berm, ft			
Discharge Pipe Downstream Invert ELEV, ft			609.3
Peak Inflow Q ₂₅ , cfs	343	642	140
Peak Outflow Q ₂₅ , cfs	0	0	122
Peak Stage in Pond, ft	616.43	573.11	613.61
Est. Flow (Q ₂₅ ) over Spillway, cfs			
Velocity (V ₂₅ ) over Spillway, fps			

Note:

⁽¹⁾ Calculations for velocity over the spillway were performed using the HYDROCALC HYDRAULICS FOR WINDOWS Computer Program developed by Dodson and Associates (Version 1.2a, 1996).

Solution:

#### MUSKOGEE COMMUNITY RDF 0086-364-11-19 APPENDIX H EROSION PROTECTION CALCULATIONS

Requried:	Determine the minimum length and median diameter of riprap required at the detention
	pond and channel outlet structures to control erosion.

 
 Reference:
 1. Haan, Barfield, and Hayes, Design Hydrology and Sedimentology for Small Catchments, 1994.

2. Dodson's and Associates, Inc., ProHec-1 Plus Program Documentation, 1995.

The riprap will be designed for the 25-year flow rates at the detention pond and channel outlet structures. The flow at the outlet structure can be divided into two categories:

#### 1. Flow over the Spillway

As shown on page H-4-8, detention pond 9, is not expected to have flow over the spillway.

#### 2. Flow through the Low Water Outlet

The flow rate through the low water outlet (LWO) is summarized below.

	Pond	LWO Invert Elev.		LWO	25-Year	25-Year Outlet
Flow	Bottom Elev	Upstream	Downstream	Diameter	Flow Rate	Velocity ¹
Structure	(ft-msl)	(ft-msl)	(ft-msl)	(in)	(cfs)	(ft/s)
P9	610.7	610.7	609.3	48	122	10.94

¹ Velocities through the low water outlet were calculated using the HYDROCALC HYDRAULICS FOR WINDOWS program developed by Dodson and Associates (Version 1.2a, 1996).

² The flow rates were obtained from the HEC-1 analysis output.

The nomograph used for design of the length of the riprap and the median diameter is shown on pages H-4-10 (Figures 5.24 and 5.25).

The minimum riprap length and median rock diameter for each outlet is summarized below. The length of the riprap is increased by 20 percent to provide for a conservative design.

Pond	Flow in LWO	Riprap Length	Adjusted Length L x 1.2	Median Rock Diameter
	(cfs)	(ft)	(ft)	(ft)
P9	122	28	33.6	1.00

Apron width (W) for ponds LWO: W = LWO diameter + 0.4 * (Adjusted RipRap Length)

	W
Pond	(ft)
P9	15.9

The median diameter of riprap is intended to determine the minimum diameter of the riprap that will be used. As an alternative, 16-inch thick gabions can be used.
#### 5. Hydraulics of Structures



Figure 5.24 Design of outlet protection—minimum tailwater condition,  $T_w < 0.5D$  (Environmental Protection Agency, 1976).



Figure 5.25 Design of outlet protection—maximum tailwater condition,  $T_w \ge 0.5D$  (Environmental Protection Agency, 1976).

into the riser 3 ft below its top, what discharge will pass through the four holes with the water level at 1, 2, 4, and 8 ft above the riser? (c) What is the total discharge through the pipe? (d) How might the orifices be sized to provide better stormwater control? (e) Explain whether you would expect two rows (each consisting of four holes) of 8-in.-diameter holes to provide better results? Assume that one row is 2 ft below the riser invert and the other row is 4 ft below the riser invert. (5.6) A gravel roadway is constructed in a low-lying area such that the roadway is frequently overtopped as a result of severe storms. The roadway is 40 ft wide, and its elevation is 36 ft. (a) If the water level upstream of the roadway is 2 ft above the crest of the roadway, what is the discharge across the roadway? (b) If the roadway is paved, what upstream depth would be required to carry the same flow? (c) Would paving reduce flooding problems?

#### **APPENDIX H-5**

## POST-DEVELOPMENT FINAL COVER SOIL LOSS CALCULATIONS

Includes Pages H-5-1 through H-5-12



#### CONTENTS

#### FINAL COVER SOIL LOSS CALCULATIONS

H-5-1



#### FINAL COVER SOIL LOSS CALCULATIONS

This appendix presents the supporting documentation for evaluation of the thickness of the vegetative and support layer for the ET final cover system at the Muskogee Community RDF. The evaluation is based on the premise of adding excess soil to increase the time required before maintenance is needed as recommended in the EPA Solid Waste Disposal Facility Criteria Technical Manual (EPA 530-R-93-017, November 1993).

The design procedure is as follows:

- 1. Minimum thickness of the vegetative layer at the end of the 30-year postclosure period is evaluated based on the depth of frost penetration or 6 inches, whichever is greater. For Muskogee County, the approximate depth of frost penetration is approximately 8 inches. The minimum vegetative and support layer thickness for the ET final cover system is 36 inches.
- 2. Soil loss is calculated using the Universal Soil Loss Equation (USLE) by following SCS procedures. The soil loss is adjusted by a safety factor of 2 and is then converted to a thickness. The thickness of the soil loss over a 30-year postclosure period is added to the minimum thickness of the vegetative and support layer (from Step 1) to yield an initial thickness to be placed at closure of the site. According to the USLE, the typical topslope and sideslope require 8.0 inches and 9.1 inches, respectively, for the vegetative and support layer. These USLE requirements include the 8-inch minimum required by regulations. Conservatively, a 36-inch vegetative and support layer is proposed for the ET final cover system. These calculations begin on page H-5-2.
- 3. Vegetation for the site will consist of native and introduced grasses with root depths of up to 48 inches. Native and introduced grasses will be hydroseeded with fertilizer on the disked (parallel to contours) vegetative layer upon final grading (or an equipment method). Temporary cold weather vegetation will be established if needed. Irrigation will be employed for 6 to 8 weeks or until vegetation is well established. Vegetative support control measures such as silt fences and straw bales will be used to minimize vegetative support until the vegetation is established. Areas that experience vegetative support or do not readily vegetate after hydroseeding will be reseeded until vegetation is established or the soil will be replaced with soil that will support the grasses.

#### MUSKOGEE COMMUNITY RDF 0086-364-11-19 APPENDIX H PROPOSED VEGETATIVE LAYER EVALUATION

<u>Required:</u>	Determine expected soil loss and minimum thickness for the vegetative and support layer.				
<u>Method:</u>	Expected soil loss is calculated using the Universal Soil Loss Equation. Minimum vegetation and support layer thickness is determined by adding the minimum thickness allowed by the regulations to the expected soil loss.				
References:         1           2         3           4         4	<ul> <li>Control of Water Pollution from Corplands . B.A. Stewart, D.A. Woolhiser, W.H.</li> <li>Wischmeier, J.H. Caro and M.H. Frere, EPA-600/2-75-026, Environmental Protection Agency.</li> <li>Use of the Universal Soil Loss Equation in Final Cover/Configuration Design, Procedural Handbook, Texas Natural Resources Conservation Commission, 1993.</li> <li>United States Department of Agriculture, Soil Conservation Service, Soil Survey of Muskogee County, Oklahoma, 1992.</li> <li>United States Environmental Protection Agency, Solid Waste Disposal Facility Criteria Technical Manual, 1993.</li> </ul>				
Solution:	1. Soil Loss Equation:	A=RKL _s CP			
	Where:	A= Soil loss (tons/ac/yr) R= Rainfall factor K= Soil erodibility factor L _s = Slope length/slope gradient factor C= Plant cover or cropping management factor P= Erosion practice factor			
	The rainfall factor, R, represents the avintensity, 30 minute storms over a 22 the SCS. Using Figure 5.3 on Sheet H of the R Factor, the R factor for Musko	verage intensity for the maximum vear period of record compiled by 5-11 (Ref 2), Average Annual Values ogee County is:			
	$\mathbf{R} =$	270			
	The soil erodibility factor, K, factor repersion as a function of the soil's physical sandy loam with organic matter contents.	presents the resistance of a soil surface to cal and chemical properties. Assume a t of 2% to determine the K on Sheet H-5-9 (Ref 2).			
	K =	0.24			
	The slope length/slope gradient factor, both slope length and degree of slope.	$L_s$ , represents the erosion of the soil due to The slopes of interest are shown on Sheet H-5-5.			
	Location 1. Typical topslope				
	slope = $5$ length = $200$	% ft			
	Location 2. Typical sideslope slope = 25	%			
	length = $120$	ft			
	Location 3. Longest topslope				
	slope = 5	%			
	length = 300	n			
	Location 4. Longest sideslope	pl			
	slope = $25$	% ft			
	engin = 125	n n			

#### MUSKOGEE COMMUNITY RDF 0086-364-11-19 APPENDIX H PROPOSED VEGETATIVE LAYER EVALUATION

#### Using the above information, the $L_{\rm s}$ factors were determined.

Case	Slope (%)	Slope Length (ft)	L _s
1. Typical topslope	5	200	0.75
<ol><li>Typical sideslope</li></ol>	25	120	6.50
<ol><li>Longest topslope</li></ol>	5	300	0.85
4. Longest sideslope	25	125	6.80

The plant cover or cropping management factor, C, represents the percentage of soil loss that would occur if the surface were partially protected by some combination of cover and management practices. C Factor for Permanent Pasture, Range, and Idle Land with No Appreciable Canopy has the following relation with percent ground cover (GC) (Ref 2).

% GC	C Factor:
0	0.45
20	0.20
40	0.10
60	0.042
80	0.013
95	0.003



Note: Vegetative layer will be maintained to provide 85% ground cover.

C Factor= 0.5837e^(-0.0502x85)

C Factor= 0.0082

The erosion control practice factor, P, measures the effect of control practices that reduce the erosion potential of the runoff by influencing drainage patterns, runoff concentration , and runoff velocity. Contouring for this site will be done only to establish vegetation.



#### MUSKOGEE COMMUNITY RDF 0086-364-11-19 APPENDIX H PROPOSED VEGETATIVE LAYER EVALUATION

#### 2. Soil loss calculations

Slope Condition	R	К	L _s	С	Р	A (tons/ac/yr)
1. Typical Topslope 5 % slope 200 ft length	270	0.24	0.75	0.0082	1.0	0.4
2. Typical Sideslope 25 % slope 120 ft length	270	0.24	6.50	0.0082	1.0	3.4
3. Longest Topslope 5 % slope 300 ft length	270	0.24	0.85	0.0082	1.0	0.5
4.Longest Sideslope 25 % slope 125 ft length	270	0.24	6.80	0.0082	1.0	3.6

Note: Vegetative layer will be maintained to provide 85% ground cover.

3. Vegetative and support layer thickness calculations:

$T_{el} = 8 in +$	AYF(2000lb/	ton)(12in/ft)			
	w(43,560sf/ac	c)			
Where:	T _{el} =	Vegetative and sup	port layer thickness		
	A =	Soil loss (ton/ac/yr)			
	Y =	Postclosure period (yr)			
	F =	Factor of Safety			
	w =	Specific weight of	soil (pcf)		
	Y =	30	yr		
	$\mathbf{F} =$	2			
	w =	110	pcf		

1. Typical Topslope Thickness:	1. Typical Topslope Thickness:						
Required thickness(in) ¹ = Specified thickness(in) =	8.1 36						
2. Typical Sideslope Thickness:							
Required thickness (in) ¹ = Specified thickness (in)=	9.0 36						
3. Longest Topslope Thickness:							
Required thickness (in) ¹ = Specified thickness (in)=	8.1 36						
4. Longest Sideslope Thickness:							
Required thickness (in) ¹ = Specified thickness (in)=	9.1 36						

Note: ¹Required thicknesses include 8 inch minimum required.

4. Summary:

As discussed above, the vegetation and support layer will be a minimum of 36 inches thick. As shown above, this is a conservative design considering the maximum expected soil loss for a 30 year period is 1.1 inches, and the maximum required thickness is 9.1 inches.





#### NOTES:

DATE

1. EXISTING CONTOURS AND ELEVATIONS PROVIDED BY HYDREX ENVIRONMENTAL COMPILED FROM AERIAL PHOTOGRAPHY FLOWN 01-25-2023.

TYPICAL SLOPE	E LENGTHS		LONGEST SLO	OPE LENGTHS		
SLOPE LENGTH I	ENGTH (FEET)	S	OPE LENGTH	LENGTH (FEET)		
TOPSLOPE SIDESLOPE	200.0 120.0		TOPSLOPE SIDESLOPE	300.0 150.0		
PREPARED FOR	PROFESSION OF CHARMEN	SSION arles R. arsh 4599 NHON D/13/20	EIGINE 223			
EMENT OF OKLAHOMA, INC			RMIT MO			
PEVISIONS	SL			UPE LENGINS FUR SUIL		
			CATION			
	м	MUSKOG USKOGEE	COUNTY, O	TY RDF KLAHOMA		
	1					

WWW.WCGRP.COM

FIGURE H-5-5

United States Environmental Protection Agency Solid Waste and Emergency Response (5305) EPA530-R-93-017 November 1993 www.epa.gov/osw



# Solid Waste Disposal Facility Criteria

**Technical Manual** 



TEXAS NATURAL RESOURCE CONSERVATION COMMISSION

# USE OF THE UNIVERSAL SOIL LOSS EQUATION IN FINAL COVER/CONFIGURATION DESIGN

# PROCEDURAL HANDBOOK

# PERMITS SECTION MUNICIPAL SOLID WASTE DIVISION

OCTOBER 1993

# Table 1 Approximate Values of Factor K for USDA Textural Classes

			Organic Matter Content				
	Texture Class	< 0.5%	. 2%	4%			
		K	K	K			
	Sand	0.05	0.03	0.02			
	Fine Sand	0.16	0.14	0.10			
	Very Fine Sand	0.42	0.36	0.28			
	Loamy Sand	0.12	0.10	0.08			
	Loamy Fine Sand	0.24	0.20	. 0.16			
	Loamy Very Fine Sand	0.44	0.38	0.30			
	Sandy Loam	0.27	0.24	0.19			
·	Fine Sandy Loam	0.35	0.30	0.24			
×.,	Very Fine Sandy Loam	0.47	0.41	0.33			
	Loam	0.38	0.32	0.29			
	Silt Loam	0.48	0.42	0.33			
	Silt	0.60	0.52	0.42			
	Sandy Clay Loam	0.27	0.25	0.21			
	Clay Loam	0.28	. 0.25	0.21			
	Silty Clay Loam	0.37	0.32	0.26			
	Sandy Clay	0.14	0.13	0.12			
	Silty Clay	0.25	0.23	0.19			
	Clay		0.13 - 0.29				

The values shown are estimated averages of broad ranges of specific-soil values. When a texture is near the borderline of two texture classes, use the average of the two K values.

6

Vegetative Canopy		Cover that contacts the soil surface					
Type and	Percent cover ³		· . · Percent ground cover				
height ²		0	20	40 ·	60	80	95+
No Appreciable Canopy		0.45	0.20	0.10	0.042	0.013	0.003
Tall weeds or	25	0.36	0.17	0.09	0.038	0.013	Q.011
average drop	50	0.26	0.13	0.07	0.035	0.012	0.003
in.	75	0.17	0.10	0.06	0.032	0.011	0.003

#### Table 2 Factor C for permanent pasture, range, and idle land¹

Extracted from:

1

2

United States Department of Agriculture, AGRICULTURE HANDBOOK NUMBER 537

The listed C values assume that the vegetation and mulch are randomly distributed over the entire area.

Canopy height is measured as the average fall height of water drops falling from the canopy to the ground. Canopy effect is inversely proportional to drop fall height and is negligible if fall height exceeds 33 ft.

³ Portions of total-area surface that would be hidden from view by canopy in a vertical projection (a bird'seye view).

7





H-5-12

FIGURE 4.—Slope-effect chart (topographic factor, LS). LS =  $(\lambda/72.6)^{m}$  (65.47 sin² $\theta + 4.56$  sin  $\theta + 0.065$ ) where  $\lambda = s$  lope length in feet;  $\theta = angle of slope;$  and m = 0.2 for gradients < 1 percent, 0.3 for 1 to 3 percent slopes, 0.4 for 3.5 to 4.5 percent slopes, and 0.5 for slopes of 5 percent or steeper.

#### MUSKOGEE COMMUNITY RECYCLING AND DISPOSAL FACILITY MUSKOGEE COUNTY, OKLAHOMA ODEQ PERMIT NO. 3551020

#### APPENDIX I USACE INFORMATION

Prepared for

Waste Management of Oklahoma, Inc.

October 2023



Prepared by

#### Weaver Consultants Group, LLC

CA 3804 PE – 06/30/2025 6420 Southwest Boulevard, Suite 206 Fort Worth, Texas 76109 817-735-9770

WCG Project No. 0086-364-11-19

#### CONTENTS

USACE Individual Permit Summary	I-1
Excerpts from the September 2023 Section 404 Individual Permit	I-2
JONATHAN V. B QUEEN 24456 01/L A HOWN 10/13/2023	

#### **USACE INDIVIDUAL PERMIT SUMMARY**

The USACE is given the authority under Section 404 of the Clean Water Act to issue permits for all construction activities that affect the nation's waters and wetlands, including dredging or placement of fill material into or adjacent to Waters of the United States (U.S.). The regulatory program that governs these permits is focused on protecting the nation's aquatic resources, while allowing reasonable development. Therefore, as part of the expansion process, a determination of Section 404 Jurisdictional Waters of the United States was performed to delineate the waters of the U.S. and wetlands within the permit boundary.

A determination indicated the presence of jurisdictional waters onsite. WMO evaluated several options to avoid and minimize impacts to the jurisdictional waters. The option selected balances long-term solid waste disposal needs in the City of Muskogee and surrounding areas while minimizing the impact to existing jurisdictional waters. The selected alternative placed an emphasis on (1) benefitting the community and local industry by providing a much-needed basic service in the form of an environmentally safe, solid waste disposal, (2) compatibility with immediately adjacent land uses, with the expanded landfill being located immediately west of the existing landfill, as well as compatibility with nearby land uses that include agricultural, rangeland and low to very low-density rural residential areas, (3) incorporating perimeter drainage features that will collect, convey and control stormwater and sediment on the project site while providing water quality improvements prior to off-site discharge of stormwater to downstream receiving waters, (4) utilizing established roadways and traffic patterns sufficient to handle waste management traffic, and (5) the creation of additional jurisdictional waters through a USACE approved in-lieu fee program such that there will be no net loss of Waters of the US with the project.

Section 404 Individual Permit Request was submitted to the USACE in September 2023. Portions of the USACE Section 404 Individual Permit is included in this appendix.

### EXCERPTS FROM THE SEPTEMBER 2023 SECTION 404 INDIVIDUAL PERMIT APPLICATION

### MUSKOGEE COMMUNITY RECYCLING AND DISPOSAL FACILITY (RDF) MUSKOGEE COUNTY, OKLAHOMA

APPENDIX F MITIGATION PLAN

Prepared for

Waste Management of Oklahoma, Inc.

August 2023

Prepared by

Weaver Consultants Group, LLC 6420 Southwest Boulevard, Suite 206 CA 3804 PE 06/30/2025 Fort Worth, Texas 76109 817-735-9770

WCG Project No. 0086-364-11-20

#### CONTENTS

1	INTF	RODUCTION	1
	1.1	Location	1
	1.2	Proposed Landfill Development	1
	1.3	Site Background	2
	1.4	Site Conditions	2
	1.5	Surrounding Land Use	3
2	MIT	IGATION PLAN OBJECTIVES	4
	2.1	Overview	4
	2.2	Mitigation Type and Amount	4
	2.3	Party Responsible for Compensatory Mitigation	4
	2.4	Timing and Sequence of Development	4
	2.5	Financial Assurance	5
	2.6	Compliance with Compensatory Mitigation Criteria	5
3	SITE	SELECTION	8
	3.1	Introduction	8
	3.2	Local, State, and Federal Regulations	8
	3.3	Ecological Suitability of the Compensatory Mitigation Plan	9
4	SITE	PROTECTION INSTRUMENTS	10
5	BASI	ELINE INFORMATION	11
	5.1	Wetland and Waters of the U.S. Information	11
	5.2	Threatened or Endangered Species	11
6	DET	ERMINATION OF CREDITS	13
7	MIT	IGATION WORK PLAN	14
	7.1	Mitigation Areas	14
		7.1.1 Mitigation Centers	14
	7.2	Sequence of Development and Storm Water Control	14
		7.2.1 Perimeter Drainage System	14
		7.2.2 Storm Water Controls During Below Grade Fill	15
		7.2.3 Storm Water Controls During Aerial Fill	15
		7.2.4 Erosion and Sedimentation Control	16
	7.3	Protection During Construction	16
	7.4	Planting Plan	17

# **CONTENTS (Continued)**

8	MAIN	TENAN	ICE PLAN	18	
	8.1 Consistency with Other Plans				
	8.2	Routir	ne Maintenance	18	
		8.2.1	Vegetation Management	18	
		8.2.2	Good Housekeeping Measures	18	
		8.2.3	Erosion Control Measures	19	
		8.2.4	Maintenance Program for Drainage Control Structures	19	
9	PERFO	ORMAN	NCE STANDARDS	21	
10	MONI	TORIN	G REQUIREMENTS	22	
11	LONG	-TERM	MANAGEMENT PLAN	23	
12	ADAP	TIVE M	IANAGEMENT PLAN	24	
13	FINAN	ICIAL A	ASSURANCE	25	

#### **1 INTRODUCTION**

#### 1.1 Location

The Muskogee Community Recycling and Disposal Facility (Muskogee Community RDF) is an existing non-hazardous municipal solid waste landfill located approximately four miles southwest of downtown Muskogee, Oklahoma. The proposed expansion area is located within Section 6 of Township 14 North, Range 18 East in Muskogee County, Oklahoma. The site is south of West 23rd Street S (West Hancock Street), west of South 54th Street W, and east of South 64th Street W outside of the City of Muskogee city limits. The site location is shown on Figure B-1 in Appendix B. The center of the landfill expansion is located approximately at latitude 35° 43' 14.18" and longitude 95° 26' 31.86".

The figures associated with this Mitigation Plan (plan) are included in Appendix B of this submittal, and include the following:

- Figure B-1 Site Location Map
- Figure B-2 Topographic Site Map
- Figure B-3 Land Use Map
- Figure B-4 Flood Insurance Rate Map (FIRM)
- Figure B-5 Landfill Excavation Plan

Additional figures for this plan presenting the Alternatives Analysis are included in Section 3 of this plan.

#### **1.2 Proposed Landfill Development**

Waste Management of Oklahoma, Inc. (WMO) proposes to expand the existing Muskogee Community RDF onto property located immediately west of the currently permitted property. The closed City of Muskogee Landfill abuts the currently permitted landfill footprint on the current landfill's western boundary, and the landfill expansion property is sited immediately west of the closed City of Muskogee Landfill property, with a parcel north of the closed City of Muskogee Landfill added to the permit boundary to facilitate access between the existing and proposed expansion properties. The existing permitted landfill and proposed landfill expansion area configuration are shown on Figure 3-4. The comparison of the permitted and proposed project area is listed in Table 1-1.

#### Table 1-1

#### **Project Area Information (Existing and Proposed Expansion Combined)**

Description	Existing Permitted Condition	Proposed Condition
Permit Boundary	80.5 acres	316.8 acres
Waste Disposal Area	60.9 acres	126.4 acres

Note that this plan addresses the newly added 236.3-acre western expansion property only, as the currently permitted property will not change from its currently permitted configuration for this effort. The evaluation of impacts to Waters of the United States (WATERS) as presented in the Waters of the US Delineation (Appendix A) is limited to the western expansion property only.

Figure 3-4 shows the proposed final configuration of the expanded western landfill property. For the proposed project, the final cover erosion control structures, ponds, and perimeter channels will be designed to effectively minimize erosion of final cover soils and increase detention of stormwater before it is discharged from the property (by infiltration or pumping to adjacent surface drainage features).

#### **1.3 Site Background**

Muskogee Community RDF was originally owned by the City of Muskogee, who operated the site from 1987 to 1992 under the name Tract A of the City of Muskogee Landfill. In 1992, the ownership of Tract A of the City of Muskogee Landfill was transferred to Waste Management of Oklahoma, Inc.

WMO is the holder of the Oklahoma Department of Environmental Quality (ODEQ) Permit No. 3551020. The facility accepts municipal solid waste and nonhazardous industrial solid waste (NHIW) in accordance with Title 252 of the Oklahoma Administrative Code, Chapter 515-Management of Solid Waste. WMO is a subsidiary of Waste Management, Inc., who operates solid waste management facilities across the US. The existing facility is located at 2801 S. 54th St. W., Muskogee, Oklahoma, 74401, in Muskogee County, Oklahoma. This location is approximately four miles southwest of downtown Muskogee.

## **1.4 Site Conditions**

The proposed expansion property is undeveloped rangeland that is currently used as a source of borrow soils for the ongoing landfill activities. Vegetation consists of native plants and shrubs. A portion of the property appears to be regularly hayed, with the remainder of the site dominated by woodlands and grasslands. A powerline right of way (ROW) traverses the northern portion of the property in an east-west direction, and a second powerline traverses the eastern half of the property in a north-south direction. Several water bodies and depressions are observed on the property. The proposed property is located east of an unnamed tributary of Pecan Creek and south of an unnamed tributary of Coody Creek. No portion of the property is located within a FEMA-designated floodplain.

A USGS Contour Map showing topographic contours of the property is provided as Figure B-2 and a Flood Insurance Rate Map (FIRM) is provided on Figure B-4 in Appendix B.

#### **1.5 Surrounding Land Use**

Land use of the adjacent properties is mostly closed trench landfill, undeveloped property, range land, agricultural property, commercial/light industrial, rural residential properties, and limited oil production. A Land Use Map is provided as Figure B-3 (Appendix B).

The use of this western expansion property as a municipal solid waste landfill represents a compatible land use for the following reasons.

- Landfill operations have occurred on the contiguous property adjacent to the expansion property since 1987, beginning with the City of Muskogee. The permit boundary also abuts the closed City of Muskogee Landfill, a compatible land use to the expanded landfill.
- Surrounding land uses are predominantly agricultural, rangeland, and low to very low density rural residential.
- Current traffic patterns and roadways are sufficient to handle the continued waste management traffic served by the landfill.

# 2.1 Overview

The objective of this plan is to provide the required information to support the issuance of an Individual Permit for this site. This plan has been developed to ensure that there will be no overall net loss of waters of the U.S. with this project. As detailed in subsequent sections of this plan, communication with various local state and federal agencies will be completed for this expansion. Approval will also be obtained from the ODEQ for the landfill prior to construction.

# 2.2 Mitigation Type and Amount

The Mitigation Plan for this site has been developed to ensure that there will be no overall net loss of waters of the U.S. with this project. A summary of the unavoidable impacts to jurisdictional waters from the proposed landfill expansion is shown on Figure B-5 (Appendix B). In addition, Table 2-1 has been developed to show compliance with 33 CFR §332.3(b).

# 2.3 Party Responsible for Compensatory Mitigation

The Muskogee Community RDF is owned and operated by Waste Management of Oklahoma, Inc. WMO is the permittee responsible for the implementation, performance, and long-term management of this project. WMO is a wholly-owned subsidiary of Waste Management, Inc. Waste Management is one of the leading providers of solid waste services in the nation. Waste Management provides non-hazardous waste collection, transfer, recycling, and disposal services to residential, municipal, and commercial customers across the country.

## 2.4 Timing and Sequence of Development

The expected active life of the landfill without the expansion has been estimated to be approximately 3.5 to 4 years. Therefore, the initial development of the expansion area will not likely begin for approximately 2 years. Large portions of the western expansion area could potentially be developed without disturbing jurisdictional waters, however, WMO intends to develop perimeter drainage features initially to support site operations and control stormwater. This perimeter drainage development will disturb portions of the on-site jurisdictional waters. Therefore, development of expansion area features that require disturbance of jurisdictional waters will not be initiated until mitigation activities are procured by WMO.

The proposed mitigation for this project will be completed off-site utilizing a US Army Corps of Engineers (USACE) approved in-lieu fee program. WMO will contract with the in-lieu fee program and notify the USACE once the off-site mitigation has been procured/confirmed. If the mitigation is not available through the in-lieu program, WMO will contact the USACE to discuss other alternatives.

#### **2.5** Financial Assurance

Financial assurance requirements for this project are detailed in Section 13. In summary, financial assurance for the mitigation activities and related site improvements, the closure of the landfill, and 30 years of post-closure care is required by ODEQ.

#### 2.6 Compliance with Compensatory Mitigation Criteria

Compliance with the Compensatory Mitigation Criteria included in 33 CFR §332.3(a) is summarized in Table 2-2.

# Table 2-1Type and Location of Compensatory Mitigation<br/>(Compliance with §332.3(b))

<b>Compensatory Mitigation Requirement</b>	Discussion
Mitigation located in same watershed as the improvements?	No. Mitigation activities will be completed through a USACE approved in-lieu fee program. As noted in §332.3(b), mitigation bank credits are a more preferred approach over onsite mitigation for compensatory mitigation. Offsite mitigation at an in-lieu fee program will be more successful, as noted below.
Mitigation located where most likely to successfully replace lost functions and services. Functions that were considered:	
<ul> <li>Aquatic habitat diversity and habitat connectivity</li> </ul>	A comprehensive mitigation project developed through an in-lieu fee program will allow for a more comprehensive increase in ecological functions and benefits for the environment than a smaller isolated onsite mitigation effort. The offsite mitigation activities through the in-lieu fee program will be more effective for successful ecological restoration as well as provide incrementally more habitats for many species of wildlife and plants.
Relationship to hydrologic source	Proposed perimeter drainage features will collect, convey, and control stormwater from upland areas around the site. Collected stormwater will be periodically pumped off site to adjacent receiving stormwater features (i.e., roadway drainage ditches or other).
<ul> <li>Trends in land use and compatibility with surrounding land use</li> </ul>	Land use is discussed in Section 1.5 above. The proposed development represents a compatible land use.
Ecological benefits	As noted above, a comprehensive mitigation projected developed through an in-lieu fee program will allow for a more comprehensive increase in ecological functions and benefits for the environmental.

# Table 2-2Compliance with Compensatory Mitigation Criteria (§332.3(a))

Item	How Requirement is Met
Likelihood for ecological success and sustainability	With the use of compensatory mitigation through an in-lieu fee program the likelihood for ecological success and sustainability is significantly greater than a smaller isolated onsite mitigation effort. This is based on the development, maintenance, and management of mitigation activities being completed by a mitigation sponsor who specializes in creation, restoration, enhancement, and preservation of wetlands and waters of the U.S.
Location of the compensation site relative to the impact site and their significance within the watershed	The unavoidable impacts on the project will not significantly impact the watershed of the project being that; (1) the area of impact is located at the headwaters of an unnamed tributary; (2) the area of impact is outside the 100-year floodplain; and (3) proposed development includes potential perimeter drainage features that will collect, convey, and control stormwater on the project site. Therefore, onsite mitigation would not provide significant benefits to the watershed. However, offsite mitigation, as discussed above, would provide significantly greater ecological success and sustainability.
Costs of the compensatory mitigation project	The estimated cost for this project is approximately \$800,000 to \$1,000,000.
Does mitigation provided offset unavoidable impacts?	Yes. This plan proposes to purchase credits from a USACE approved in-lieu fee program to offset the loss of jurisdictional waters of the U.S. that are impacted by this project. A total of 2.39 acres of wetlands and 2,076 linear feet of jurisdictional waters are proposed to be impacted as a part of this project.

#### **3 SITE SELECTION**

#### **3.1 Introduction**

WMO (Applicant) evaluated several options to avoid and minimize impacts to Section 404 Jurisdictional Areas located within the 236.3-acre landfill expansion permit boundary. A description of the evaluation process and alternatives considered are provided on Figure 3-1 through Figure 3-4. The selected alternative places a priority on:

- Benefitting the community and local industry by providing a much-needed basic service in the form of an environmentally safe, solid waste disposal facility.
- Compatibility with immediately adjacent land uses, with the expanded landfill being located immediately west of the existing landfill.
- Compatibility with nearby land uses that include agricultural, rangeland and low to very low-density rural residential areas.
- Utilizing established roadways and traffic patterns sufficient to handle waste management traffic.
- WMO's stated willingness to provide mitigation of jurisdictional waters impacted by the project.
- Incorporating perimeter drainage features that will collect, convey and control stormwater and sediment on the project site while providing water quality improvements prior to off-site discharge of stormwater to downstream receiving waters. Note that FEMA 100-year floodplains are not impacted by the project.

#### **3.2** Local, State, and Federal Regulations

The expansion of the Muskogee Community RDF will require a Tier III Modification to the ODEQ Solid Waste Permit. The modification will be developed to show that the design and operation of the Muskogee Community RDF expansion will be in compliance with Title 252 of the Oklahoma Administrative Code, Chapter 515 – Management of Solid Waste. This requires coordination or use of publicly available local, state, and federal resources to indicate compliance.

#### 3.3 Ecological Suitability of the Compensatory Mitigation Plan

This mitigation plan has been developed to ensure that there will be no overall net loss of waters of the U.S. with this project. As noted on Figure B-4 for the selected alternative, there is an unavoidable impact to 2.39 acres of wetlands and 2,067 linear feet of jurisdictional waters that will be mitigated by WMO obtaining credits from a USACE approved in-lieu fee program.

The creation of perimeter drainage channels and a sedimentation and storage ponds onsite will allow for the conveyance, collection, and control of stormwater and will improve water quality that discharges offsite. These features will also allow for native vegetation to establish which will increase the potential for nutrient assimilation.

Given the location of unavoidable impacts to the jurisdictional waters located (1) at the headwaters of an unnamed tributary; (2) the area of impact is outside the 100year floodplain; and (3) proposed development includes potential perimeter drainage features that will collect, convey, and control stormwater on the project site.



#### ALTERNATIVE 1-DO NOT EXPAND LANDFILL

THIS OPTION INCLUDES NOT EXPANDING THE CURRENT LANDFILL BEYOND THE CURRENTLY PERMITTED FOOTPRINT AND GRADES. THIS OPTION WAS ELIMINATED FROM CONSIDERATION FOR THE FOLLOWING REASONS:

- THE EXISTING LANDFILL HAS LESS THAN 4 YEARS OF DISPOSAL CAPACITY REMAINING. THIS ALTERNATIVE WOULD LEAVE THE CITY OF MUSKOGEE AND SURROUNDING COMMUNITIES WITHOUT A VIABLE WASTE DISPOSAL OPTION WITHIN A REASONABLE DISTANCE FROM THE COMMUNITIES AND INCREASE DISPOSAL COSTS TO THE RESIDENTS DEPENDENT ON THE LANDFILL FOR WASTE MANAGEMENT SERVICES.
- THIS ALTERNATIVE WOULD PLACE ADDITIONAL PRESSURES ON THE LONG-TERM WASTE MANAGEMENT STRATEGIES WITHIN THE STATE OF OKLAHOMA BY REMOVING OR CLOSING A CURRENTLY VIABLE ALTERNATIVE FOR WASTE DISPOSAL AND MANAGEMENT.
- NOT EXPANDING THE LANDFILL ONTO ADJACENT WASTE DISPOSAL PROPERTY IS A POOR USE OF RESOURCES FROM A LAND USE PERSPECTIVE, AS THE CURRENT LANDFILL PROPERTY AND ADJACENT PROPERTIES HAVE PROVEN TO BE COMPATIBLE WITH OTHER NEARBY RURAL LAND USES.
- THIS ALTERNATIVE WOULD REQUIRE WMO AND THE CITY OF MUSKOGEE TO SEEK ALTERNATIVE DISPOSAL OPTIONS, INCLUDING IMPACTING LANDS THAT WERE NOT PREVIOUSLY UTILIZED FOR WASTE MANAGEMENT ACTIVITIES, JURISDICTIONAL WATERS IMPACTS, VISUAL IMPACTS, LOCAL OR REGIONAL OPPOSITION, OR OTHER SITING CRITERIA IMPACTS.
- THE ONGOING USE OF THE PROPERTY WEST OF THE CURRENTLY PERMITTED LANDFILL PROPERTY WILL RENDER THE PROPERTY UNUSABLE FOR FUTURE COMMERCIAL OR RESIDENTIAL DEVELOPMENT.

NOTES:

1. EXISTING CONTOURS AND ELEVATIONS PROVIDED BY HYDREX ENVIRONMENTAL FROM AERIAL PHOTOGRAPHY FLOWN 01-25-2023.

#### ALTERNATIVE 2-EXPAND LANDFIL

THIS OPTION INCLUDES A LATERAL EXPANS OF THE CLOSED CITY OF MUSKOGEE LAND WATERS BUT WAS ELIMINATED FROM CONSI

- THE CITY LANDFILL IS A TRENCH FILL, DEPTH, PLACEMENT METHOD AND COME 1980'S, IT IS REASONABLY ASSUMED TI BENEFIT OF LINER OR LEACHATE COLLE RECONFIGURED LANDFILL PROVIDES DIS WHICH IS APPROXIMATELY 13 YEARS OF EXPANSION.
- THE DEVELOPMENT OVER THE CLOSED PURCHASE OF THE ADJACENT PROPERT ENVIRONMENTAL RISKS AND LIABILITIES OWNED BY THE CITY.
- EXPANSION OVER A CLOSED TRENCH FI ASSOCIATED WITH LINER SYSTEM INTEG SETTLEMENT OF VERTICAL EXPANSIONS FAILURE OF LINER SYSTEMS AND LEACH FOUNDATION IMPROVEMENTS.
- THE NEED TO EXCAVATE AND RELOCATI DETERMINED TO BE HIGHLY COMPRESSI ISSUES OF DIFFERENTIAL SETTLEMENT ( EXCAVATION AND RELOCATION OF WAST MANAGEMENT FEATURES REQUIRED FOR
- THE EXPANSION OF THE MSW LANDFILL SIGNIFICANT ENGINEERING CHALLENGES, RISKS TO WMO, WITH POTENTIAL ENVIRO OPTION.

DRAFT FOR PERMITTING PURPOSES ONLY SSUED FOR CONSTRUCTION			E M.
DATE: 08/2023 FILE: 0086-364-11 CAD: FIG 3-1-PROJECT OVERVIEW.DWG	DRAWN BY: RAA DESIGN BY: JBP REVIEWED BY: JVQ	NO.	DA
Weaver Consultants Group CA 3804 PE - 06/30/2025			

FUTURE SITE ENTRANCE FIELD IDENTIFIED OVERHEAD POWER	- STORMWATER POND				
		EXISTING PERMITTED LANDFILL			
		EXISTING SITE ENTRANCE			
FIELD IDENTIFI OVERHEAD PC	en e	SOUTH 54TH ST. W			
L ONTO CLOSED CITY OF MUSKOGEE LANDFILL ION FROM THE CURRENT LANDFILL TO THE WEST, OVER THE TOP FILL. THIS OPTION AVOIDS MOST IMPACTS TO JURISDICTIONAL DERATION FOR THE FOLLOWING REASONS: WITH LIMITED INFORMATION AVAILABLE REGARDING THE LOCATION, POSTION OF WASTE WITHIN THE LANDFILL. OPERATIONAL IN THE HAT THE DISPOSAL TRENCHES WERE CONSTRUCTED WITHOUT THE ECTION SYSTEMS, OR OTHER ENVIRONMENTAL CONTROLS. POSAL CAPACITY OF APPROXIMATELY 4.5 MILLION CUBIC YARDS, F ADDITIONAL LIFE WITH LIMITED OPPORTUNITY FOR FUTURE					
CITY OF MUSKOGEE LANDFILL WOULD REQUIRE WMO TO NEGOTIATE Y WHICH REASONABLY WOULD REQUIRE THAT WMO ASSUME THE ASSOCIATED WITH THE EXISTING HISTORIC LANDFILL CURRENTLY ILL LANDFILL POSES SIGNIFICANT ENGINEERING CHALLENGES RITY, LEACHATE MANAGEMENT, AND CONSTRUCTION. DIFFERENTIAL					
ATE MANAGEMENT SYSTEMS WITHOUT SIGNIFICANT LINER AATE MANAGEMENT SYSTEMS WITHOUT SIGNIFICANT LINER E WASTE FROM THE TRENCH FILL IS OFTEN REQUIRED FOR WASTE BLE OR OF SUCH VARYING THICKNESS AND COMPOSITION THAT (AND POTENTIAL ENVIRONMENTAL IMPACTS) ARE UNACCEPTABLE. E WOULD LIKELY BE REQUIRED TO PROVIDE NEEDED STORMWATER DEVELOPMENT. OVER THE CLOSED CITY OF MUSKOGEE LANDFILL PRESENTS AND FORMIDABLE ENVIRONMENTAL PROTECTION AND LIABILITY					
PREPARED FOR					
REVISIONS DATE DESCRIPTION	ALTERNATIVES	ASSESSMENT			
	MUSKOGEE CO MUSKOGEE COUI	MMUNITY RDF NTY, OKLAHOMA			
	WWW.WCGRP.COM	FIGURE 3-1			



#### NOTES:

1. EXISTING CONTOURS AND ELEVATIONS PROVIDED BY HYDREX ENVIRONMENTAL FROM AERIAL PHOTOGRAPHY FLOWN 01-25-2023.

#### ALTERNATIVE 3-RECONFIGURATION OF PROPOSED FACILITY

THIS OPTION INCLUDES A RECONFIGURATION OF THE PROPOSED LANDFILL TO THE LAYOUT SHOWN IN ABOVE FIGURE. RECONFIGURED LANDFILL PROVIDES ADDITIONAL DISPOSAL CAPACITY OF APPROXIMATELY 2.6 MILLION CUBIC YARDS, WHICH REPRESENTS APPROXIMATELY 8 YEARS OF ADDITIONAL DISPOSAL CAPACITY. THIS OPTION WAS ELIMINATED FROM CONSIDERATION FOR THE FOLLOWING REASONS:

- THIS CONFIGURATION PROVIDES ONLY 8 YEARS OF ADDITIONAL DISPOSAL CAPACITY WHICH IS NOT A LONG TERM SOLUTION FOR WASTE DISPOSAL IN THE COMMUNITY. ADDITIONALLY, THIS LAYOUT IS VERY INEFFICIENT REGARDING THE AIRSPACE GAINED COMPARED TO THE LANDFILL FOOTPRINT.
- LANDFILL RECONFIGURATION REDUCES THE AVAILABLE LANDS FOR SOIL BORROWING AND STORMWATER MANAGEMENT FACILITIES IN THE SOUTHERN PORTION OF THE PROPERTY, AT A POTENTIAL COST TO WMO ASSOCIATED WITH OFF-SITE BORROWING AND TRANSPORT OF SOILS FOR CONSTRUCTION AND WASTE MANAGEMENT.

DRAFT FOR PERMITTING PURPOSES ONLY ISSUED FOR CONSTRUCTION	(	WASTE	E MANA	PREPARI	ed for OF	OKLAHOMA,	INC.
DATE: 08/2023 FILE: 0086-364-11 CAD: FIG 3-2-PROJECT OVERVIEW.DWG	DRAWN BY: JDW DESIGN BY: JBP REVIEWED BY: JVQ	NO.	DATE	REVIS	IONS	DESCRIPTION	
Weaver Consultants Group ca 3804 PE - 06/30/2025							

# MUSKOGEE COMMUNITY RDF MUSKOGEE COUNTY, OKLAHOMA

FIGURE 3-2

SECTION 404 PERMIT ALTERNATIVES ASSESSMENT

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_SOUTH 54TH

ENTRANCE





- 1. FLOODPLAIN REPRODUCED FROM FEMA FIRM PANEL NO. 40101C0075F AND 40101C0090F FOR MUSKOGEE COUNTY AND INCORPORATED AREAS, EFFECTIVE FEBRUARY 4, 2011.
- 2. WETLAND INFORMATION REPRODUCED FROM THE U.S. FISH AND WILDLIFE SERVICE'S NATIONAL WETLAND INVENTORY.

I-17

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#### ALTERNATIVE 4-LANDFILL DEVELOPMENT ON A DIFFERENT PROPERTY

THIS OPTION INVOLVES NOT EXPANDING THE EXISTING LANDFILL, BUT INSTEAD PERMITTING A NEW LANDFILL ON A PROPERTY REMOTE TO THE EXISTING LANDFILL PROPERTY. THIS OPTION WAS ELIMINATED FROM CONSIDERATION FOR THE FOLLOWING REASONS:

THE DEVELOPMENT OF A SEPARATE TRACT WOULD IMPACT COMMUNITIES AND NEIGHBORHOODS CURRENTLY NOT IMPACTED OR FAMILIAR WITH WASTE MANAGEMENT ACTIVITIES. WHILE WASTE MANAGEMENT AND DISPOSAL REPRESENTS AN INVALUABLE AND NEEDED SERVICE TO ALL RESIDENTS AND COMMUNITIES IN MUSKOGEE COUNTY AND NORTHEAST OKLAHOMA, THE DEVELOPMENT OF NEW LANDFILLS IS CONSIDERED A NEGATIVE IMPACT ON NEARBY CITIZENS AND PROPERTIES, AND IS USUALLY CONTESTED AT A LOCAL AND STATE LEVEL TO PREVENT DEVELOPMENT OF THESE NEW FACILITIES. LEGAL COSTS ALONE CAN EXCEED \$1,000,000 FOR SITING OF NEW LANDFILL FACILITIES IF SUBJECT TO PUBLIC OPPOSITION AND THE TIMEFRAME CAN STRETCH INTO 3 TO 5 YEARS OR MORE. WITH THE CURRENT FACILITY HAVING APPROXIMATELY 4 YEARS OF REMAINING LIFE, IT IS UNLIKELY THAT A NEW LANDFILL COULD BE SITED, PERMITTED, AND CONSTRUCTED BEFORE THE EXISTING LANDFILL REACHED ITS CAPACITY.

• THE DEVELOPMENT OF A SEPARATE PROPERTY WOULD REQUIRE THE PURCHASE OF BETWEEN 200 AND 400 ACRES OF PROPERTY (INCLUDES ACCESS, ENTRANCE AND STAGING FACILITIES, EQUIPMENT MAINTENANCE AND STORAGE FACILITIES, AND STORMWATER CONTROL FACILITIES, AND LANDS NOT USABLE DUE TO JURISDICTIONAL WATERS IMPACTS OR OTHER ENVIRONMENTAL IMPACTS). THIS PROPERTY IS 566 ACRES AND FOR SALE FOR \$3.3 MILLION DOLLARS. THIS PROPERTY IS LARGE ENOUGH TO SITE A LANDFILL BUT THE DEVELOPMENT IS LIMITED BY FLOODPLAIN, WETLANDS, AND OTHER LOCATION RESTRICTIONS. THIS POTENTIAL LANDFILL COULD YIELD APPROXIMATELY 11 MILLION CUBIC YARDS OF AIRSPACE AND IS LOCATED APPROXIMATELY 5 MILES FROM THE MUSKOGEE LANDFILL.

• THE DEVELOPMENT OF SEPARATE LANDFILL ON THIS TRACT OF LAND COULD POTENTIALLY HAVE A GREATER IMPACT ON JURISDICTIONAL WATERS THAN THOSE IMPACTED BY THE PROPOSED LANDFILL EXPANSION. AS SHOWN, THE PROPERTY HAS WETLANDS (FORESTED, EMERGENT, AND RIVERINE) ON SITE, AND THESE WETLANDS ARE AT THE DOWNSTREAM END OF THEIR CONTRIBUTING DRAINAGE BASINS NEXT TO THE ARKANSAS RIVER. AS OPPOSED TO THE UPLAND WETLANDS NEAR THE MUSKOGEE LANDFILL, THESE WETLANDS ARE WELL ESTABLISHED AND WILL NEED LARGE BUFFER ZONES TO AVOID IMPACTS.

• THE DEVELOPMENT OF THIS SEPARATE TRACT OF LAND WOULD REQUIRE THE DEVELOPMENT OF OPERATIONAL INFRASTRUCTURE (E.G., OFFICES, SCALEHOUSE, MAINTENANCE SHOP, ENTRANCE ROADS AND ROADWAY IMPROVEMENTS, UTILITY CONNECTIONS, AND OTHER IMPACTS). THIS ADDITIONAL OPERATIONAL INFRASTRUCTURE WOULD CREATE AN ADDITIONAL AND UNDUE FINANCIAL BURDEN ESTIMATED BETWEEN \$1,500,000 TO \$2,500,000, WHICH IS IN ADDITION TO PROPERTY ACQUISITION AND PERMITTING COSTS.

• THE DEVELOPMENT OF A SEPARATE LANDFILL WOULD NOT BE A REASONABLE UTILIZATION OF RESOURCES OR LANDS CONSIDERING THE AVAILABILITY OF RESOURCES (LAND) IMMEDIATELY ADJACENT TO THE EXISTING LANDFILL WITH OVER 20 YEARS OF SUCCESSFUL WASTE MANAGEMENT AND DISPOSAL. DEVELOPMENT OF THIS SEPARATE LANDFILL WOULD REQUIRE WMO TO OBTAIN A NEW MSW PERMIT FROM THE ODEQ FOR A GREENFIELD SITE. THIS WOULD CREATE AN UNDUE FINANCIAL BURDEN CONSIDERING WMO HOLDS A PERMIT FOR THE CURRENT LANDFILL, AND THE PROPOSED EXPANDED LANDFILL IS BEING PERMITTED AS A MODIFICATION TO THE EXISTING PERMIT. ADDITIONALLY, WMO IS WILLING TO PROVIDE FOR OFF-SITE MITIGATION.

PREPARED FOR WANAGEMENT OF OKLAHOMA, INC.		SECTION 404 PERMIT ALTERNATIVES ASSESSMENT	
	REVISIONS		
DATE	DESCRIPTION	MUSKOGEE COMMUNITY RDF MUSKOGEE COUNTY, OKLAHOMA	
		WWW.WCGRP.COM	FIGURE 3-3



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DATE: 08/2023
FILE: 0086-364-11
CAD: 3-4 PROJECT OVERVIEW.DWG

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CA 3804 PE - 06/30/2025



NOTES:

- 1. EXISTING CONTOURS AND ELEVATIONS PROVIDED BY HYDREX ENVIRONMENTAL FROM AERIAL PHOTOGRAPHY FLOWN 01-25-2023.
- 2. THE EXPANSION WILL ADD A SEPARATE UNIT TO THE WEST OF THE EXISTING LANDFILL. NO CHANGES ARE PROPOSED TO THE EASTERN UNIT.
- 3. THE CITY OF MUSKOGEE LANDFILL IS NOT PART OF THE PROPOSED EXPANSION AND IS SHOWN FOR REFERENCE PURPOSES.
- PERMIT BOUNDARY AND EXISTING OVERHEAD EASEMENTS WERE REPRODUCED FROM LEGAL DESCRIPTION PREPARED BY WEAVER CONSULTANTS GROUP, SIGNED BY MICHAEL D BYTNER, LLS# 1986.
- 5. AN EASEMENT RECORD WAS NOT FOUND OR PROVIDED TO THE SURVEYOR. THE OVERHEAD POWER RUNNING ALONG THE NORTHERN AND SOUTHERN PROPERTY LINE AND NORTHWEST/SOUTHEAST ACROSS THE PROPERTY WAS FIELD IDENTIFIED AND SURVEYED.

#### PROPOSED LANDFILL DEVELOPMENT

THIS ALTERNATIVE PROVIDES FOR THE LONG-TERM WASTE MANAGEMENT NEEDS OF MUSKOGEE COUNTY AND OTHER COMMUNITIES CURRENTLY SERVED BY THE LANDFILL. THIS ALTERNATIVE WAS SELECTED FOR THE FOLLOWING REASONS:

• THE LANDFILL EXPANSION BENEFITS THE COMMUNITY AND LOCAL INDUSTRY BY PROVIDING A MUCH-NEEDED BASIC SERVICE IN THE FORM OF AN ENVIRONMENTALLY SAFE SOLID WASTE DISPOSAL FACILITY.

• THE LANDFILL PROPERTY AND ADJACENT PROPERTIES HAVE A HISTORY OF LAND USE COMPATIBILITY, HAVING SUCCESSFULLY OPERATED AT THIS LOCATION SINCE 1987, BEGINNING WITH THE CITY OF MUSKOGEE LANDFILL.

• SURROUNDING LAND USES ARE COMPATIBLE WITH THE PROPOSED LANDFILL DEVELOPMENT AND ARE PREDOMINANTLY AGRICULTURAL, RANGELAND, AND LOW TO VERY LOW DENSITY RURAL RESIDENTIAL AREAS.

• THE CURRENT TRAFFIC PATTERNS AND ROADWAYS ARE SUFFICIENT TO HANDLE THE CONTINUED WASTE MANAGEMENT TRAFFIC SERVED BY THE LANDFILL.

WASTE MANAGEMENT OF OKLAHOMA, INC. HAS EXPRESSED A WILLINGNESS TO PROVIDE MITIGATION OF JURISDICTIONAL WATERS IMPACTED BY THE DEVELOPMENT BY PURCHASE OF CREDITS FROM USACE-APPROVED IN-LIEU FEE PROGRAM.

• INCORPORATING PERIMETER DRAINAGE FEATURES THAT WILL COLLECT, CONVEY AND CONTROL STORMWATER AND SEDIMENT ON THE PROJECT SITE WHILE PROVIDING WATER QUALITY IMPROVEMENTS PRIOR TO OFF-SITE DISCHARGE TO DOWNSTREAM RECEIVING WATERS.

PREPARED FOR MANAGEMENT OF OKLAHOMA, INC. REVISIONS		SECTION 404 PERMIT ALTERNATIVES ASSESSMENT	
		WWW.WCGRP.COM	FIGURE 3-4

#### **4 SITE PROTECTION INSTRUMENTS**

Long-term protection of the project site is provided by the following mechanisms which prohibit incompatible uses that might otherwise jeopardize the objectives of the project.

- ODEQ Solid Waste Facility Operating Permit. After the USACE Individual Permit is issued, the permit, including this plan, will be incorporated into the ODEQ Solid Waste Operating Permit (ODEQ Permit No. 3551020). The permit provisions of ODEQ Permit No. 3551020 are enforced by the ODEQ Solid Waste Compliance Section. The active life of the expanded Muskogee Community RDF has been estimated as 24 years. Also, as part of the MSW permit, the facility will be maintained for a period of 30 years after the landfill is closed (postclosure care period). During the active life of the landfill and the postclosure period, the site will be routinely inspected by the ODEQ to ensure compliance with the ODEQ Solid Waste Permit provisions.
- County Land Records Notice. WMO will upon closure develop and file a notice in the county land records of Muskogee County that will serve notice in perpetuity that the site was used for the processing and disposal of solid waste and has been closed for the entire 316.8-acre project area.
### 5.1 Wetland and Waters of the U.S. Information

Goshawk Environmental Consulting, Inc. (Goshawk) performed a Waters of the U.S. Delineation of the expansion area for Muskogee Community RDF. The delineation report, dated May 2023, identified possible waters of the U.S. and wetlands on the property. The report is incorporated into this application as Appendix A.

A summary of the proposed compensatory mitigation is provided in Section 6 of this plan.

## **5.2 Threatened or Endangered Species**

A threatened and endangered species habitat assessment was completed by Goshawk and is included as Appendix D. Goshawk's habitat assessment literature review concluded that the site has potential to provide habitat for the American burying beetle. No additional species were identified within the assessment.

The American burying beetle utilizes a wide range of vegetative habitats including woodlands, shrublands, grasslands, riparian areas, and forests. They will even utilize lightly grazed pastures. The presence of small mammals, birds, and other carrion sources may be more important than vegetation types. The American burying beetle are not typically found in highly modified habitats, especially when soils have been impacted. The Muskogee Community RDF expansion property has both grassland and woodland vegetative habitats that are relatively unimpacted by previous disturbances. These areas are limited to the south, east, and northern portions of the property. Some of the grasslands are regularly mowed for hay production and there is evidence of some terracing. Since the American burying beetle has been documented within relatively close proximity to the property, there exists a potential that American burying beetle may use portions of the site not previously impacted.

Based on the site having a potential to provide habitat for the American burying beetle, a formal consultation with the US Fish and Wildlife Service was conducted. The consultation indicated that any incidental take of the American burying beetle that may occur as a result of the expansion is not prohibited under the Act, Section 4

(d) rule adopted for this species at so CFR § 17.47(d). A copy of the consistency letter from US Fish and Wildlife Service is included in Appendix D.

Habitat does not exist within the site for any of the other listed species. While it is possible that the migratory bird species may utilize the landfill site during migration, use would be transitory in nature and of short duration. Lack of suitable habitat makes the occurrence of the migratory species highly unlikely.

### **6 DETERMINATION OF CREDITS**

A summary of the unavoidable impacts to jurisdictional waters from the proposed landfill expansion is shown on Figure B-5 (Appendix B) and summarized below. However, this plan should also be judged by the overall positive impact this project will have on the community and local industry. For example, the benefits to the community and local industry include providing a much-needed basic service in the form of an environmentally safe, solid waste disposal facility. The benefits of the project are delineated in Section 3.1 and on Figure 3-4 of this plan.

Unavoidable Impact Summary

- 0.17 acres of emergent wetlands
- 1.90 acres of open water pond
- 0.32 acres of forested wetlands
- 2,067 linear feet of ephemeral streamline

To offset the unavoidable impacts and to ensure that there will be no overall net loss of waters of the U.S., WMO is proposing to obtain the following credits from the USACE approved in-lieu fee Program (Terra Foundation, Inc.).

- 2.55 acres of wetlands credits
- 10,207 credits of ephemeral streamline

The majority of our site (the southwestern portion) lies within the Terra Foundations lower Arkansas A service area. The northeastern portion of the site lies with the Terra Foundations lower Arkansas B service area. The Terra Foundation currently has available wetlands credits in both service areas.

The proposed credits would represent the following ratio of mitigation to disturbance for wetlands.

- 1:1 for emergent wetlands
- 1:1 for open water ponds
- 1:5:1 for forested wetlands

To determine the proposed stream credits the Oklahoma Stream Mitigation Method (July 2021) was utilized. The OSMM Adverse Impact Features Worksheet A-1 is included in Appendix A. The Terra Foundation currently has available stream credits in both service areas.

## 7.1 Mitigation Areas

The unavoidable impacts on the project will not significantly impact the watershed of the project being that; (1) the area of impact is located at the headwaters of the watersheds; (2) the area of impact is outside the 100-year floodplain; and (3) proposed development includes perimeter drainage features that will collect, convey, and control stormwater and sediment on the project site. The proposed mitigation will be completed through a USACE approved in-lieu fee program.

#### 7.1.1 Mitigation Centers

The proposed mitigation for this project will be completed off-site utilizing a USACE approved in-lieu fee program. WMO will contract with the in-lieu fee program and notify the USACE once the off-site mitigation has been procured/confirmed. If the mitigation is not available, WMO will contact the USACE to discuss other alternatives.

As described in Section 6 of this plan, The proposed mitigation for this project includes the mitigation of 2.39 acres of wetlands and 2,067 linear feet of jurisdictional waters at a mitigation ratio presented in Section 6.

## 7.2 Sequence of Development and Storm Water Control

The expected active life of the landfill without the expansion has been estimated to be approximately 3.5 to 4 years. Therefore, the initial development of the expansion area will not likely begin for approximately 2 years. Large portions of the western expansion area could potentially be developed without disturbing jurisdictional waters, however, WMO intends to develop perimeter drainage features initially to support site operations and control stormwater. This perimeter drainage development will disturb portions of the on-site jurisdictional waters. Therefore, development of expansion area features that require disturbance of jurisdictional waters will not be initiated until mitigation activities are procured by WMO.

#### 7.2.1 Perimeter Drainage System

The stormwater controls for the landfill have been designed consistent with the ODEQ regulations for MSW landfills. Per ODEQ requirements, the runon stormwater

controls have been designed to prevent flow onto the active portion of the landfill during the peak discharge from a 25-year storm and the runoff stormwater controls have been designed to collect and control the water volume resulting from a 24-hour, 25-year storm. These include drainage controls for the final cover, perimeter channels, culverts, and a sedimentation and storage pond.

The drainage system is shown on Figure 3-4 (Section 3). Drainage from the landfill itself is directed through a system of swales, chutes, and perimeter channels to a detention/sedimentation pond located along the western boundary of the property. From the pond, waters will infiltrate, evaporate, or will be pumped to adjacent existing surface water drainage features.

#### 7.2.2 Storm Water Controls During Below Grade Fill

Control of stormwater runon and runoff within excavation areas will be achieved using temporary diversion berms, channels, and containment areas as needed. The temporary stormwater control structures are used to divert uncontaminated stormwater runoff into temporary storage areas. The stormwater will be used for liner construction, control of dust, and establishing vegetation. If discharge of uncontaminated stormwater is required, it will be discharged consistent with National Pollutant Discharge Elimination System (NPDES) requirements.

Contaminated stormwater consists of stormwater that has come into contact with waste. Control of the contaminated stormwater will be provided through temporary diversion berms, channels, and containment areas. Contaminated stormwater structures will be constructed to capture the stormwater for a 24-hour, 25-year storm runoff volume as required by the landfill permit. Contaminated stormwater will be diverted and contained on approved areas only. Contaminated water and leachate will be disposed of as required by the permit application to be submitted to the ODEQ.

#### 7.2.3 Storm Water Controls During Aerial Fill

Additional storm water controls will be necessary as the site is brought above grade. Temporary diversion berms, channels, and containment areas will continue to be used for control of uncontaminated and contaminated stormwater runon and runoff. Separation of the contaminated stormwater and uncontaminated stormwater runoff will be provided. Diversion berms, channels, and containment areas will be implemented for the active fill portions of the landfill.

The final cover will incorporate drainage swales and letdown structures or chutes for conveyance of stormwater off of the final cover. These swales and chutes have been designed to protect the final cover from erosion. As areas of the final cover are completed, vegetation will be established to provide additional erosion protection. The final cover will be designed consistent with ODEQ regulations. Surface water runon and runoff will be managed consistent with the ODEQ regulations. Areas that receive waste will be covered with cover soils. As such, runoff from these areas will be considered uncontaminated consistent with ODEQ requirements. Also, by implementing (1) the site design and proper operating practices and (2) ongoing placement of final cover, contaminated water will be kept to a minimum. Routine daily cover, in combination with the other operating practices (e.g., use of small landfill working face), will minimize the generation of contaminated water. Contaminated stormwater will be diverted and contained on approved areas only. Contaminated water and leachate will be pumped to an existing leachate pond, temporary leachate storage pond, or leachate storage tanks, or transported to an offsite treatment facility for disposal in accordance with the permit application to be submitted to ODEQ.

#### 7.2.4 Erosion and Sedimentation Control

Erosion and sedimentation control is provided on site during construction activities and is incorporated into the design of the perimeter drainage system and final cover system. During construction of the excavations, perimeter berms, perimeter channels, and a sedimentation and storage pond, erosion and sedimentation control will be provided through the use of temporary diversion berms, drainage channels, silt fences, vegetation, and hay bales. These measures will provide for control of erosion and sediment prior to stormwater flows leaving the site. An erosion and sedimentation control plan will be developed as part of the permit application to be submitted to the ODEQ.

Permanent erosion control features have been included in the site design. These features include design of perimeter channels for non-erodible velocities. In areas where erosion has been anticipated, erosion protection of the channels in the form of turf reinforcement matting and gabions are provided. Permanent features included in the final cover design are drainage swales and chutes are shown on Drawing B-6 (Appendix B). Erosion protection will also be provided on areas susceptible to erosion. Establishment of vegetation on the final cover, perimeter channel, and buffer zones will be ongoing as the site is developed.

#### 7.3 Protection During Construction

Contractors will be required to implement the site Storm Water Pollution Prevention Plan (SWPPP) or comply with the current site SWPPP to minimize and control the discharge of pollutants in storm water runoff during construction. The site SWPPP was developed to meet the requirements of the OPDES General Permit.

### 7.4 Planting Plan

Planting lists, methods, and success criteria for the offsite mitigation activities will be based on the USACE approved in-lieu fee program's approved plan.

## 8.1 Consistency with Other Plans

Certain other environmental management plans may contain provisions for managing stormwater. The site operator has the responsibility to incorporate these provisions into the plan, as applicable. Examples of compatible environmental plans include the following.

- Landfill Operating Permit developed for the provisions of Title 252 of the Oklahoma Administrative Code, Chapter 515-Management of Solid Waste.
- Spill Prevention, Control, and Countermeasure Plan developed for the provisions of 40 Code of Federal Regulations Part 112.
- Stormwater Pollution Prevention Plan developed for the provisions of the NPDES Stormwater Permit.

#### 8.2 Routine Maintenance

Maintenance activities described in this section will be performed on a regular basis by landfill personnel on the project site. Routine maintenance of the offsite mitigation activities will be performed in accordance with the USACE approved inlieu fee program's approved plan.

#### 8.2.1 Vegetation Management

The site will be operated in a manner to minimize disturbances to vegetation established on the project site. Vegetation management of the offsite mitigation activities will be performed in accordance with the USACE approved in-lieu fee program's approved plan.

#### 8.2.2 Good Housekeeping Measures

Good housekeeping measures will reduce or eliminate the impact of landfill operations on precipitation and runoff at the project site. The measures used to ensure areas that could contribute pollutants are kept in a neat and orderly fashion consist of the BMPs identified in this facility's storm water pollution prevention plan. The BMPs to be used at the landfill include the following:

- Routine litter patrolling by landfill staff
- Dust control on access roads internal to the landfill.

Good housekeeping measures for the offsite mitigation activities will be performed in accordance with the USACE approved in-lieu fee program's approved plan.

#### 8.2.3 Erosion Control Measures

During project site development, BMPs will be employed to control erosion at the active landfill area, soil stockpiles, and inactive areas with and without final cover. BMPs may include the use of temporary rock riprap, silt fences, straw bales, check dams, interceptor swales and berms, temporary and permanent seeding and sodding, surface roughening, matting and mulching, sediment traps, and surface wetting for dust control. Landfill personnel will follow the guidelines in the SWPPP to select appropriate erosion control measures to minimize sediment discharge from the site.

Erosion control measures for the offsite mitigation activities will be performed in accordance with the USACE approved in-lieu fee program's approved plan.

#### 8.2.4 Maintenance Program for Drainage Control Structures

WMO personnel will restore and repair the drainage control structures in the event of washout or failure from extreme storm events. In addition, BMPs will also be replaced or repaired in the event of failure. Excessive sediment will be removed, as needed, so that the perimeter channels, and sediment ponds function as designed.

The following items will be evaluated during the routine inspections:

- Erosion of perimeter channels, sedimentation ponds, berms, and other drainage features
- Settlement of perimeter channels and other drainage features
- Silt and sediment build-up in perimeter channels, chutes, swales, and sedimentation ponds
- Obstructions or foreign objects
- Presence of erosion or sediment discharge at offsite storm water discharge locations
- Presence of sediment discharges along the site boundary in areas, which have been disturbed by site activities.

Maintenance activities will be performed to correct damaged or deficient items noted during the routine site inspections. These activities will be performed as soon as possible after the inspection. The time frame for correction of damaged or deficient items will vary based on weather, ground conditions, and other site-specific conditions.

Maintenance activities will consist of the following, as needed.

- Placement of additional temporary or permanent vegetation
- Placement, grading, and stabilization of additional soils in eroded areas or in areas that have settled
- Replacement of gabions, turf reinforcement mat, or other structural lining
- Removal of obstructions and/or foreign objects
- Removal of silt and sediment build-up
- Repairs to erosion and sedimentation controls
- Installation of additional erosion and sedimentation controls

Maintenance programs for the offsite mitigation activities will be performed in accordance with the USACE approved in-lieu fee program's approved plan.



#### **9 PERFORMANCE STANDARDS**

The landfill will be constructed and operated in accordance with the site MSW permit, ODEQ regulations, and industry standards. During the active life of the landfill (approximately 24 years at time of closure of the expansion area) and the post-closure period (30 years), the site will be routinely inspected by the ODEQ to ensure compliance with the ODEQ MSW permit.

Offsite mitigation will be completed utilizing a USACE approved in-lieu fee program. Mitigation activities including performance standards, long term management, and monitoring/reports will be completed in accordance with the USACE approved inlieu fee program's approved plan.

#### **10 MONITORING REQUIREMENTS**

The landfill will be constructed and operated in accordance with the site MSW permit, ODEQ regulations, and industry standards. During the active life of the landfill (approximately 24 years at time of closure of the expansion area) and the post-closure period (30 years), the site will be routinely inspected by the ODEQ to ensure compliance with the ODEQ MSW permit.

Offsite mitigation will be completed utilizing a USACE approved in-lieu fee program. Mitigation activities including performance standards, long term management, and monitoring/reports will be completed in accordance with the USACE approved inlieu fee program's approved plan.

#### **11 LONG-TERM MANAGEMENT PLAN**

The expected active life of the landfill with the expansion has been estimated as 24 years. However, the actual site life may vary depending on the incoming waste volume and other site-specific factors (e.g., waste density; amount of cover soil used). Also, as part of the MSW permit, the facility will be maintained for a period of 30 years after the landfill is closed (post-closure care period). During the active life of the landfill constructed drainage and erosion control structures will be inspected on a regular basis and after every significant rainfall event. During the postclosure care period, the site will be inspected according to a schedule set by ODEQ. Any repairs or maintenance to drainage and erosion control structures will be performed as soon as practical following the inspections.

Landfill personnel who conduct the inspections will be familiar with the MSW permit, ODEQ regulations, and industry standards related to perimeter drainage systems, stormwater controls, erosion sedimentation controls and landfill operations.

The landfill will be constructed and operated in accordance with the site MSW permit, ODEQ regulations, and industry standards. During the active life of the landfill and the postclosure period, the site will be routinely inspected by the ODEQ to ensure compliance with the ODEQ MSW permit.

Offsite mitigation will be completed utilizing a USACE approved in-lieu fee program. Mitigation activities including performance standards, long term management, and monitoring/reports will be completed in accordance with the USACE approved inlieu fee program's approved plan.

#### **12 ADAPTIVE MANAGEMENT PLAN**

If the compensatory mitigation cannot be constructed in accordance with the USACE approved in-lieu fee program's approved plan, the in-lieu fee program will notify the USACE.

#### **13 FINANCIAL ASSURANCE**

Financial assurance will be maintained to meet ODEQ requirements by WMO. Financial assurance is required for all solid waste landfill facilities whose debts and liabilities could become the debts and liabilities of a state or the United States (i.e., in the event of forced closure, which occurs when an operational municipal solid waste landfill facility can no longer operate because of an inability to manage the incurred debts and liabilities). At such time, the responsibility for closure would be assumed by the ODEQ. WMO provides financial assurance to the ODEQ to cover the cost of hiring a third party to close the landfill and to provide continuous maintenance and monitoring during the 30-year post-closure period for the landfill. Closure of the landfill includes construction of final cover over uncapped areas, site grading and drainage improvement, and stabilizing disturbed areas. Postclosure activities include annual site inspections, periodic site monitoring, and routine maintenance.

Offsite mitigation will be completed utilizing a USACE approved in-lieu fee program. Mitigation activities including performance standards, long term management, and monitoring/reports will be completed in accordance with the USACE approved inlieu fee program's approved plan.

Financial assurance for the offsite mitigation activities will be provided by the sponsor of the USACE approved in-lieu fee program in accordance with the approved plan. Therefore, this plan also shows compliance with 33 CFR §332.3(n).

## MUSKOGEE COMMUNITY RECYCLING AND DISPOSAL FACILITY MUSKOGEE COUNTY, OKLAHOMA ODEQ PERMIT NO. 3551020

## APPENDIX J ALTERNATIVE FINAL COVER SYSTEM DESIGN

Prepared for

Waste Management of Oklahoma, Inc.

October 2023



Prepared by

Weaver Consultants Group, LLC

CA 3804 PE-06/30/2025 6420 Southwest Boulevard, Suite 206 Fort Worth, Texas 76109 817-735-9770

WCG Project No. 0086-364-11-19

#### INTRODUCTION

This appendix includes a copy of the approved Alternative Final Cover (AFC) Design Permit Modification submitted by Weaver Boos Consultants, LLC–Southwest in September 2008 and ODEQ's approval letter dated October 13, 2008 (see following two pages). The ODEQ approved AFC design has been modified based on current conditions and to include a vegetation plan. The modifications include revisions to Figures A-1 through A-3.

Since this alternative final cover design is not a function of a particular area and can be used for both top slopes and sideslopes, it is also applicable for the expansion area. The current vegetation plan required to meet OAC 252.515-19-54 is also included (see Appendix E of the Alternative Final Cover Design).

#### MUSKOGEE COMMUNITY RECYCLING AND DISPOSAL FACILITY MUSKOGEE COUNTY, OKLAHOMA ODEQ PERMIT NO. 3551020

#### TIER I PERMIT MODIFICATION

#### ALTERNATIVE FINAL COVER SYSTEM DESIGN

Prepared for

Waste Management of Oklahoma, Inc.

Approved October 13, 2008

Revised October 2023



Prepared by

#### Weaver Consultants Group, LLC

CA 3804 PE 06/30/2025 6420 Southwest Boulevard, Suite 206 Fort Worth, Texas 76109 817-735-9770

WCG Project No. 0086-364-11-19

#### WEAVER

#### BOOS

### CONSULTANTS

#### SOUTHWEST

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Chicago, IL Springfield, IL Naperville, IL Griffith, IN South Bend, IN Denver, CO St. Louis, MO Columbus, OH Fort Worth, TX

September 15, 2008 Project No. 0086-364-11-01-01

Saba Tahmassebi, Ph.D., P.E. Chief Engineer Oklahoma Department of Environmental Quality 707 North Robinson Oklahoma City, Oklahoma 73101-1677

Re: Tier I Permit Modification –Alternative Final Cover System Design Muskogee Community Recycling and Disposal Facility, Permit No. 3551020 Muskogee County, Oklahoma

Dear Dr. Tahmassebi,

On behalf of Waste Management of Oklahoma, Inc. (WM), please find attached two copies of the referenced permit modification. The purpose of this permit modification is to permit an evapotranspiration (ET) monolithic soil alternative final cover system at the Muskogee Community Recycling and Disposal Facility (RDF). This permit modification will provide WM the option to construct an ET monolithic soil alternative final cover system in addition to the existing permitted final cover options. Note that this permit modification is similar to the ET monolithic soil alternative final cover system included in the recently approved Tier III Permit Modification Lateral Expansion for East Oak RDF.

Please process this application per Oklahoma Administrative Code (OAC) 252:4-7-58(2)(A)(ii) given that the proposed ET monolithic soil alternative final cover system meets the requirements set forth in OAC 252:515-19-53(c). Two copies of this application are provided for your use and distribution. Consistent with OAC 252:515-19-40(a), a copy of this application has been placed in the site operating record for this facility.

During the course of your review, if you need additional information or have any questions please call.

Sincerely, Weaver Boos Consultants, LLC–Southwest

Jeffrey P. Young, P.E. Senior Engineer

Attachment: Tier I Permit Modification (2 copies)

cc: Pete Schultze, Waste Management of Oklahoma, Inc.

## CONTENTS

LIST	OF TA	BLES	iii
LIST	OF AP	PENDICES	iv
1	INTF	RODUCTION	1
	1.1	Purpose	1
	1.2	Equivalency Demonstration Requirements	2
	1.3	Alternative Final Cover Model Summary	3
	1.4	Equivalency Demonstration Summary	7
	1.5	Alternative Final Cover Material Requirements	7

### LIST OF TABLES

Table	<u>Page</u>
1-1 Pre-Subtitle D/Subtitle D Final Cover System Design	2
1-2 Summary of Percolation Rates through Prescriptive Subtitle D Composite Final Cover Systems	5
1-3 Equivalency Demonstration Summary	8

#### LIST OF APPENDICES

#### **APPENDIX A**

Figures

#### **APPENDIX B**

Equivalency Design Demonstration

- Prescriptive Subtitle D Composite Final Cover System UNSAT-H Analysis
- ET Monolithic Soil Alternative Final Cover System UNSAT-H Analysis

#### **APPENDIX C**

Daily Climatological Data Summary

#### APPENDIX D

Alternative Final Cover Quality Assurance/Quality Control Plan

## APPENDIX E

Vegetation Plan

## 1.1 Purpose

The purpose of this permit modification is to permit an Evapotranspiration (ET) monolithic soil alternative final cover system at the Muskogee Community Recycling and Disposal Facility (RDF). The monolithic final cover system will consist of the following layers.

- 12-inch-thick Vegetative Layer. Soil for this layer will be selected to support the rapid establishment of a vegetative cover that will consist of native and introduced grasses that will thrive in the Muskogee area climate.
- 24-inch-thick Vegetative Support Layer. The soils in this layer will be capable of storing moisture in the ET monolithic soil alternative final cover system so that the moisture can be removed by evaporation and transpiration from vegetation growing on the cover.
- Existing 12-inch-thick Intermediate Cover Layer. This layer functions as a foundation layer for the vegetative support layer. However, this material will also function as a vegetative support layer.

The above ET monolithic soil alternative final cover system is designed to minimize infiltration of stormwater into the underlying wastes. The ET alternative final cover is basically a monolithic soil cover that employs a thick layer of soil with adequate soil-water storage capacity to retain any infiltrated water until it can be removed through ET. The ET alternative final cover concept relies on the soil to act like a sponge. A key to the design is that the "soil sponge" or "soil rooting medium" be designed thick enough to hold infiltration of precipitation until the water can be consumed by ET.

Consistent with OAC 252:515-19-53(c), the proposed alternative cover is designed to be equivalent with the Subtitle D prescriptive composite final cover system and also by extension the pre-Subtitle D area final cover system. As shown on Figure A-1 (Appendix A), no final cover has previously been constructed at the Muskogee Community RDF. Consistent with OAC 252:515-25-33, the site will initiate final cover activities 90 days after the final receipt of waste. As shown on Figure A-2 (Appendix A), approximately 3 acres of final cover has been previously constructed at Muskogee Community RDF.

The following sections of this permit modification discuss the requirements of the equivalency demonstration, a description of the numeric model used to make the equivalency demonstration, a summary of the

Rev. 1. 10/2023

model input parameters, a summary of the equivalency demonstration, and specifications for the ET monolithic soil alternative final cover system. In addition, the following information is presented in appendices that are attached to this permit modification.

- Appendix A Figures. This appendix includes site plans that depict the ET monolithic soil alternative final cover system area and various details of the proposed alternative final cover system.
- Appendix B Equivalency Design Demonstration. This appendix includes the UNSAT-H model input and output files for both the prescriptive Subtitle D composite final cover system and the ET monolithic soil alternative final cover system.
- Appendix C Daily Climatological Data Summary. This appendix includes climatological data that was used as input into the UNSAT-H model.
- Appendix D Alternative Final Cover Quality Assurance/Quality Control Plan. This plan sets forth the QA/QC plan for each component of the ET monolithic soil alternative final cover system.

## **1.2** Equivalency Demonstration Requirements

Consistent with OAC 252:515-19-53(c), the ET monolithic soil alternative final cover system is designed to be equivalent with the prescriptive Subtitle D composite final cover system design and also by extension the pre-Subtitle D final cover design. Both designs are summarized in Table 1-1.

Pre-Subtitle D Final Cover	Prescriptive Subtitle D Cover	
12-inch-thick erosion layer	12-inch-thick erosion layer	
	Drainage geocomposite or geotextile	
24-inch-thick compacted clay layer	• 40-mil-thick LLDPE geomembrane	
(k ≤ 1x10 ^{.7} cm/s)	• 24-inch-thick compacted clay layer ( $k \le 1x10^{-7}$ cm/s)	

Table 1-1Pre-Subtitle D/Subtitle D Final Cover System Design

As stated previously, the purpose of this permit modification is to demonstrate that the ET monolithic soil alternative final cover system is equivalent to the pre-Subtitle D/Subtitle D final cover systems listed in Table 1-1. The following two criteria will be used to demonstrate equivalency.

1) Through the use of UNSAT-H, it will be demonstrated that the estimated percolation through the ET monolithic soil alternative final cover system will be less than the prescriptive Subtitle D final cover system and by extension the pre-Subtitle D area final cover system.

2) Also through the use of UNSAT-H, it will be demonstrated that the moisture content in the bottom layer of the ET monolithic soil alternative final cover system will remain at a consistent level over the analysis period. This demonstrates that the alternative final cover system will function as designed. For example, the moisture content in the upper levels of the ET monolithic soil alternative final cover system will vary due to evapotranspiration and infiltration. However, if the moisture content in the bottom layer of the ET monolithic soil alternative final cover system remains consistent, then this is an indication that no significant amount of water will infiltrate past the bottom final cover layer.

As documented in numerous references regarding alternative final cover demonstrations, the estimate of infiltration through the prescriptive Subtitle D composite final cover system (Criteria 1 listed above) has been studied extensively over the past few years. Until recently, relatively unsophisticated models (e.g., HELP) and limited information from constructed composite final covers has been available. However, more sophisticated models (e.g., UNSAT-H) and information from the EPA's Alternate Cover Assessment Program (ACAP) are now available to more accurately predict percolation through a composite final cover system. A summary of various percolation rates through composite final covers, including the estimated percolation rate using UNSAT-H for the Muskogee Community RDF alternative final cover system is summarized in Table 1-2.

Given the above (Criteria 1 and 2) and the information listed in Table 1-2, a percolation rate of 14.0 mm (0.553 inches) has been used as the benchmark for Criterion 1. This value is the lowest percolation rate listed in Table 1-2 and ensures a conservative comparison.

#### **1.3 Alternative Final Cover Model Summary**

UNSAT-H (Fayer and Junes, 1990) was selected to model both the prescriptive Subtitle D composite final cover system and the ET monolithic soil alternative final cover system. UNSAT-H is a one-dimensional physically based model and is one of the most commonly used models for alternative final covers. The various UNSAT-H input parameters for climate, vegetation, and soils are discussed in the following subsections.

#### Climate

Muskogee County, Oklahoma has a continental climate characterized by rapid changes in temperature. Occasionally the county may experience influence exerted by warm, moist air currents from the Gulf of Mexico. The winters are generally mild and the summers are hot. Actual daily rainfall data obtained from the Bixby, Oklahoma, weather station are used in the simulations. Weather data includes rainfall, temperature, wind speed, solar radion, and humidity for one year (2001). Total rainfall measured during this period was 37.34 inches.

The area is occasionally subjected to large hail and violent windstorms which occur mostly during the spring and early summer, although occurrences have been noted throughout the year. Snowfall averages less than 10 inches per year and seldom remains on the ground very long. Occasional periods of brief freezing rain and sleet storms can occur during winter months.

UNSAT-H, the modeling program used for this analysis, requires daily climatological data. Climate data for the model input were compiled from year 2001 records for using the Bixby, Oklahoma, weather station by using Mesonet (an Oklahoma climate data provider). Bixby, Oklahoma, is less than 31 miles from the site. Individual data was occasionally missing because of equipment failures at the weather station. Because the UNSAT-H model requires input of daily climate data, those gaps were filled in by replacing missing records with an average value from the day before and the day after the gap. The weather data used is included in Appendix C.

# Table 1-2Summary of Percolation Rates through Prescriptive Subtitle D<br/>Composite Final Cover Systems

Case	Average Annual Precipitation	Estimated Yearly Percolation Rate	Comments
CaseAverage Annual PrecipitationEstimated Yearly Percolation RateMuskogee Community RDF37.34 inches114.0 mm or 0.553 inchesTh Ap pat FinOmaha, NE EPA ACAP Site28 inches5.5 mm or 0.22 inchesTh Co 	The input/output files for this modeling effort are included in Appendix B. A discussion of the various modeling parameters is included in the section entitled, "Alternative Final Cover Model Summary."		
Omaha, NE EPA ACAP Site	28 inches	5.5 mm or 0.22 inches	This information was reproduced from the EPA's Alternate Cover Assessment Program 2002 Annual Report (Albright and Benson).
Cedar Rapids, ID EPA ACAP Site	36.4 inches	6.1 mm or 0.24 inches	This information was reproduced from the EPA's Alternate Cover Assessment Program 2002 Annual Report (Albright and Benson).
EPA ACAP Program Current Recommended Percolation Rate for AFC	N/A	5 mm or 0.20 inches	This information was obtained from Craig H. Benson, PhD, P.E. who is a key advisor for the EPA ACAP Program.

1 This value is from 2001 and was used for this demonstration.

#### Vegetation

With UNSAT-H, plant cover over the course of the year is represented by the leaf area index (LAI). The LAI is the ratio of the total leaf surface area to the size of the area vegetation is growing (USEPA, 1994). For this analysis, it was assumed that the vegetative growing season started in spring on the average date of the last frost (early April) and the transpiration ended on the average date of the first severe frost (early November).

#### Soils

The major soil input parameters for both cover systems are discussed below.

- Saturated Water Content of Soils. Saturated water content of 40 percent has been used for the top one-foot layer of soil for the prescriptive subtitle D composite final cover system and for the entire thickness of the ET monolithic soil alternative final cover system. The saturated water content for soils below the top one-foot layer is assumed to be 32 percent for the prescriptive Subtitle D composite final cover system.
- Residual Water Content of Soils. The residual water content for the top one-foot thick layer is assumed to be 8 percent for both the prescriptive Subtitle D composite and the ET monolithic alternative soil final cover systems. The layers below the top one-foot layer are assumed to have 15 percent residual water content for both the prescriptive Subtitle D composite and the ET monolithic soil alternative final cover system.
- Saturated Hydraulic Conductivity. A saturated conductivity of  $5x10^{-5}$  cm/s has been used for the top 12 inches of both the prescriptive Subtitle D composite and the ET monolithic soil alternative final cover systems. A hydraulic conductivity of  $1x10^{-7}$  cm/s has been used at lower portion of the prescriptive Subtitle D composite final cover system. Additionally, a thin layer (below the top one-foot layer) is modeled with  $3.2x10^{-12}$  cm/s hydraulic conductivity to represent the geomembrane cover. A hydraulic conductivity of  $2x10^{-5}$  cm/s has been used for the lower portion of the ET monolithic alternative soil final cover system. This value is typical for the hydraulic conductivity of silty or sandy clay material placed with low levels of compactive effort, which is the suggested soil placement technique for monolithic covers.
- Suction Head. UNSAT-H utilizes the initial suction head of the soil layers for initial hydraulic conditions. Initial suction head for the ET monolithic soil alternative final cover system is 15,000 cm, which corresponds to wilting point, and suggested to be used for ET monolithic soil alternative final cover systems. However, considering that the soils will not be at the wilting point when placed, a value of 10,000 cm was used for the erosion layer of the prescriptive Subtitle D composite and all layers of the ET monolithic alternative soil final cover systems. Considering that the prescriptive Subtitle D composite final cover system infiltration layer will be constructed with a relatively high moisture content and

the geomembrane does not have a suction head, a value of 1,000 cm and 100 cm, respectively, was used for these layers.

#### **1.4 Equivalency Demonstration Summary**

A summary of the equivalency demonstration is provided in Table 1-3. As shown, the ET monolithic soil alternative final cover system performs better than the prescriptive Subtitle D composite final cover system for each of the following equivalency criteria.

- Percolation Rate. As shown on Table 1-3 and Figure 1, the ET monolithic soil alternative final cover system estimated percolation rate is less than (1) the site specific modeling estimate, (2) similar ACAP sites, and (3) ACAP's current alternative final cover recommended percolation rate.
- Moisture Content Variance. Figure 2 provides a summary of the moisture content fluctuation of the various layers of the ET monolithic soil alternative final cover system. As shown, the moisture content fluctuates substantially in the upper layers of the system; however, the moisture content in the bottom layer is relatively constant. This analysis provides another demonstration that very little surface water infiltrates through the ET monolithic soil alternative final cover system.

#### **1.5 Alternative Final Cover Material Requirements**

As discussed in previous sections of this permit modification, the ET monolithic soil alternative final cover system includes three layers (from top to bottom): a vegetation layer; a vegetation support layer; and a existing intermediate cover layer. A description of the material requirements for each layer is listed below.

- Vegetation Layer. Soil for this layer will consist of material that is capable of supporting the rapid establishment of vegetation. If on-site "topsoil" is not available, this material will be imported from an off-site source. Muskogee Community RDF will notify the ODEQ of the vegetation layer source two weeks prior to construction.
- Vegetation Support Layer. This 24-inch-thick layer will be capable of storing moisture to be used by the vegetation layer. This layer will consist of cohesive soils that classify as either CL, CH, ML, SM, or SC according to the Unified Soil Classification System (USCS). As recommended in information developed for the EPA's ACAP, to promote adequate root development, the soil in a vegetation support layer should be compacted to between 75 and 85 percent of Standard Proctor density at a moisture content less than optimum. This layer will be placed in one 24-inch lift.

Table 1-3Equivalency Demonstration Summary

Equivalency Criteria	Prescriptive Subtitle D Composite Final Cover System	ET Monolithic Soil Alternative Final Cover System
1. Estimated percolation rate through the final cover systems	<ul> <li>14.0 mm using UNSAT-H</li> <li>5.5 - 6.1 mm - similar ACAP sites</li> <li>5 mm ACAP current recommendation</li> </ul>	4.7 mm using UNSAT-H

• Existing 12-inch-thick Intermediate Layer. This layer functions as a foundation layer for the vegetation support layer. However, this material will also function as a vegetation support layer. Although this layer has no specific material requirements, it is composed of soils that are classified as CL, CH, ML, SM, or SL, according to the USCS. Therefore, this layer meets the specifications of the vegetation support layer.

The existing 12-inch-thick intermediate layer thickness will be verified prior to placement of the 24-inch-thick vegetation support layer. As an option to the intermediate layer thickness verification, the vegetation support layer may be installed as a 36-inch-thick single layer.

Construction Quality Assurance and Quality Control (QA/QC) procedures for each of these layers, including surveying requirements, are addressed in the Alternative Final Cover QA/QC Plan included in Appendix D.





## APPENDIX A FIGURES

Includes pages A-1 through A-3



0 300	0 600
SCALE IN	I FEET
LEGE	END
	EXISTING PERMIT BOUNDARY
	PROPOSED PERMIT BOUNDARY
	PERMITTED LIMITS OF WASTE
	PROPOSED LIMITS OF WASTE
N 273,000	STATE PLANE COORDINATE GRID
640	EXISTING CONTOUR
OHE	EXISTING OVERHEAD EASEMENT
	IN-PLACE FINAL COVER
⊕ ^{MW-5R}	EXISTING GROUNDWATER MONITORING WELL
● ^{GP-5}	EXISTING LANDFILL GAS MONITORING PROBE

NOTES:

PREPARED FOR

REVISIONS

DESCRIPTION

REVISED TO INCLUDE EXPANSION AREA

- 1. EXISTING CONTOURS AND ELEVATIONS PROVIDED BY HYDREX ENVIRONMENTAL COMPILED FROM AERIAL PHOTOGRAPHY FLOWN 01-25-2023.
- 2. THE EXPANSION WILL ADD A SEPARATE UNIT TO THE WEST OF THE EXISTING LANDFILL. NO CHANGES ARE PROPOSED TO THE EASTERN UNIT.
- 3. THE CITY OF MUSKOGEE LANDFILL IS NOT PART OF THE PROPOSED EXPANSION AND IS SHOWN FOR REFERENCE PURPOSES.
- 4. PERMIT BOUNDARY AND EXISTING OVERHEAD EASEMENTS WERE REPRODUCED FROM LEGAL DESCRIPTION PREPARED BY WEAVER CONSULTANTS GROUP, SIGNED BY MICHAEL D BYTNER, LLS# 1986.
- AN EASEMENT RECORD WAS NOT FOUND OR PROVIDED TO THE SURVEYOR. THE OVERHEAD POWER RUNNING ALONG THE NORTHERN AND SOUTHERN PROPERTY LINE AND NORTHWEST/SOUTHEAST ACROSS THE PROPERTY WAS FIELD IDENTIFIED AND SURVEYED.

ROFESSION AL
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AHOW
10/13/2023
•

ALTERNATE	FINAL	COVER
SIT	E PLA	Ν

MUSKOGE	Е СОММИ	NITY RDF
MUSKOGEE	COUNTY,	OKLAHOMA

WWW.WCGRP.COM

FIGURE A-1





NOTES:

- 1. EXISTING CONTOURS AND ELEVATIONS PROVIDED BY HYDREX ENVIRONMENTAL COMPILED FROM AERIAL PHOTOGRAPHY FLOWN 01-25-2023.
- 2. THE EXPANSION WILL ADD A SEPARATE UNIT TO THE WEST OF THE EXISTING LANDFILL. NO CHANGES ARE PROPOSED TO THE EASTERN UNIT.
- 3. THE CITY OF MUSKOGEE LANDFILL IS NOT PART OF THE PROPOSED EXPANSION AND IS SHOWN FOR REFERENCE PURPOSES.
- 4. PERMIT BOUNDARY AND EXISTING OVERHEAD EASEMENTS WERE REPRODUCED FROM LEGAL DESCRIPTION PREPARED BY WEAVER CONSULTANTS GROUP, SIGNED BY MICHAEL D BYTNER, LLS# 1986.
- 5. AN EASEMENT RECORD WAS NOT FOUND OR PROVIDED TO THE SURVEYOR. THE OVERHEAD POWER RUNNING ALONG THE NORTHERN AND SOUTHERN PROPERTY LINE AND NORTHWEST/SOUTHEAST ACROSS THE PROPERTY WAS FIELD IDENTIFIED AND SURVEYED.
- 6. PERMITTED FINAL COVER CONTOURS AND DRAINAGE REPRODUCED FROM THE TIER I PERMIT MODIFICATION PREPARED BY WEAVER CONSULTANTS GROUP, LLC IN NOVEMBER 2021 AND APPROVED BY ODEQ ON JANUARY 24, 2022.

ALTERNATE FINAL COVER FINAL CONTOUR PLAN	
- MUSKOGEE COMMUNITY RDF - MUSKOGEE COUNTY, OKLAHOMA	
WWW.WCGRP.COM	FIGURE A-2
	ALTERNATE FIN FINAL CONTO MUSKOGEE CO MUSKOGEE COU WWW.WCGRP.COM




1. AN ALTERNATE TO THE 2 FEET OF CLAY IS 1 FOOT OF CLAY OVERLAIN BY A GCL.

DRAFT FOR PERMITTING PURPOSES ONLY WASTE ISSUED FOR CONSTRUCTION DRAWN BY: JDW DESIGN BY: JBP REVIEWED BY: JVQ DATE: 09/2008 FILE: 0086-364-11 CAD: FIGURE A-3.DWG Weaver Consultants Group CA 3804 PE - 06/30/2025

<u>?</u>?



MANA	PREPARED FOR GEMENT OF OKLAHOMA, INC.	TIER I PERMIT MODIFICATION ALTERNATIVE FINAL COVER					
	REVISIONS	FINAL COVER DETAILS					
DATE	DESCRIPTION	MUSKOOFE OG					
/2023	REVISED DETAIL TO BE CONSISTENT WITH TIER III MODIFICATION.	MUSKOGEE COMMUNITY RDF MUSKOGEE COUNTY, OKLAHOMA					
		WWW.WCGRP.COM	FIGURE A-3				

## **APPENDIX B**

# EQUIVALENCY DESIGN DEMONSTRATION

### PRESCRIPTIVE SUBTITLE D COMPOSITE FINAL COVER SYSTEM UNSAT-H ANALYSIS

Program DATAINH Version2.03 Input Filename: S:\Nevzat\WinUnsatH\project.inp Date Processed: 15-JUL-** Time Processed: 9: 4:33 Title: Muskogee LF Subtitle D Final Cover Options chosen include: IPLANT = 1 IHEAT = 0 NGRAV = 1 LOWERH = 1 LOWER = 1UPPERH = 0ISWDIF = 1DAYEND = 365 NDAYS = 365 NYEARS = 5 ICONVH = 0 NFHOUR = 2 ITOPBC = 0 ET_OPT = 1 NPRINT = 0IRAIN = 1NSURPE = 1ICLOUD = 0 

 KOPT
 =
 4
 KEST
 =
 3
 IVAPOR
 =
 1

 INMAX
 =
 2
 INHMAX
 =
 1
 1

 HIRRI
 =
 1.00
 HDRY
 =
 1.000E-06
 HTOP
 =
 15.0

 SH OPT = 1 DHMAX = 10.0DMAXBA = 1.000E-03 DELMAX = 0.250 DELMIN = 1.000E-04 STOPHR = 24.0 OUTTIM = 0.250TORT = 0.660 TSOIL = 291. VAPDIF = 0.240 QHTOP = 0.00 IVAPOR = 1: This option allows vapor flow Saturated vapor density (g/cm3) of soil when soil temperature is a constant equal to TSOIL = 1.521E-05= 2.123E-04 = 5.702E+02 100*MOLAR*GRAV/GASCON (K/cm) VC (cm5/g/h) 

 VC (cm5/g/h) = 5.702E+02

 TGRAD = 0.00
 TSMEAN = 0.00
 TSAMP = 0.00
 QHLEAK = 0.00

 WTF = 0.500 RFACT = 1.00RAINIF = 1.000E - 03 DHFACT = 0.100MATN = 3NPT = 25 KOPT = 4: van Genuchten functions for soil hydraulic properties THETA vs H, MAT 1, AIRINT = 0.0000 THET = 0.40000THTR = 8.00000E-02ALPHA = 5.00000E-03M = 0.32432N = 1.4800K vs H, MAT 1, AIRINK = 0.0000 SK = 0.18000A = 5.00000E - 03N = 1.4800M = 0.32432KMODEL = 2.0000EPIT = 0.50000THETA vs H, MAT 2, AIRINT = 0.0000 THET = 1.00000E-02THTR = 1.00000E - 02ALPHA = 5.00000E-03

N	=	1.4800		М	=	0.32432
K vs H,	MA	т 2,				
AIRINK	-	0.0000		SK	=	1.15000E-08
A	=	5.00000E-03		N	=	1.4800
М	=	0.32432	H	MODEL	=	2.0000
EPIT		0.50000				
THETA vs H,	MA	т З,				
AIRINT	=	0.0000		THET	=	0.32000
THTR	=	0.15000		ALPHA	=	5.00000E-03
N	=	1.4800		М	===	0.32432
K vs H,	MA	т З,				
AIRINK	1000	0.0000		SK	-	3.60000E-02
А		5.00000E-03		N	=	1.4800
М	==	0.32432	E	MODEL	=	2.0000
EPIT	===	0.50000				

Surface node hydraulic properties

HIRRI = 1.0 , THETA = 0.4000, K = 1.5281E-01, C = -6.0345E-05 HDRY = 1.00E-06, THETA = 0.4000, K = 1.8000E-01, C = -7.9592E-08 NDAY = 0

NODE	Z	MAT	HEAD	CONDUCTIVITY	CAPACITY	THETA	TEMP
1	0.00	1	1.0000E+03	9.6214E-05	-6.3106E-05	0.2236	291.0
2	5.00	1	1.0000E+03	9.6214E-05	-6.3106E-05	0.2236	291.0
3	9.00	1	1.0000E+03	9.6214E-05	-6.3106E-05	0.2236	291.0
4	12.00	1	1.0000E+03	9.6214E-05	-6.3106E-05	0.2236	291.0
5	15.00	1	1.0000E+02	2.1082E-02	-3.6699E-04	0.3697	291.0
6	18.00	1	1.0000E+02	2.1082E-02	-3.6699E-04	0.3697	291.0
7	21.00	1	1.0000E+02	2.1082E-02	-3.6699E-04	0.3697	291.0
8	25.00	1	1.0000E+02	2.1082E-02	-3.6699E-04	0.3697	291.0
9	30.00	1	1.0000E+02	2.1082E-02	-3.6699E-04	0.3697	291.0
10	30.50	1	1.0000E+02	2.1082E-02	-3.6699E-04	0.3697	291.0
11	30.60	2	1.0000E+02	1.3469E-09	0.0000E+00	0.0100	291.0
12	30.70	3	1.0000E+02	4.2164E-03	-1.9496E-04	0.3039	291.0
13	35.00	3	1.0000E+02	4.2164E-03	-1.9496E-04	0.3039	291.0
14	40.00	3	1.0000E+02	4.2164E-03	-1.9496E-04	0.3039	291.0
15	45.00	3	1.0000E+02	4.2164E-03	-1.9496E-04	0.3039	291.0
16	50.00	3	1.0000E+02	4.2164E-03	-1.9496E-04	0.3039	291.0
17	55.00	3	1.0000E+02	4.2164E-03	-1.9496E-04	0.3039	291.0
18	60.00	3	1.0000E+02	4.2164E-03	-1.9496E-04	0.3039	291.0
19	65.00	3	1.0000E+02	4.2164E-03	-1.9496E-04	0.3039	291.0
20	70.00	3	1.0000E+02	4.2164E-03	-1.9496E-04	0.3039	291.0
21	75.00	3	1.0000E+02	4.2164E-03	-1.9496E-04	0.3039	291.0
22	80.00	3	1.0000E+02	4.2164E-03	-1.9496E-04	0.3039	291.0
23	85.00	3	1.0000E+02	4.2164E-03	-1.9496E-04	0.3039	291.0
24	90.00	3	1.0000E+02	4.2164E-03	-1.9496E-04	0.3039	291.0
25	91.50	3	1.0000E+02	4.2164E-03	-1.9496E-04	0.3039	291.0

Total Initial Storage = 27.8173 cm

IPLANT = 1

Total number of Growth Day - Leaf Area Index (LAI) data pairs = 5

Growth Day	LAI
0	0.100
81	2.000
320	2.000
335	2.000
364	0.100

BARE =	0.050										
DAY	LAI	DAY	LAI	DAY	LAI	DAY	LAI	DAY	LAI	DAY	LAI
1	0.123	2	0.147	3	0.170	4	0.194	5	0.217	6	0.241
7	0.264	8	0.288	9	0.311	10	0.335	11	0.358	12	0.381
13	0.405	14	0.428	15	0.452	16	0.475	17	0.499	18	0.522
19	0.546	20	0.569	21	0.593	22	0.616	23	0.640	24	0.663
25	0.686	26	0.710	27	0.733	28	0.757	29	0.780	30	0.804
31	0.827	32	0.851	33	0.874	34	0.898	35	0.921	36	0.944
37	0.968	38	0.991	39	1.015	40	1.038	41	1.062	42	1.085
43	1.109	44	1.132	45	1.156	46	1.179	47	1.202	48	1.226
49	1.249	50	1.273	51	1.296	52	1.320	53	1.343	54	1.367
55	1.390	56	1.414	57	1.437	58	1.460	59	1.484	60	1.507
61	1.531	62	1.554	63	1.578	64	1.601	65	1.625	66	1.648
67	1.672	68	1.695	69	1.719	70	1.742	71	1.765	72	1.789
73	1.812	74	1.836	75	1.859	76	1.883	77	1.906	78	1.930
79	1.953	80	1.977	81	2.000	82	2.000	83	2.000	84	2.000
85	2.000	86	2.000	87	2.000	88	2.000	89	2.000	90	2.000
91	2.000	92	2.000	93	2.000	94	2.000	95	2.000	96	2.000
97	2.000	98	2.000	99	2.000	100	2.000	101	2.000	102	2.000
103	2.000	104	2.000	105	2.000	106	2.000	107	2.000	108	2.000
109	2.000	110	2.000	111	2.000	112	2.000	113	2.000	114	2.000
115	2.000	116	2.000	117	2.000	118	2.000	119	2.000	120	2.000
121	2.000	122	2.000	123	2.000	124	2.000	125	2.000	126	2.000
127	2.000	128	2.000	129	2.000	130	2.000	131	2.000	132	2.000
133	2.000	134	2.000	135	2.000	136	2.000	137	2.000	138	2.000
139	2.000	140	2.000	141	2.000	142	2.000	143	2.000	144	2.000
145	2.000	146	2.000	147	2.000	148	2.000	149	2.000	150	2.000
151	2.000	152	2.000	153	2.000	154	2.000	155	2.000	156	2.000
157	2.000	158	2.000	159	2.000	160	2.000	161	2.000	162	2.000
163	2.000	164	2.000	165	2.000	166	2.000	167	2.000	168	2.000
169	2.000	170	2.000	171	2.000	172	2.000	173	2.000	174	2.000
175	2.000	176	2.000	177	2.000	178	2.000	179	2.000	180	2.000
181	2.000	182	2.000	183	2.000	184	2.000	185	2.000	186	2.000
187	2.000	188	2.000	189	2.000	190	2.000	191	2.000	192	2.000
193	2.000	194	2.000	195	2.000	196	2.000	197	2.000	198	2.000
199	2.000	200	2.000	201	2.000	202	2.000	203	2.000	204	2.000
205	2.000	206	2.000	207	2.000	208	2.000	209	2.000	210	2.000
211	2.000	212	2.000	213	2.000	214	2.000	215	2.000	216	2.000
217	2.000	218	2.000	219	2.000	220	2.000	221	2.000	222	2.000
223	2.000	224	2.000	225	2.000	226	2.000	227	2.000	228	2.000
229	2.000	230	2.000	231	2.000	232	2.000	233	2.000	234	2.000
235	2.000	236	2.000	237	2.000	238	2.000	239	2.000	240	2.000
241	2.000	242	2.000	243	2.000	244	2.000	245	2.000	246	2.000

247 253 259 265 271 277	2.000 2.000 2.000 2.000 2.000 2.000	248 254 260 266 272 278	2.000 2.000 2.000 2.000 2.000 2.000	249 255 261 267 273 279	2.000 2.000 2.000 2.000 2.000 2.000	250 256 262 268 274 280	2.000 2.000 2.000 2.000 2.000 2.000	251 257 263 269 275 281	2.000 2.000 2.000 2.000 2.000 2.000	252 258 264 270 276 282	2.000 2.000 2.000 2.000 2.000 2.000
283	2.000	284	2.000	285	2.000	286	2.000	287	2.000	288	2.000
209	2.000	290 296	2.000	291	2.000	292	2.000	293	2.000	294 300	2.000
301	2.000	302	2.000	303	2.000	304	2.000	305	2.000	306	2.000
307	2.000	308	2.000	309	2.000	310	2.000	311	2.000	312	2.000
313	2.000	314	2.000	315	2.000	316	2.000	317	2.000	318	2.000
319	2.000	320	2.000	321	2.000	322	2.000	323	2.000	324	2.000
325	2.000	326	2.000	327	2.000	328	2.000	329	2.000	330	2.000
331	2.000	332	2.000	333	2.000	334	2.000	335	2.000	336	1.934
337	1.869	338	1.803	339	1.738	340	1.672	341	1.607	342	1.541
343	1.476	344	1.410	345	1.345	346	1.279	347	1.214	348	1.148
349	1.083	350	1.017	351	0.952	352	0.886	353	0.821	354	0.755
355	0.690	356	0.624	357	0.559	358	0.493	359	0.428	360	0.362
361	0.297	362	0.231	363	0.166	364	0.100	365	0.000		

NFROOT = 1: Negative exponential representation of root growth

AA	(intersection of the curve at z=0 with abscissa)	=	1.300
В1	(coefficient defining degree of curvature)	==	0.13000
В2	(coefficient that determines the value of asymptote	==	0.020

Root depth, density, and weight/node versus depth

DAY	MAX	ROOT	NORMALIZED
	ROOT DEPTH	DENSITY	DENSITY
		(cm/cm)	(1/cm)
1	0.00	0.000	0.0000
1	5.00	0.699	0.1718
1	9.00	0.423	0.1041
1	12.00	0.293	0.0721
1	15.00	0.205	0.0504

MXROOT (deepest node to which roots penetrate) = 5NUPTAK = 1: Feddes et al. 1975 moisture dependent sink term

For Material No. 1

THETAW (wilting point moisture content)=0.1203THETAD (lower limit of optimum moisture content)=0.1667THETAN (upper limit of optimum moisture content)=0.3988

For Material No. 2

THETAW	(wiltir	ng poir	nt r	noisture	content)		=	0.0100
THETAD	(lower	limit	of	optimum	moisture	content)	=	0.0100
THETAN	(upper	limit	of	optimum	moisture	content)	=	0.0100

For Material No. 3

THETAW (wilting point moisture content) = 0.1714THETAD (lower limit of optimum moisture content) = 0.3194THETAN (upper limit of optimum moisture content) = 0.3194NFHOUR = 2: User subroutine for hourly PET distribution  $0.0100 \quad 0.0100 \quad 0.0100 \quad 0.0100 \quad 0.0100 \quad 0.0150 \quad 0.0440$  $0.0699 \quad 0.0911 \quad 0.1061 \quad 0.1139 \quad 0.1139 \quad 0.1061 \quad 0.0911 \quad 0.0699$  $0.0440 \quad 0.0150 \quad 0.0100 \quad 0.0100 \quad 0.0100 \quad 0.0100 \quad 0.0100 \quad 0.0100$ ET_OPT = 1 and IHEAT = 0: PET calculated from meteorological data using subroutine CALPEN ALBEDO = 2.000E-01ALT = 7.040E+02 (m)

 $\begin{array}{rcl} ALT &=& 7.040E+02 & (m) \\ ZU &=& 5.000E-01 & (m) \\ PMB &=& 1.000E+03 & (mb) \end{array}$ 

ET_OPT = 1: Meteorological Data

	Te	mpera	ture	Solar	Wind	Cloud	
IDAY	Max	Min	Dew	Rad	Speed	Cover	Prec.
	F	F	F	ly/d	mph	tenth	in
1.	29.	12.	18.6	225.	3.9	0.0	0.03
2.	24.	Ο.	8.9	268.	2.5	0.0	0.01
3.	36.	10.	20.3	273.	4.1	0.0	0.04
4.	48.	14.	25.1	275.	2.4	0.0	0.31
5.	56.	22.	26.8	283.	1.6	0.0	0.42
6.	57.	25.	29.5	245.	3.4	0.0	0.20
7.	43.	33.	31.3	153.	7.5	0.0	0.00
8.	49.	25.	27.3	286.	3.0	0.0	0.00
9.	45.	22.	26.9	285.	4.9	0.0	0.00
10.	45.	28.	30.0	150.	5.3	0.0	0.08
11.	41.	32.	35.1	90.	3.0	0.0	0.16
12.	43.	27.	36.2	69.	6.1	0.0	0.01
13.	48.	37.	39.6	25.	13.5	0.0	0.02
14.	50.	28.	31.4	292.	6.0	0.0	0.00
15.	51.	23.	27.4	269.	1.9	0.0	0.00
16.	37.	27.	29.0	61.	7.2	0.0	0.00
17.	35.	29.	28.9	84.	9.2	0.0	0.04
18.	46.	28.	26.4	205.	5.2	0.0	0.01
19.	33.	21.	17.0	269.	8.9	0.0	0.00
20.	36.	12.	13.2	293.	3.2	0.0	0.00
21.	49.	24.	24.0	253.	2.4	0.0	0.00
22.	54.	22.	26.8	314.	2.8	0.0	0.00
23.	55.	33.	35.6	159.	4.0	0.0	0.00
24.	51.	31.	29.0	456.	5.6	0.0	0.00
25.	50.	20.	22.0	334.	8.1	0.0	0.00
26.	57.	28.	32.4	326.	11.5	0.0	0.00
27.	39.	24.	24.5	221.	7.1	0.0	0.40
28.	43.	36.	37.5	23.	5.6	0.0	2.07
29.	46.	35.	36.0	92.	7.0	0.0	0.60
30.	54.	37.	31.2	270.	9.1	0.0	0.00

31.	50.	27.	22.0	352.	6.6	0.0	0.00
32.	54.	23.	25.0	324.	7.9	0.0	0.00
33.	44.	17.	16.9	857.	20.6	0.0	0.00
34.	58.	29.	30.3	884.	23.4	0.0	0.00
35.	57.	34.	30.5	950.	31.2	0.0	0.00
36	62	25	28 6	936	23 9	0.0	0.00
37	62.	36	35 0	990.	1/ 8	0.0	0.00
30	66	37	18 1	1036	14.0	0.0	0.00
20.	00. 72	57. 61	40.4	1000.	0.0	0.0	0.00
39.	73.	01.	00.4	1005	0.0	0.0	0.00
40.	69.	23.	20.7	1065.	0.0	0.0	0.00
41.	38.	20.	19.2	943.	13.2	0.0	0.00
42.	46.	26.	26.6	943.	22.5	0.0	0.00
43.	43.	38.	39.3	59.	6.5	0.0	0.01
44.	55.	42.	48.0	1055.	0.0	0.0	0.00
45.	65.	31.	50.6	1160.	0.0	0.0	0.00
46.	31.	29.	28.4	25.	5.8	0.0	0.00
47.	34.	25.	25.2	927.	33.4	0.0	0.15
48.	38.	19.	16.4	872.	20.8	0.0	0.05
49.	49.	21.	26.3	881.	24.0	0.0	0.02
50.	65.	34.	38.9	979.	25.7	0.0	0.00
51.	57.	37.	44.5	1029.	0.0	0.0	0.00
52.	46.	28.	28.8	993.	25.5	0.0	0.01
53.	48.	25.	26.2	972.	15.3	0.0	0.00
54.	44.	32.	36.9	955.	32.7	0.0	2.12
55.	65.	42.	46.3	1086.	0.0	0.0	0.00
56.	52.	36.	30.7	1072.	0.0	0.0	0.00
57.	65.	36.	37.1	1086.	0.0	0.0	0.00
58.	54.	31.	39.8	1103.	0.0	0.0	0.00
59.	31.	27.	27.8	950.	23.9	0.0	0.00
60.	43.	30.	34.5	107.	6.4	0.0	0.15
61.	47.	29.	36.7	161.	2.9	0.0	0.00
62.	53.	35.	36.2	272.	7.5	0.0	0.00
63.	58.	36.	32 0	466	74	0.0	0.00
64.	56.	28.	31 0	470	5 0	0 0	0.00
65	56	29	29.2	332	43	0.0	0 00
66	61	22.	27.2	133	1.9	0.0	0.00
67	55	32.	32 0	200	51	0.0	0.00
68	61	30	28 1	290. 484	1 1	0.0	0.00
60.	65	30.	20.1	404.	4.4	0.0	0.00
70	64	54.	12 0	409.	9.5	0.0	0.00
70.	64. E0	20.	42.9	123.	0./	0.0	0.59
11.	59. 72	39. 20	42.0	420.	5.9	0.0	0.00
12.	13.	38.	35.0	517. 016	4./	0.0	0.00
13.	юю. ГЭ	40.	45.8	210.	10.3	0.0	0.00
/4.	53.	41. 22	41.9	/3.	12.3	0.0	0.09
/5.	52.	33.	29.5	378.	7.9	0.0	0.00
/6.	54.	30.	28.6	420.	6.6	0.0	0.00
77.	50.	39.	32.9	188.	5.1	0.0	0.00
78.	55.	39.	34.6	323.	4.8	0.0	0.00
79.	62.	29.	31.2	544.	3.4	0.0	0.00
80.	72.	32.	32.8	454.	6.4	0.0	0.00
81.	76.	50.	42.2	437.	8.2	0.0	0.00
82.	74.	44.	45.0	439.	4.8	0.0	0.00
83.	59.	36.	28.1	376.	17.5	0.0	0.00
84.	45.	30.	20.4	277.	8.0	0.0	0.00
85.	48.	32.	17.2	463.	6.3	0.0	0.00
86.	50.	40.	26.4	183.	4.3	0.0	0.09
87.	45.	40.	39.8	76.	5.0	0.0	0.05

88.	50.	42.	42.5	123.	2.9	0.0	0.02
89.	65.	39.	42.7	513.	3.1	0.0	0.00
90.	62.	38.	38.7	460.	5.6	0.0	0.01
91.	69.	33.	36.8	263.	6.8	0.0	0.00
92.	73.	59.	57.4	109.	10.8	0.0	0.01
93.	87.	66.	64.5	336.	9.4	0.0	0.00
94.	89.	59.	64.4	503.	5.7	0.0	0.00
95.	80.	67.	67.3	236	10.7	0.0	0.00
96.	84.	71.	63.0	223	15.9	0.0	0.00
97	84	67	46.6	594	12 1	0.0	0 00
98	86	66	63 6	425	12.1	0.0	0.00
99.	86	68	61 5	700	9 1	0.0	0.00
100	80	70	65.8	163	10 9	0.0	0.00
101	75	54	43.7	609	18 /	0.0	0.00
102	68	42	40.7	3/9	4 1 9 1	0.0	0.40
102.	7Q	40	47 0	54J. 612	1 1	0.0	0.00
104	72.		57 Q	234	7 0	0.0	0.00
104.	72.	51	53.2	622	7.0	0.0	0.00
105.	63	JI. 45	20.0	460	7.2	0.0	0.77
107	0J. E0	40.	27.0	409.	9.7	0.0	0.00
107.	50.	20.	2E 0	600. E06	0.0 E 0	0.0	0.00
100.	07.	32. EE	55.0	200.	5.8	0.0	0.00
109.	70.	25. 70	51.9 CA 7	287.	14.6	0.0	0.00
110.	76.	70.	64./	203.	15.6	0.0	0.00
110	74.	68.	63.4	162.	13.7	0.0	0.00
112.	79.	63.	61./	150.	16.1	0.0	1.15
113.	70.	46.	41.5	638.	7.3	0.0	0.10
114.	70.	40.	41.8	657.	2.1	0.0	0.00
115.	80.	45.	46.5	665.	3.7	0.0	0.00
116.	84.	50.	50.6	640.	5.4	0.0	0.00
117.	82.	54.	50.6	657.	5.5	0.0	0.00
118.	82.	51.	53.7	626.	3.8	0.0	0.00
119.	82.	52.	52.4	661.	6.3	0.0	0.00
120.	81.	53.	52.3	535.	7.4	0.0	0.00
121.	84.	61.	59.8	620.	10.0	0.0	0.00
122.	81.	66.	62.9	494.	13.6	0.0	0.00
123.	82.	65.	63.2	483.	9.1	0.0	0.00
124.	80.	64.	62.5	356.	10.2	0.0	0.10
125.	76.	64.	63.1	390.	9.5	0.0	0.27
126.	77.	60.	62.1	353.	8.6	0.0	0.00
127.	76.	52.	52.9	579.	7.0	0.0	0.00
128.	81.	45.	51.8	694.	2.4	0.0	0.00
129.	84.	55.	59.2	680.	6.0	0.0	0.00
130.	81.	64.	62.6	327.	7.9	0.0	0.07
131.	80.	61.	63.8	273.	5.6	0.0	0.52
132.	81.	59.	60.1	693.	3.7	0.0	0.00
133.	84.	56.	62.7	604.	5.2	0.0	0.00
134.	85.	66.	64.6	636.	8.2	0.0	0.00
135.	86.	67.	63.2	683.	9.9	0.0	0.00
136.	86.	68.	67.7	490.	6.8	0.0	0.00
137.	85.	65.	69.3	465.	7.3	0.0	0.46
138.	82.	65.	65.5	499.	6.3	0.0	0.72
139.	81.	61.	65.9	462.	2.8	0.0	0.03
140.	80.	66.	67.3	372.	5.7	0.0	0.50
141.	67.	50.	49.5	369.	8.2	0.0	0.10
142.	77.	45.	45.5	718.	7.1	0.0	0.00
143.	82.	49.	50.2	699.	5.8	0.0	0.02
144.	69.	49.	42.4	656.	6.3	0.0	0.00

145.	72.	48.	46.6	660.	4.4	0.0	0.00
146.	82.	47.	53.7	676.	3.4	0.0	0.00
147.	87.	58.	63.1	532.	5.3	0.0	1.47
148.	84.	61.	63.3	712.	4.7	0.0	0.00
1/9	73	65	66.2	116	7 8	0.0	1 19
150	75.	62.	61 0	256	7.0	0.0	1 47
150.	15.	02.	04.9	200.	1.4	0.0	1.4/
151.	68.	57.	58.3	187.	4.9	0.0	0.00
152.	81.	50.	58.5	700.	4.5	0.0	0.00
153.	78.	68.	57.3	710.	6.9	0.0	0.00
154.	88.	67.	67.9	481.	9.3	0.0	0.00
155.	88.	77.	71.6	549.	11.3	0.0	0.00
156.	87.	73.	70.6	491.	9.8	0.0	0.00
157.	87.	69.	67.9	643.	5.4	0.0	0.00
158.	89.	67.	68.6	639.	6.5	0.0	0.00
159.	88.	68.	70.4	579.	4.0	0.0	0.08
160.	88.	69.	70.0	630.	3.1	0.0	0.00
161.	89	68	70.2	629	5 2	0.0	0.00
162	90. 90	69	68 9	718	63	0.0	0.00
163	90. 90	71	68 5	710.	11 /	0.0	0.00
164	90.	/⊥. 7⊑	70.1	712.	14 0	0.0	0.00
104.	90. 00	75.	72.1	559.	14.0	0.0	0.00
105.	88.	67.	/1.0	278.	10.2	0.0	0.89
166.	88.	62.	61.5	/10.	3.0	0.0	0.01
167.	90.	62.	60.7	737.	6.5	0.0	0.00
168.	89.	65.	61.7	608.	7.3	0.0	0.00
169.	90.	67.	63.9	726.	8.2	0.0	0.00
170.	90.	69.	66.8	691.	6.5	0.0	0.00
171.	85.	69.	69.0	480.	5.6	0.0	0.02
172.	80.	62.	64.8	419.	5.5	0.0	0.11
173.	84.	57.	58.8	661.	3.2	0.0	0.00
174.	88.	60.	61.8	700.	4.2	0.0	0.00
175.	90.	63.	63.9	704.	5.0	0.0	0.00
176.	89.	64.	63.9	704.	5.2	0.0	0.00
177	88	63	64 7	663	4 9	0 0	0 00
178	91 91	65	67 1	652	1.5	0.0	0.00
170.	76	69.	67 2	102.	9.0	0.0	1 50
100	01	67	60 2	170	0.0	0.0	1.00
101	04.	70	70.0	4/2.	9.7	0.0	0.40
101.	85.	70.	70.2	517. CA7	4.5	0.0	0.00
182.	88.	68.	/1.4	647.	3.7	0.0	0.00
183.	89.	69.	70.0	6/6.	4./	0.0	0.00
184.	90.	68.	70.4	616.	4.9	0.0	0.00
185.	96.	70.	71.2	677.	5.8	0.0	0.00
186.	91.	75.	72.8	474.	6.3	0.0	0.00
187.	97.	73.	74.3	698.	6.3	0.0	0.00
188.	96.	77.	73.6	703.	7.5	0.0	0.00
189.	97.	74.	72.2	701.	6.9	0.0	0.00
190.	98.	74.	72.0	706.	4.6	0.0	0.00
191.	97.	72.	72.7	649.	4.0	0.0	0.00
192.	97.	73.	73.9	678.	5.1	0.0	0.00
193.	101.	77.	73.4	646.	5.8	0.0	0.00
194	82.	75	72.3	299	9.7	0.0	0.00
195	86	70	65 0	516	5 a	0.0	0 00
196	87	66	67 0	151	7 1	0.0	0.00
107	07. QA	οο. 7 Λ	72 6	4J1. 510	/ • ±	0.0	0.00
100	94. 00	74.	75.0	OTO.	9.9	0.0	0.10
190.	Уð.	10.	10.0	064.	8.5	0.0	0.00
199.	98.	/6.	/4.6	663.	8.0	0.0	0.00
200.	97.	79.	73.7	667.	7.2	0.0	0.00
201.	95.	75.	71.4	665.	6.3	0.0	0.00

202.	100.	73.	71.6	674.	5.3	0.0	0.00
203.	100.	74.	70.5	686.	6.4	0.0	0.00
204.	100.	73.	70.3	666.	6.2	0.0	0.00
205.	100.	75.	73.3	656.	4.3	0.0	0.00
206.	99.	79.	72.7	603.	6.2	0.0	0.00
207.	97.	75.	73.8	513.	4.7	0.0	0.00
208	95	78	73.8	410	6 1	0 0	0 00
209	97	77	73 5	553	65	0 0	0.06
210	95	75	7/ 9	570 570	55	0.0	0.00
210.	100	76	73 0	629	J.J 7 1	0.0	0.00
212	100. QQ	70.	73.0	651	7.1	0.0	0.00
212	99. QQ	77.	60 0	660	7.2	0.0	0.00
213.	99. 07	72.	71 0	621	J.7 E 0	0.0	0.00
214. 015	97.	74. 60	/1.9 CO C	642	5.0	0.0	0.00
210	101	70	60.0	640	2.1	0.0	0.00
210.	101.	70.	69.0	649.	2.6	0.0	0.00
217.	101.	/4.	70.4	651.	3.9	0.0	0.00
218.	98.	68.	66.2	641.	3.9	0.0	0.00
219.	97.	70.	/0.3	609.	3.0	0.0	0.00
220.	98.	13.	/0.0	555.	4.5	0.0	0.00
221.	104.	79.	68.7	595.	6.8	0.0	0.00
222.	98.	74.	71.6	505.	4.2	0.0	0.28
223.	88.	73.	74.1	335.	2.1	0.0	0.00
224.	94.	73.	73.4	495.	3.4	0.0	0.00
225.	96.	70.	66.7	597.	4.5	0.0	0.00
226.	95.	66.	63.0	637.	3.9	0.0	0.00
227.	95.	70.	68.0	419.	9.3	0.0	0.73
228.	91.	68.	68.1	585.	3.1	0.0	0.00
229.	93.	67.	69.1	590.	4.4	0.0	0.00
230.	93.	68.	69.7	581.	5.3	0.0	0.00
231.	92.	66.	59.7	627.	5.3	0.0	0.00
232.	98.	67.	65.6	557.	6.5	0.0	0.00
233.	96.	73.	67.6	570.	9.2	0.0	0.00
234.	96.	78.	68.3	585.	5.1	0.0	0.00
235.	90.	74.	69.5	218.	6.3	0.0	0.32
236.	92.	73.	68.0	388.	8.7	0.0	0.07
237.	97.	73.	70.4	395.	7.2	0.0	0.49
238.	92.	69.	70.3	533.	4.0	0.0	0.00
239.	94.	71.	70.8	606.	2.6	0.0	0.00
240.	93.	68.	70.0	591.	3.8	0.0	0.00
241.	90.	68.	68.3	555.	5.4	0.0	0.00
242.	86.	73.	69.5	492.	7.0	0.0	0.00
243.	89.	66.	65.9	492.	4.9	0.0	0.00
244.	90.	66.	67.5	571.	4.0	0.0	0.00
245.	92.	67.	68.6	581.	3.4	0.0	0.00
246.	94.	68.	68.9	495	2.8	0.0	0.00
247	91	68	69.3	382	3 0	0.0	0 00
248	88	72	71 4	243	3.7	0.0	0.03
249	88	72.	707	402	10.2	0.0	0.00
250	91	66	70.7	380	1/ 1	0.0	0.01
251	77 77	62	61 1	213	74.7	0.0	0.41
251.	70	υ <i>2</i> . 55	56 0	21J. 520	5.9	0.0	0.00
252.	12.	10	50.2 53 0	ンイン・ 500	2.0	0.0	0.01
200.	0Z.	43. につ	55.0 56.0	900. 560	∠.V ⊃ 1	0.0	0.00
204. 255	04.	JZ.	50.0 E0 7	309. E44	3.1	0.0	0.00
200. 256	00.	33. EF	JØ./	544. EEO	2.5	0.0	0.00
200. 257	88.	55.	60.4	55Z.	2.1	0.0	0.00
257.	88.	60.	64.4	402.	4.1	0.0	0.00
208.	16.	66.	65.4	τ8/.	6.6	0.0	0.26

259.	77.	68.	68.4	144.	4.8	0.0	0.18
260.	84.	70.	69.9	232.	6.6	0.0	0.01
261.	77.	66.	67.6	252.	7.0	0.0	1.04
262	81	63	61 1	479	4 0	0 0	0 00
262.	77	50.	62 0	210	1.0	0.0	0.00
203.		50.	02.9	210.	4.0	0.0	0.00
204.	οU.	62.	05.0	315.	3.9	0.0	0.00
265.	85.	60.	66.2	405.	2.8	0.0	0.00
266.	82.	60.	61.3	437.	9.1	0.0	0.01
267.	69.	50.	47.9	418.	9.9	0.0	0.00
268.	71.	42.	45.3	527.	2.6	0.0	0.00
269.	77.	40.	48.6	515.	2.0	0.0	0.00
270.	78.	48.	51.4	490.	3.6	0.0	0.00
271.	80.	50.	53.8	484.	2.7	0.0	0.00
272.	77.	48.	51.0	498.	4.0	0.0	0.00
273	78	44	495	500	1 9	0 0	0 00
274	80	11.	10.0	193	1 7	0.0	0.00
275	Q1	44.	-1J.J 51 /	495.	<u> </u>	0.0	0.00
215.	01.	47.	51.4	400.	0.0	0.0	0.01
276.	82.	57.	55.6	436.	7.9	0.0	0.00
277.	83.	63.	63.4	378.	9.3	0.0	0.00
278.	73.	38.	48.8	167.	11.2	0.0	0.09
279.	68.	33.	38.0	483.	2.0	0.0	0.00
280.	69.	36.	40.5	466.	5.6	0.0	0.00
281.	76.	47.	48.9	445.	11.7	0.0	0.00
282.	73.	66.	61.2	126.	12.2	0.0	0.59
283.	75.	60.	63.4	27.	7.4	0.0	2.14
284.	62.	57.	58.7	57.	3.5	0.0	0.24
285.	73.	56	58 7	313	7 0	0 0	0.57
286	69	17	51 5	263	1 2	0.0	0 00
200.	71	42	11.0	203.	4.2	0.0	0.00
207.	71.	43.	44.5	407.	3.5	0.0	0.00
288.	13.	37.	44.3	2/1.	9.3	0.0	0.00
289.	66.	31.	34.8	449.	2.5	0.0	0.00
290.	70.	36.	37.5	436.	6.8	0.0	0.00
291.	75.	53.	47.2	405.	10.4	0.0	0.00
292.	68.	46.	52.5	184.	2.4	0.0	0.00
293.	82.	47.	57.0	370.	5.6	0.0	0.00
294.	80.	60.	59.4	384.	8.4	0.0	0.00
295.	80.	58.	62.8	245.	9.3	0.0	0.00
296.	85.	58.	61.2	336.	7.6	0.0	0.00
297.	72.	42.	44.9	398.	8.9	0.0	0.00
298	67	34	29.8	408	4 2	0.0	0.00
299	67	31	30.8	398	3 2	0.0	0.00
300	65	30	30.0	400	3.1	0.0	0.00
201	70	20.	24 6	400.	5.1	0.0	0.00
201.	70.	39.	34.0	380.	9.4	0.0	0.00
302.	15.	48.	40.5	333.	8.5	0.0	0.00
303.	13.	43.	41.2	378.	6.3	0.0	0.00
304.	71.	53.	49.8	220.	12.9	0.0	0.00
305.	78.	62.	60.5	148.	11.3	0.0	0.00
306.	69.	61.	62.8	86.	5.1	0.0	0.00
307.	64.	59.	61.4	46.	4.4	0.0	2.78
308.	74.	53.	59.3	214.	1.5	0.0	0.00
309.	75.	49.	55.0	233.	1.2	0.0	0.00
310.	77.	47.	53.5	294.	3.9	0.0	0.00
311.	78.	49.	48.3	337.	5.9	0.0	0.00
312	62	49	46.9	224	8.4	0.0	0.03
313	52. 58	10.	40.2	224.	л р Л	0.0	0 00
311	71 71	-0. /5	15 0	220.	2.0	0.0	0.00
914. 915	11.	40.	40.2	JJZ.	2.9	0.0	0.00
JTD.	11.	40.	40.0	313.	3.0	0.0	0.00

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		ਹਿਸ਼ਾਂਧਾ	יעסייים	טק סביט	A DO	DAV	היינס	סייף אוזכ	
	to (1	b the 1972)	relatio	onship	develop	ed by R	litchie		
	PE	ET is	partit	ioned i	nto PT	and PE	accordi	ng	
NFPET	= 1:								
365.	38.	13.	16.4	266.	1.6	0.0	0.00		
364.	27.	16.	15.1	76.	3.3	0.0	0.00		
363.	37.	20.	14.2	275.	8.6	0.0	0.00		
362.	58.	22.	26.7	230.	4.0	0.0	0.00		
361.	51.	23.	26.0	233.	2.8	0.0	0.00		
360.	39.	21.	14.7	284.	6.5	0.0	0.00		
359.	39.	16.	19.1	261.	2.4	0.0	0.00		
358.	42.	19.	16.1	289.	5.4	0.0	0.00		
357	43.	20.	14.7	279	6.1	0.0	0.00		
356	60.	28.	36.6	268	11.5	0.0	0.00		
355	63.	32	33 2	268	10 6	0.0	0.00		
354	61	20.	28.2	204.	49	0.0	0.00		
353	50. 50	28	23.5	284	7 0	0.0	0.00		
331. 350	57.	55. 21	37.0 35.5	イ10. 277	5.3 7 0	0.0	0.33		
35U. 251	とう。 につ	4/. 25	49.3	16. 276	6.1 5 0	0.0	1.49		
349.	49.	40.	42.2	52.	11.4	0.0	0.00		
348.	48.	37.	40.7	111.	6.6	0.0	0.00		
347.	49.	44.	45.0	54.	4.6	0.0	0.25		
346.	52.	46.	48.7	31.	4.0	0.0	0.13		
345.	51.	28.	35.3	141.	8.9	0.0	0.26		
344.	56.	20.	25.1	285.	5.2	0.0	0.00		
343.	55.	22.	23.4	288.	1.4	0.0	0.00		
342.	53.	28.	27.4	257.	10.0	0.0	0.00		
341.	67.	33.	43.0	206.	4.1	0.0	0.00		
340.	66.	38.	39.6	140.	3.0	0.0	0.00		
339.	75.	51.	53.6	95.	12.2	0.0	0.00		
338.	75.	61.	60.6	155.	12.6	0.0	0.00		
337.	62.	53.	54.8	71.	9.8	0.0	0.00		
336.	71.	33.	44.3	280.	7.6	0.0	0.00		
335.	62.	28.	37.4	199.	3.1	0.0	0.00		
334.	57.	30.	32.6	291.	7.9	0.0	0.31		
222. 222	30. 34	20. 28	23.0	55. 51	13.1 67	0.0	0.00		
33⊺. 337.	39. 30	29. 26	25 Q	10Z. 55	0.4 12 1	0.0	0.00		
33U. 331	63. 20	38. 20	3/.⊥ 21 2	8⊥. 160	11.3	0.0	0.00		
329.	10.	პქ. ვი	31.5 27 1	294.	8.2	0.0	0.00		
328.	59.	48.	41.7 27 5	60.	11.3	0.0	0.00		
327.	73.	53.	55.3	122.	13.2	0.0	0.00		
326.	67.	41.	42.4	283.	11.2	0.0	0.00		
325.	59.	28.	29.5	252.	5.3	0.0	0.00		
324.	56.	26.	26.2	308.	2.6	0.0	0.00		
323.	61.	32.	34.0	254.	11.9	0.0	0.35		
322.	74.	55.	57.1	222.	8.2	0.0	0.41		
321.	67.	56.	53.2	104.	3.7	0.0	0.00		
320.	69.	45.	50.9	139.	1.9	0.0	0.00		
319.	72.	46.	51.0	275.	2.9	0.0	0.00		
318.	72.	55.	54.8	226.	8.2	0.0	0.00		
317.	72.	48.	53.2	225.	8.6	0.0	0.00		
316.	68.	45.	47.4	218.	7.0	0.0	0.00		

11 of 19

1	0.0344	0.0000	0.0344	2	0.0446	0.0000	0.0446
3	0.0652	0.0000	0.0652	4	0.0920	0.0000	0.0920
5	0 1353	0 0000	0 1353	6	0 1549	0 0000	0 1549
7	0.1085	0.0000	0.1085	8	0.1473	0.0000	0.1473
,	0.1202	0.0000	0.1202	10	0.1475	0.0000	0.1475
11	0.1392	0.0000	0.1392	10	0.0714	0.0000	0.0714
	0.0000	0.0000	0.0000	12	0.0000	0.0000	0.0000
13	0.0037	0.0000	0.0037	14	0.1895	0.0000	0.1895
15	0.1157	0.0000	0.1157	16	0.0000	0.0000	0.0000
17	0.0122	0.0000	0.0122	18	0.1497	0.0000	0.1497
19	0.2042	0.0000	0.2042	20	0.1231	0.0000	0.1231
21	0.1310	0.0000	0.1310	22	0.1736	0.0000	0.1736
23	0.0905	0.0000	0.0905	24	0.3505	0.0000	0.3505
25	0.3079	0.0000	0.3079	26	0.3789	0.0000	0.3789
27	0.1306	0.0000	0.1306	28	0.0000	0.0000	0.0000
29	0.0307	0.0000	0.0307	30	0.3811	0.0000	0.3811
31	0.3426	0.0000	0.3426	32	0.3257	0.0000	0.3257
33	0.8340	0.0000	0.8340	34	1.1620	0.0000	1.1620
35	1.5681	0.0000	1.5681	36	1.2792	0.0000	1.2792
37	1 1146	0 0000	1 1146	38	0 6816	0 0000	0 6816
30	1 0053	0.0000	1 0053	40	0.6010	0.0000	0.6911
11	0 6259	0.0000	1.0055	40	0.0911	0.0000	0.0911
12	0.0259	0.0000	0.0259	42	0.0055	0.0000	0.0059
4.5	0.0000	0.0000	0.0000	44	0.0000	0.0000	0.6556
40	0.7157	0.0000	0.7157	46	0.0000	0.0000	0.0000
4 /	0.6604	0.0000	0.6604	48	0.7685	0.0000	0.7685
49	0.8301	0.0000	0.8301	50	1.2925	0.0000	1.2925
51	0.6273	0.0000	0.6273	52	0.9353	0.0000	0.9353
53	0.8044	0.0000	0.8044	54	0.5666	0.0000	0.5666
55	0.7538	0.0000	0.7538	56	0.6519	0.0000	0.6519
57	0.7344	0.0000	0.7344	58	0.6304	0.0000	0.6304
59	0.4525	0.0000	0.4525	60	0.0000	0.0000	0.0000
61	0.0021	0.0000	0.0021	62	0.2246	0.0000	0.2246
63	0.4896	0.0000	0.4896	64	0.3410	0.0000	0.3410
65	0.2563	0.0000	0.2563	66	0.3918	0.0000	0.3918
67	0.2355	0.0000	0.2355	68	0.4158	0.0000	0.4158
69	0.6366	0.0000	0.6366	70	0.3189	0.0000	0.3189
71	0.3127	0.0000	0.3127	72	0.5490	0.0000	0.5490
73	0.2615	0.0000	0.2615	74	0.1037	0.0000	0.1037
75	0.3805	0.0000	0.3805	76	0.3753	0.0000	0.3753
77	0.1698	0.0000	0.1698	78	0.2764	0.0000	0.2764
79	0.4015	0.0000	0.4015	80	0.5375	0.0000	0.5375
81	0.7257	0.5377	0.1880	82	0.4540	0.3364	0.1176
83	0.8822	0.6537	0.2285	84	0.3375	0.2501	0.0874
85	0.4707	0.3488	0.1219	86	0.2076	0.1538	0.0538
87	0.0000	0.0000	0.0000	88	0.0020	0.0015	0.0005
89	0.3704	0.2744	0.0959	90	0.4033	0.2988	0.1045
91	0.3358	0.2488	0.0870	92	0.2821	0.2090	0.0731
93	0.6193	0.4589	0.1604	94	0.5844	0.4330	0.1514
95	0.3606	0.2672	0.0934	96	0.8724	0.6464	0.2260
97	1,4021	1.0389	0 3632	98	0.8415	0.6235	0.2180
99	0 9910	0 7343	0 2567	100	0 4049	0 3000	0 1049
101	1 4297	1 0593	0.3704	100	0.4977	0 3688	0.1289
103	0.5631	0.4172	0 1459	104	0.1902	0.1409	0.0493
105	0.6423	0 4759	0 1664	106	0 6165	0 4568	0 1597
107	0 7052	0 5225	0 1827	108	0 5406	0 4006	0 1400
100	0 5963	0 1110	0.15/5	110	0.5212	0 2026	0 1376
111	0 2971	0 2011	0.1020	110	0.5242	0 2800	0 1262
11 R	0 7/30	0.2944	0.1029	11 <i>1</i>	0.5061	0 3750	0.1211
T T O	0.1400	0.0011	V. 1921	114	0.0001	0.0700	A. TOTT

115	0.6527	0.4836	0.1691	116	0.7484	0.5545	0.1939
117	0 7901	0 5854	0 2047	118	0 6231	0 4617	0 1614
11Q	0 7811	0.5788	0.2017	120	0.7122	0 5277	0 1845
101	0.0011	0.5700	0.2025	120	0.7122	0.0277	0.1045
121	0.9063	0.6715	0.2348	122	0.8571	0.6351	0.2220
123	0.6854	0.5079	0.1//6	124	0.5626	0.4169	0.145/
125	0.4837	0.3584	0.1253	126	0.4056	0.3005	0.1051
127	0.6455	0.4783	0.1672	128	0.5944	0.4404	0.1540
129	0.7397	0.5481	0.1916	130	0.4770	0.3534	0.1236
131	0.2837	0.2102	0.0735	132	0.6780	0.5024	0.1756
133	0.5976	0.4428	0.1548	134	0.8391	0.6217	0.2174
135	1.0227	0.7578	0.2649	136	0.6221	0.4609	0.1611
137	0 5109	0 3786	0 1323	138	0 5629	0 4171	0 1458
139	0 3874	0 2870	0.1003	140	0.3751	0.2779	0.1400
1/1	0.0074	0.2070	0.1071	140	0.0701	0.6077	0.0072
141	0.4134	0.3063	0.1071	142	0.8201	0.6077	0.2125
143	0.7913	0.5863	0.2050	144	0.7289	0.5401	0.1888
145	0.6285	0.4657	0.1628	146	0.6186	0.4583	0.1602
147	0.5850	0.4335	0.1515	148	0.7391	0.5477	0.1915
149	0.0709	0.0526	0.0184	150	0.2189	0.1622	0.0567
151	0.1140	0.0845	0.0295	152	0.6301	0.4669	0.1632
153	0.9309	0.6898	0.2412	154	0.7048	0.5222	0.1826
155	0.9301	0.6892	0.2409	156	0.7459	0.5527	0.1932
157	0.7551	0.5595	0.1956	158	0.7739	0.5734	0.2005
159	0 6097	0 4518	0 1579	160	0 6518	0 4829	0 1688
161	0.7081	0.5247	0.1934	162	0.0010	0.4020	0.2204
1 ( )	1 1100	0.0247	0.1034	102	1.0000	0.0301	0.2294
103	1.1189	0.8291	0.2899	164	1.0209	0.7565	0.2645
165	0.4273	0.3166	0.1107	166	0./48/	0.554/	0.1939
167	0.9632	0.7137	0.2495	168	0.8810	0.6528	0.2282
169	1.0437	0.7734	0.2704	170	0.9056	0.6710	0.2346
171	0.5513	0.4085	0.1428	172	0.4140	0.3067	0.1072
173	0.6546	0.4850	0.1696	174	0.7641	0.5662	0.1979
175	0.8270	0.6128	0.2142	176	0.8353	0.6189	0.2164
177	0.7473	0.5537	0.1936	178	0.7491	0.5550	0.1940
179	0.2159	0.1600	0.0559	180	0.6179	0.4579	0.1601
181	0 5507	0 4081	0 1427	182	0 6576	0 4872	0 1703
183	0.7570	0.5609	0.1961	184	0.6957	0.5155	0 1802
105	0.7570	0.5009	0.1901	104	0.0957	0.01057	0.1602
107	0.0795	0.0317	0.2270	100	1 0240	0.4057	0.1090
107	0.9202	0.6819	0.2384	188	1.0349	0.7668	0.2681
189	1.0046	0./443	0.2602	190	0.9197	0.6814	0.2382
191	0.7917	0.5866	0.2051	192	0.8609	0.6379	0.2230
193	0.9664	0.7160	0.2503	194	0.4335	0.3212	0.1123
195	0.6941	0.5143	0.1798	196	0.5936	0.4398	0.1538
197	0.8362	0.6196	0.2166	198	1.0559	0.7824	0.2735
199	1.0075	0.7465	0.2610	200	1.0264	0.7605	0.2659
201	0.9393	0.6960	0.2433	202	0.9369	0.6942	0.2427
203	1.0363	0.7679	0.2685	204	0.9943	0.7367	0.2576
205	0 8685	0 6435	0 2250	206	0 9519	0 7053	0 2466
207	0 6932	0.5136	0.1796	200	0.6550	0.4853	0 1697
207	0.0000	0.5150	0.1790	200	0.0550	0.4055	0.1057
209	0.0400	0.0209	0.2192	210	0.7557	0.5564	0.1952
211	0.9938	0./363	0.2574	212	1.0591	0.7848	0.2/44
213	0.9442	0.6996	0.2446	214	0.8850	0.6557	0.2293
215	0.7401	0.5484	0.1917	216	0.7783	0.5767	0.2016
217	0.8736	0.6473	0.2263	218	0.8200	0.6076	0.2124
219	0.7123	0.5278	0.1845	220	0.7677	0.5688	0.1989
221	1.1037	0.8178	0.2859	222	0.6909	0.5119	0.1790
223	0.3156	0.2339	0.0818	224	0.5781	0.4283	0.1497
225	0.7996	0.5925	0.2071	226	0.8013	0.5937	0.2076
227	0.8153	0.6041	0.2112	228	0.6433	0.4767	0.1667

229	0.6946	0.5147	0.1799	230	0.7213	0.5345	0.1869	
231	0 8749	0 6483	0 2266	232	0 8671	0 6425	0 2246	
222	1 0454	0.0405	0.2200	202	0.00/1	0.0425	0.2240	
200	1.0454	0.7740	0.2708	234	0.0040	0.6556	0.2292	
235	0.4236	0.3138	0.1097	236	0.7541	0.5588	0.1954	
237	0.7128	0.5281	0.1846	238	0.6166	0.4569	0.1597	
239	0.6723	0.4982	0.1742	240	0.6731	0.4987	0.1744	
241	0.6841	0.5069	0.1772	242	0.6660	0.4935	0.1725	
243	0.6043	0.4477	0.1565	244	0.6399	0.4742	0.1658	
245	0.6446	0.4776	0.1670	246	0.5561	0.4121	0.1441	
247	0.4198	0.3110	0.1087	248	0.2844	0.2107	0.0737	
249	0.6649	0.4927	0.1722	250	0.7054	0.5226	0 1827	
251	0 1615	0 1197	0 0418	252	0 5604	0 4152	0 1452	
253	0.1019	0.3763	0.1316	252	0.5406	0.4102	0.1401	
200	0.5079	0.3703	0.1310	254	0.5400	0.4000	0.1401	
200	0.5107	0.3843	0.1344	256	0.5131	0.3802	0.1329	
257	0.4341	0.3217	0.1125	258	0.2004	0.1485	0.0519	
259	0.1046	0.0775	0.0271	260	0.3088	0.2288	0.0800	
261	0.2311	0.1712	0.0599	262	0.5095	0.3775	0.1320	
263	0.1579	0.1170	0.0409	264	0.2702	0.2002	0.0700	
265	0.3548	0.2629	0.0919	266	0.6048	0.4481	0.1567	
267	0.5710	0.4230	0.1479	268	0.4115	0.3049	0.1066	
269	0.3871	0.2868	0.1003	270	0.4552	0.3373	0.1179	
271	0.4293	0.3181	0.1112	272	0.4714	0.3493	0.1221	
273	0 3963	0 2937	0 1027	274	0 3929	0 2911	0 1018	
275	0.5505	0.2007	0.1424	274	0.5525	0.2011	0.1674	
213	0.5557	0.4103	0.1434	270	0.0401	0.4700	0.1074	
277	0.0001	0.4194	0.1466	278	0.2359	0.1748	0.0611	
279	0.3330	0.2467	0.0863	280	0.4341	0.3216	0.1124	
281	0.6975	0.5168	0.1807	282	0.3427	0.2539	0.0888	
283	0.0145	0.0108	0.0038	284	0.0000	0.0000	0.0000	
285	0.3002	0.2224	0.0778	286	0.1890	0.1400	0.0490	
287	0.3567	0.2643	0.0924	288	0.3780	0.2801	0.0979	•
289	0.3196	0.2368	0.0828	290	0.5014	0.3715	0.1299	
291	0.7384	0.5471	0.1913	292	0.0683	0.0506	0.0177	
293	0.3565	0.2641	0.0923	294	0.5466	0.4050	0.1416	
295	0.3154	0.2337	0 0817	296	0.4782	0.3543	0 1239	
297	0 5144	0 3812	0.001333	298	0 4111	0 3046	0 1065	
299	0.3368	0.2495	0.1933	300	0.3168	0.2348	0.1000	
299	0.5500	0.2495	0.0072	200	0.5100	0.2340	0.0021	
202	0.6412	0.4/51	0.1661	302	0.6391	0.4735	0.1656	
303	0.4886	0.3620	0.1266	304	0.5330	0.3949	0.1381	
305	0.3829	0.2837	0.0992	306	0.0000	0.0000	0.0000	
307	0.0000	0.0000	0.0000	308	0.0963	0.0713	0.0249	
309	0.1217	0.0902	0.0315	310	0.2510	0.1860	0.0650	
311	0.4512	0.3343	0.1169	312	0.2730	0.2023	0.0707	
313	0.1789	0.1326	0.0464	314	0.2793	0.2070	0.0724	
315	0.2241	0.1660	0.0581	316	0.2495	0.1848	0.0646	
317	0.2563	0.1899	0.0664	318	0.3105	0.2300	0.0804	
319	0.1929	0.1429	0.0500	320	0.0373	0.0277	0.0097	
321	0.0752	0.0557	0.0195	322	0.2803	0.2077	0.0726	
323	0 1231	0.3135	0.01096	324	0 1968	0 1458	0.0510	
325	0 2/15	0 1700	0 0626	206	0 1572	0.7300	0 1104	
307	0.2410	0.103	0.0020	220	0.4012	0.000	0.1104	
321	0.30/0	0.22/9	0.0/9/	328	0.2044	0.2107	0.0131	
329	0.4053	0.3003	0.1050	330	0.3241	0.2401	0.0840	
331	0.2078	0.1539	0.0538	332	0.0000	0.0000	0.0000	
333	0.0000	0.0000	0.0000	334	0.2953	0.2188	0.0765	
335	0.0960	0.0711	0.0249	336	0.2655	0.0000	0.2655	
337	0.0183	0.0000	0.0183	338	0.3385	0.0000	0.3385	
339	0.3181	0.0000	0.3181	340	0.1046	0.0000	0.1046	
341	0.1241	0.0000	0.1241	342	0.3445	0.0000	0.3445	

343	0.1436	0.0000	0.1436	34	4 0.2246	0.0000	0.2246
345	0.0740	0.0000	0.0740	34	6 0.0000	0.0000	0.0000
347	0.0000	0.0000	0.0000	34	8 0.0000	0.0000	0.0000
349	0.0000	0.0000	0.0000	35	0 0.0000	0.0000	0.0000
351	0.2005	0.0000	0.2005	35	2 0.3478	0.0000	0.3478
353	0.3040	0.0000	0.3040	35	4 0.2643	0.0000	0.2643
355	0.4404	0.0000	0.4404	35	6 0.2832	0.0000	0.2832
357	0.2536	0.0000	0.2536	35	8 0.2110	0.0000	0.2110
359	0.0921	0.0000	0.0921	36	0 0.2384	0.0000	0.2384
361	0.1201	0.0000	0.1201	36	2 0.1714	0.0000	0.1714
363	0.2621	0.0000	0.2621	36	4 0.0000	0.0000	0.0000
365	0.0829	0.0000	0.0829				

Totals: PET = 148.7921 PTRANS = 108.1140 PEVAPO = 75.0677

#### IRAIN = 1

### Rainfall/Irrigation Details

Day	Time (hr)	Amount (cm)	Application Type	Efficiency	Changes In Rate/Head
1	0.000	0.0762	1	1.000	2
2	0.000	0.0000	1	1.000	2
3	0.000	0.1016	1	1.000	2
4	0.000	0.0000	1	1.000	2
5	0.000	1.0000 0.0668	1	1.000	3
6	2.000	0.0000 0.5080	1	1.000	2
10	1.000	0.0000	1	1.000	2
11	0.000	0.4064	1	1.000	2
12	0.000	0.0254	1	1.000	2
13	0.000	0.0508	1	1.000	2
17	0.000 1.000	0.1016 0.0000	1	1.000	2
18	0.000 1.000	0.0254 0.0000	1	1.000	2
27	0.000 1.000	1.0000 0.0160	1	1.000	3
28	2.000 0.000 5.000	0.0000 5.0000 0.2578	1	1.000	3
29	6.000 0.000 1.000	0.0000 1.0000 0.5240	1	1.000	3

	2.000	0.0000			
43	0.000	0.0254	1	1.000	2
	1.000	0.0000	_		
47	0.000	0.3810	1	1.000	2
	1.000	0.0000			
48	0.000	0.1270	1	1.000	2
	1.000	0.0000			
49	0.000	0.0508	1	1.000	2
	1.000	0.0000			
52	0.000	0.0254	1	1.000	2
	1.000	0.0000			
54	0.000	5.0000	1	1.000	3
	5.000	0.3848			
	6.000	0.0000			
60	0.000	0.3810	1	1.000	2
	1.000	0.0000			
70	0.000	1.0000	1	1.000	3
	1.000	0.4986			
	2.000	0.0000		1	0
71	0.000	0.1524	1	1.000	2
7 4	1.000	0.0000	-	1 000	0
/4	0.000	0.2286	T	1.000	Z
0.0	1.000	0.0000	٦	1 000	0
86	1 000	0.2286	Т.	1.000	Z
07	1.000	0.0000	1	1 000	2
07	1 000	0.1270	Д.	1.000	2
88	1.000	0.0000	1	1 000	2
00	1 000	0.0000	Ŧ	1.000	2
90	0.000	0.0254	1	1.000	2
5.0	1.000	0.0000	-	1.000	
92	0.000	0.0254	1	1.000	2
	1.000	0.0000			
101	0.000	1.0000	1	1.000	3
	1.000	0.2446			
	2.000	0.0000			
105	0.000	1.0000	1	1.000	3
	1.000	0.9558			
	2.000	0.0000			
112	0.000	2.0000	1	1.000	3
	2.000	0.9210			
	3.000	0.0000			
113	0.000	0.2540	1	1.000	2
	1.000	0.0000	_		0
124	0.000	0.2540	1	1.000	2
105	1.000	0.0000	-	1 000	0
125	0.000	0.6858	T	1.000	2
120	1.000	0.0000	1	1 000	2
130	1 000	0.1778	T	1.000	Z
1 3 1	1.000	1 0000	1	1 000	з
TOT	1 000	1.0000	T	T.000	5
	2 000	0.0200			
137	0.000	1.0000	1	1.000	З
	1.000	0.1684	±		0
	2.000	0.0000			
138	0.000	1.0000	1	1.000	3

	1.000	0.8288			•
	2.000	0.0000			
139	0.000	0.0762	1	1.000	2
	1.000	0.0000			
140	0.000	1.0000	1	1.000	3
	1.000	0.2700			
	2.000	0.0000			
141	0.000	0.2540	1	1,000	2
	1.000	0.0000			
143	0.000	0 0508	1	1 000	2
210	1,000	0.0000	±	1.000	2
147	0 000	3 0000	1	1 000	З
	3 000	0 7338	±	1.000	5
	4 000	0.0000			
149	0 000	3 0000	1	1 000	З
117	3 000	0.0226	1	1.000	5
	4 000	0.0220			
150	4.000	3,0000	1	1 000	З
100	3 000	0 7220	T	1.000	J
	3.000	0.7550			
150	4.000	0.0000	1	1 000	2
199	1 000	0.2032	T	1.000	Z
165	1.000	0.0000	1	1 000	2
100	2 000	2.0000	1.	1.000	2
	2.000	0.2606			
100	3.000	0.0000	1	1 000	0
100	1.000	0.0254	1	1.000	Z
171	1.000	0.0000	1	1 000	2
1/1	1.000	0.0508	Т	1.000	2
170	1.000	0.0000	1	1 000	0
1/2	1 000	0.2794	1	1.000	Z
170	1.000	0.0000	1	1 000	2
1/9	0.000	3.0000	Ţ	1.000	3
	3.000	0.8100			
100	4.000	0.0000	1	1 000	2
180	1.000	1.0000	Т	1.000	3
	1.000	0.1684			
107	2.000	0.0000	1	1 000	2
197	1.000	0.4572	T	1.000	Z
200	1.000	0.0000	-	1 000	0
209	1 000	0.1524	Ţ	1.000	Z
000	1.000	0.0000	-	1 000	0
222	0.000	0.7112	T	1.000	Z
007	1.000	0.0000	1	1 000	2
221	1.000	1.0000	T	1.000	3
	1.000	0.8542			
0.05	2.000	0.0000	1	1 000	0
235	0.000	0.8128		1.000	Z
0.2.6	1.000	0.0000	7	1 000	0
236	0.000	0.1//8	1	1.000	Z
0.07	1.000	0.0000	4	1 000	2
231	0.000	1.0000	T	1.000	3
	1.000	0.2446			
0.4.0	2.000	0.0000	4	1 000	~
248	0.000	0.0/62	1	1.000	2
0.4.0	1.000	0.0000		1 000	~
249	0.000	0.0254	1	1.000	2
	T.000	0.0000			

250	0.000 1.000 2.000	1.0000 0.0414	1	1.000	3
251	0.000 2.000 3.000	2.0000 0.1844	1	1.000	3
252	0.000	0.0254	1	1.000	2
258	0.000	0.6604	1	1.000	2
259	0.000	0.4572	1	1.000	2
260	0.000	0.0254	1	1.000	2
261	0.000 2.000 3.000	2.0000 0.6416 0.0000	1	1.000	3
266	0.000 1.000	0.0254 0.0000	1	1.000	2
275	0.000 1.000	0.0254 0.0000	1	1.000	2
278	0.000 1.000	0.2286 0.0000	1	1.000	2
282	0.000 1.000 2.000	1.0000 0.4986 0.0000	1	1.000	3
283	0.000 5.000 6.000	5.0000 0.4356 0.0000	1	1.000	3
284	0.000	0.6096	1	1.000	2
285	0.000 1.000 2.000	1.0000 0.4478 0.0000	1	1.000	3
307	0.000 7.000 8.000	7.0000 0.0612 0.0000	1	1.000	3
312	0.000	0.0762	1	1.000	2
322	0.000 1.000 2.000	1.0000 0.0414 0.0000	1	1.000	3
323	0.000 1.000	0.8890 0.0000	1	1.000	2
334	0.000 1.000	0.7874 0.0000	1	1.000	2
345	0.000 1.000	0.6604 0.0000	1	1.000	2
346	0.000 1.000	0.3302 0.0000	1	1.000	2
347	0.000 1.000	0.6350 0.0000	1	1.000	2
350	0.000 3.000 4.000	3.0000 0.7846 0.0000	1	1.000	3
351	0.000	0.8382	1	1.000	2

18 of 19

NWATER (number of days of rain/irrigation) = 87

Total Water Applied = 89.5604 cm



#### UNSAT-H Version 2.03 INITIAL CONDITIONS

Input Filename: S:\Nevzat\WinUnsatH\project.inp
Results Filename: S:\Nevzat\WinUnsatH\project.res
Date of Run: 15-JUL-**
Time of Run: 9: 4:36
Title:
Muskogee LF Subtitle D Final Cover

		Initial Conditions					Initial Conditions		
NODE	DEPTH (cm)	HEAD (cm)	THETA (vol.)	TEMP (K)	NODE	DEPTH (cm)	HEAD (cm)	THETA (vol.)	TEMP (K)
1	0.000E+00	8.418E+02	0.2348	0.00	2	5.000E+00	7.756E+02	0.2403	0.00
3	9.000E+00	7.230E+02	0.2451	0.00	4	1.200E+01	6.919E+02	0.2481	0.00
5	1.500E+01	6.673E+02	0.2506	0.00	6	1.800E+01	6.480E+02	0.2527	0.00
7	2.100E+01	6.331E+02	0.2544	0.00	8	2.500E+01	6.192E+02	0.2559	0.00
9	3.000E+01	6.088E+02	0.2571	0.00	10	3.050E+01	6.083E+02	0.2572	0.00
11	3.060E+01	5.877E+02	0.0100	0.00	12	3.070E+01	5.330E+02	0.2492	0.00
13	3.500E+01	5.289E+02	0.2495	0.00	14	4.000E+01	5.258E+02	0.2497	0.00
15	4.500E+01	5.241E+02	0.2498	0.00	16	5.000E+01	5.235E+02	0.2499	0.00
17	5.500E+01	5.237E+02	0.2499	0.00	18	6.000E+01	5.244E+02	0.2498	0.00
19	6.500E+01	5.255E+02	0.2497	0.00	20	7.000E+01	5.266E+02	0.2497	0.00
21	7.500E+01	5.277E+02	0.2496	0.00	22	8.000E+01	5.286E+02	0.2495	0.00
23	8.500E+01	5.292E+02	0.2495	0.00	24	9.000E+01	5.295E+02	0.2494	0.00
25	9.150E+01	5.295E+02	0.2494	0.00					

Initial Water Storage = 22.7987 cm

NOTE: There are no temperature data when plants are modelled.

DAILY SUMMARY: Day = 1, Simulated Time = 24.0000 hr 2 Node Number 12 = 25 Depth (cm)=5.0000030.7000091.50000Water (cm3/cm3)=0.246700.249190.24944Head (cm) = 7.06097E+02 5.33012E+02 5.29480E+02Water Flow (cm) =-1.20049E-02 1.23016E-04 2.92969E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 INFIL RUNOFF DRAIN NEWSTOR PRESTOR EVAPO TRANS STORAGE 22.7987+ 0.0762+ 0.0000 - 0.0340- 0.0000- 0.0029 = 22.8379 Versus 22.8627 Mass Balance = -2.4817E-02 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.1375 cm, Actual = 0.0340 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 92.6 %; TMEAN = 266.8 K; HDRY = 1.0584E+05 cm; DAYUBC = 0

DAILY SUMMARY: Day = 2, Simulated Time = 24.0000 hr _____ 500 2 Node Number 12 25 Node Number=21225Depth (cm)=5.0000030.7000091.50000Water (cm3/cm3)=0.246390.249190.24944 Head (cm) = 7.09292E+025.33012E+025.29480E+02Water Flow (cm) =-1.60660E-02 1.23016E-04 2.92969E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 22.8627+ 0.0254+ 0.0000 - 0.0442- 0.0000- 0.0029 = 22.8410 Versus 22.8596 Mass Balance = -1.8641E-02 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.1786 cm, Actual = 0.0442 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 87.7 %; TMEAN = 262.0 K; HDRY = 1.8049E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 3, Simulated Time = 24.0000 hr _____ Node Number = 2 12 25 Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.24956 0.24919 0.24944 Head (cm) = 6.77703E+02 5.33012E+02 5.29480E+02Water Flow (cm) =  $1.58816E-02 \ 1.23016E-04 \ 2.92969E-03$ Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 22.8596+ 0.1016+ 0.0000 - 0.0646- 0.0000- 0.0029 = 22.8938 Versus 22.9082 Mass Balance = -1.4482E-02 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.2608 cm, Actual = 0.0646 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 89.7 %; TMEAN = 268.2 K; HDRY = 1.4889E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 4, Simulated Time = 24.0000 hr Node Number=21225Depth (cm)=5.0000030.7000091.50000Water (cm3/cm3)=0.279990.249090.24947 Head (cm) = 4.44443E+025.34364E+025.29164E+02Water Flow (cm) = 4.79248E-01 1.03763E-04 2.93111E-03Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 22.9082+ 0.7874+ 0.0000 - 0.0911- 0.0000- 0.0029 = 23.6016 Versus 23.6047 Mass Balance = -3.1528E-03 cm; Time step attempts =97643 and successes =97643 Evaporation: Potential = 0.3681 cm, Actual = 0.0911 cmTranspiration: Potential = 0.0000 cm, Actual = 0.0000 cmRHMEAN = 79.4 %; TMEAN = 272.6 K; HDRY = 3.1556E+05 cm; DAYUBC = 0

DAILY SUMMARY: Day = 5, Simulated Time = 24.0000 hr _____ Node Number ____ 2 12 25 Depth (cm) = 5.00000 30.70000 91.50000 = 0.30463 0.24957 0.24950Water (cm3/cm3) Head (cm) = 3.17028E+02 5.27669E+02 5.28691E+02Water Flow (cm) =  $7.55842E-01 \ 1.82369E-03 \ 2.93805E-03$ Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 23.6047+ 1.0593+ 0.0075 - 0.1326- 0.0000- 0.0029 = 24.5284 Versus 24.5286 Mass Balance = -1.7548E-04 cm; Time step attempts = 3247 and successes = 3247Evaporation: Potential = 0.5414 cm, Actual = 0.1326 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cmRHMEAN = 62.8 %; TMEAN = 277.0 K; HDRY = 6.3781E+05 cm; DAYUBC = 50 DAILY SUMMARY: Day = 6, Simulated Time = 24.0000 hr Node Number = 2 12 25 Depth (cm) = 5.00000 30.70000 Water (cm3/cm3) = 0.31329 0.25039 Depth (cm) 91.50000 0.24953 Head (cm) = 2.80502E+02 5.16642E+02 5.28278E+02Water Flow (cm) = 2.95137E-01 5.82611E-03 2.94502E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 24.5286+ 0.5080+ 0.0000 - 0.1534- 0.0000- 0.0029 = 24.8803 Versus 24.8803 Mass Balance = -1.7166E-05 cm; Time step attempts = 2096 and successes = 2096 Evaporation: Potential = 0.6197 cm, Actual = 0.1534 cmTranspiration: Potential = 0.0000 cm, Actual = 0.0000 cmRHMEAN = 64.8 %; TMEAN = 278.2 K; HDRY = 5.9459E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 7, Simulated Time = 24.0000 hr _____ Node Number = 2 12 25 Depth (cm) ----5.00000 30.70000 91.50000 SourceSource30.7000091.50000Water (cm3/cm3)=0.309150.250760.24956Head (cm) = 2.97545E+02 5.11693E+02 5.27933E+02Water Flow (cm) =-7.97446E-02 7.38057E-03 2.95103E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 24.8803+ 0.0000+ 0.0000 - 0.1085- 0.0000- 0.0030 = 24.7688 Versus 24.7688 Mass Balance = 0.0000E+00 cm; Time step attempts = 96 and successes = 96

Evaporation: Potential = 0.4341 cm, Actual = 0.1085 cmTranspiration: Potential = 0.0000 cm, Actual = 0.0000 cmRHMEAN = 77.6 %; TMEAN = 276.5 K; HDRY = 3.4736E+05 cm; DAYUBC = DAILY SUMMARY: Day = 8, Simulated Time = 24.0000 hr ______ 2 Node Number = 12 25 Depth (cm) Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.30326 0.25090 0.24957 Head (cm) = 3.23146E+02 5.09923E+02 5.27672E+02Water Flow (cm) =-1.05306E-01 6.90128E-03 2.95570E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 24.7688+ 0.0000+ 0.0000 - 0.1473- 0.0000- 0.0030 = 24.6185 Versus 24.6185 Mass Balance = -1.9073E-06 cm; Time step attempts = 96 and successes = 96 Evaporation: Potential = 0.5893 cm, Actual = 0.1473 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 69.0 %; TMEAN = 275.9 K; HDRY = 5.0887E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 9, Simulated Time = 24.0000 hr _____ Node Number = Depth (cm) = 2 12 2.5 5.00000 30.70000 91.50000 = 0.29834 0.25089 0.24959Water (cm3/cm3) Head (cm) = 3.45930E+02 5.09960E+02 5.27472E+02Water Flow (cm) =-1.04691E-01 6.10810E-03 2.95924E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 24.6185+ 0.0000+ 0.0000 - 0.1392- 0.0000- 0.0030 = 24.4763 Versus 24.4763 Mass Balance = 1.9073E-06 cm; Time step attempts = 96 and successes = 96 Evaporation: Potential = 0.5570 cm, Actual = 0.1392 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cmRHMEAN = 77.5 %; TMEAN = 274.0 K; HDRY = 3.4943E+05 cm; DAYUBC = DAILY SUMMARY: Day = 10, Simulated Time = 24.0000 hr _____ Node Number=21225Depth (cm)=5.0000030.7000091.50000Water (cm3/cm3)=0.304590.250960.24960 = 3.17217E+02 5.09132E+02 5.27307E+02 Head (cm) Water Flow (cm) = 8.70202E-02 5.65331E-03 2.96196E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00

PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 24.4763+ 0.2032+ 0.0000 - 0.0707- 0.0000- 0.0030 = 24.6059 Versus 24.6058 Mass Balance = 7.0572E-05 cm; Time step attempts = 2096 and successes = 2096 Evaporation: Potential = 0.2856 cm, Actual = 0.0707 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 78.1 %; TMEAN = 275.7 K; HDRY = 3.3921E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 11, Simulated Time = 24.0000 hr ____ Node Number = 2 12 25 Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.31849 0.25139 0.24961 = 2.60128E+02 5.03473E+02 5.27134E+02Head (cm) Water Flow (cm) =  $3.08355E-01 \ 6.77402E-03 \ 2.96455E-03$ Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 24.6058+ 0.4064+ 0.0000 - 0.0000- 0.0000- 0.0030 = 25.0092 Versus 25.0092 Mass Balance = 6.2943E-05 cm; Time step attempts = 2096 and successes = 2096 Evaporation: Potential = 0.0000 cm, Actual = 0.0000 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 94.9 %; TMEAN = 275.7 K; HDRY = 7.2186E+04 cm; DAYUBC = 0 _____ DAILY SUMMARY: Day = 12, Simulated Time = 24.0000 hr Node Number Depth (cm) = 2 12 25 Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.31853 0.25176 0.24963 Head (cm) = 2.59984E+02 4.98690E+02 5.26931E+02Water Flow (cm) = 2.53315E-02 8.37801E-03 2.96745E-03Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 25.0092+ 0.0254+ 0.0000 - 0.0000- 0.0000- 0.0030 = 25.0316 Versus 25.0315 Mass Balance = 8.3923E-05 cm; Time step attempts = 2096 and successes = 2096 Evaporation: Potential = 0.0000 cm, Actual = 0.0000 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = **** %; TMEAN = 274.8 K; HDRY = -6.1934E+04 cm; DAYUBC = 0 DAILY SUMMARY: Day = 13, Simulated Time = 24.0000 hr _______ Node Number 25 2 12 -= 5.00000 30.70000 91.50000 Depth (cm)

Water (cm3/cm3) = 0.31971 0.25207 0.24965Head (cm) = 2.55519E+02 4.94787E+02 5.26688E+02Water Flow (cm) = 3.89231E-02 8.81049E-03 2.97089E-03Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 25.0315+ 0.0508+ 0.0000 - 0.0037- 0.0000- 0.0030 = 25.0757 Versus 25.0756 Mass Balance = 4.1962E-05 cm; Time step attempts = 2096 and successes = 2096 Evaporation: Potential = 0.0149 cm, Actual = 0.0037 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cmRHMEAN = 89.9 %; TMEAN = 279.0 K; HDRY = 1.4591E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 14, Simulated Time = 24.0000 hr _______ Node Number = 2 12 2.5 Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.31079 0.25223 0.24967 Head (cm) =  $2.90703E+02 \ 4.92719E+02 \ 5.26385E+02$ Water Flow (cm) =-1.24680E-01 8.90725E-03 2.97525E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 25.0756+ 0.0000+ 0.0000 - 0.1895- 0.0000- 0.0030 = 24.8831 Versus 24.8831 Mass Balance = -5.7220E-06 cm; Time step attempts = 96 and successes = 96 Evaporation: Potential = 0.7582 cm, Actual = 0.1895 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cmRHMEAN = 75.1 %; TMEAN = 277.0 K; HDRY = 3.9301E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 15, Simulated Time = 24.0000 hr 2 = Node Number 12 25 Depth (cm) == 5.00000 30.70000 Water (cm3/cm3) = 0.30754 0.25218 91.50000 0.24970 Head (cm) = 3.04358E+02 4.93290E+02 5.26023E+02Water Flow (cm) =-9.39608E-02 7.85602E-03 2.98045E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR PRESTOR STORAGE 24.8831+ 0.0000+ 0.0000 - 0.1157- 0.0000- 0.0030 = 24.7644 Versus 24.7644 Mass Balance = 1.9073E-06 cm; Time step attempts = 96 and successes = 96 Evaporation: Potential = 0.4628 cm, Actual = 0.1157 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cmRHMEAN = 69.3 %; TMEAN = 275.9 K; HDRY = 5.0351E+05 cm; DAYUBC = 0 

DAILY SUMMARY: Day = 16, Simulated Time = 24.0000 hr

_____ Node Number = 2 12 25 Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.30910 0.25212 0.24973 Head (cm) = 2.97724E+02 4.94065E+02 5.25591E+02Water Flow (cm) =-1.27043E-02 6.94122E-03 2.98666E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 24.7644+ 0.0000+ 0.0000 - 0.0000- 0.0000- 0.0030 = 24.7614 Versus 24.7614 Mass Balance = -1.9073E-06 cm; Time step attempts = 96 and successes = 96 Evaporation: Potential = 0.0000 cm, Actual = 0.0000 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 89.1 %; TMEAN = 273.2 K; HDRY = 1.5860E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 17, Simulated Time = 24.0000 hr _____ Node Number Depth (cm) == 2 12 25 Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.31189 0.25220 0.24976 Head (cm) = 2.86169E+02 4.93075E+02 5.25105E+02Water Flow (cm) = 7.01197E-02 6.84626E-03 2.99371E-03Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 24.7614+ 0.1016+ 0.0000 - 0.0121- 0.0000- 0.0030 = 24.8479 Versus 24.8479 Mass Balance = 3.8147E-06 cm; Time step attempts = 2096 and successes = 2096 Evaporation: Potential = 0.0489 cm, Actual = 0.0121 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 88.7 %; TMEAN = 273.2 K; HDRY = 1.6392E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 18, Simulated Time = 24.0000 hr _____ Node Number = 2 12 Depth (cm) = 5.00000 30.70000 25 91.50000 Water (cm3/cm3) = 0.30536 0.25226 0.24980Head (cm) = 3.13827E+02 4.92335E+02 5.24542E+02Water Flow (cm) =-7.48594E-02 7.08562E-03 3.00200E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 24.8479+ 0.0254+ 0.0000 - 0.1482- 0.0000- 0.0030 = 24.7222 Versus 24.7221 Mass Balance = 9.5367E-05 cm; Time step attempts = 2096 and successes = 2096 Evaporation: Potential = 0.5987 cm, Actual = 0.1482 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 66.6 %; TMEAN = 275.9 K; HDRY = 5.5728E+05 cm; DAYUBC = 0 

DAILY SUMMARY: Day = 19, Simulated Time = 24.0000 hr = Node Number 2 12 25 Depth (cm) = 5.00000 30.70000 91.50000 = 0.29681 0.25216 0.24985 Water (cm3/cm3) Head (cm) = 3.53302E+02 4.93535E+02 5.23887E+02Water Flow (cm) =-1.42656E-01 6.46334E-03 3.01184E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 24.7221+ 0.0000+ 0.0000 - 0.2042- 0.0000- 0.0030 = 24.5148 Versus 24.5148 Mass Balance = -1.9073E-06 cm; Time step attempts = 96 and successes = 96 Evaporation: Potential = 0.8170 cm, Actual = 0.2042 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 66.9 %; TMEAN = 270.4 K; HDRY = 5.5057E+05 cm; DAYUBC = DAILY SUMMARY: Day = 20, Simulated Time = 24.0000 hr ______ Node Number = 12 25 2 Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.29385 0.25197 0.24990 = 3.67961E+02 4.95960E+02 5.23175E+02 Head (cm) Water Flow (cm) =-1.03963E-01 5.41850E-03 3.02273E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 24.5148+ 0.0000+ 0.0000 - 0.1231- 0.0000- 0.0030 = 24.3886 Versus 24.3886 Mass Balance = -1.9073E-06 cm; Time step attempts = 96 and successes = 96 Evaporation: Potential = 0.4926 cm, Actual = 0.1231 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cmRHMEAN = 64.3 %; TMEAN = 268.7 K; HDRY = 6.0445E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 21, Simulated Time = 24.0000 hr Node Number ----2 12 25 Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.28909 0.25178 0.24996Head (cm) =  $3.92666E+02 \ 4.98468E+02 \ 5.22420E+02$ Water Flow (cm) =- $9.72019E-02 \ 4.57526E-03 \ 3.03448E-03$ Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 24.3886+ 0.0000+ 0.0000 - 0.1310- 0.0000- 0.0030 = 24.2547 Versus 24.2546 Mass Balance = 1.9073E-06 cm; Time step attempts = 96 and successes = 96 Evaporation: Potential = 0.5238 cm, Actual = 0.1310 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm

RHMEAN = 61.7 %; TMEAN = 275.7 K; HDRY = 6.6173E+05 cm; DAYUBC =

DAILY SUMMARY: Day = 22, Simulated Time = 24.0000 hr _____ Node Number = 2 12 25 Depth (cm) Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.28167 0.25157 0.25002 Head (cm) = 4.34394E+025.01163E+025.21636E+02Water Flow (cm) =-1.19522E-01 3.88918E-03 3.04688E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 24.2546+ 0.0000+ 0.0000 - 0.1736- 0.0000- 0.0030 = 24.0780 Versus 24.0780 Mass Balance = -3.8147E-06 cm; Time step attempts = 96 and successes = 96 Evaporation: Potential = 0.6946 cm, Actual = 0.1736 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 65.2 %; TMEAN = 276.5 K; HDRY = 5.8689E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 23, Simulated Time = 24.0000 hr Node Number ----2 12 25 Depth (cm) Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.28045 0.25134 0.25008 Head (cm) = 4.41663E+02 5.04169E+02 5.20836E+02Water Flow (cm) =-8.46008E-02 3.20479E-03 3.05971E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 24.0780+ 0.0000+ 0.0000 - 0.0905- 0.0000- 0.0031 = 23.9844 Versus 23.9844 Mass Balance = 0.0000E+00 cm; Time step attempts = 96 and successes = 96 Evaporation: Potential = 0.3621 cm, Actual = 0.0905 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 73.3 %; TMEAN = 279.8 K; HDRY = 4.2514E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 24, Simulated Time = 24.0000 hr ____ Node Number = 2 12 25 Depth (cm) Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.25952 0.25111 0.25014 = 5.88808E+02 5.07098E+02 5.20040E+02Head (cm) Water Flow (cm) =-1.78051E-01 2.64242E-03 3.07270E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 23.9844+ 0.0000+ 0.0000 - 0.3505- 0.0000- 0.0031 = 23.6308 Versus 23.6308

Mass Balance = -5.7220E-06 cm; Time step attempts = 360 and successes = 360 Evaporation: Potential = 1.4021 cm, Actual = 0.3505 cmTranspiration: Potential = 0.0000 cm, Actual = 0.0000 cmRHMEAN = 63.6 %; TMEAN = 278.2 K; HDRY = 6.2118E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 25, Simulated Time = 24.0000 hr Node Number = 2 12 25 Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.24703 0.25083 0.25019 Head (cm) = 7.02785E+02 5.10767E+02 5.19269E+02Water Flow (cm) =-1.99918E-01 1.98307E-03 3.08553E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 23.6308+ 0.0000+ 0.0000 - 0.3079- 0.0000- 0.0031 = 23.3199 Versus 23.3199 Mass Balance = 0.0000E+00 cm; Time step attempts = 500 and successes = 500 Evaporation: Potential = 1.2314 cm, Actual = 0.3079 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cmRHMEAN = 60.3 %; TMEAN = 274.8 K; HDRY = 6.9394E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 26, Simulated Time = 24.0000 hr ______ = 12 Node Number 2 25 Depth (cm) = Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.23715 0.25053 0.25025 Head (cm) = 8.12324E+02 5.14760E+02 5.18556E+02Water Flow (cm) =-1.86160E-01 1.23439E-03 3.09778E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 23.3199+ 0.0000+ 0.0000 - 0.3789- 0.0000- 0.0031 = 22.9378 Versus 22.9378 Mass Balance = 4.1962E-05 cm; Time step attempts = 2247 and successes = 2247 Evaporation: Potential = 1.5158 cm, Actual = 0.3789 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cmRHMEAN = 68.6 %; TMEAN = 279.0 K; HDRY = 5.1683E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 27, Simulated Time = 24.0000 hr _____ 2 Node Number = 12 25 Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.28122 0.25035 0.25030 Head (cm) = 4.37047E+025.17119E+025.17912E+02Water Flow (cm) = 4.07863E-01 8.26102E-04 3.10867E-03

Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00

PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 22.9378+ 1.0160+ 0.0000 - 0.1279- 0.0000- 0.0031 = 23.8227 Versus 23.8228 Mass Balance = -6.6757E-05 cm; Time step attempts = 5110 and successes = 5110 Evaporation: Potential = 0.5222 cm, Actual = 0.1279 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 76.1 %; TMEAN = 272.9 K; HDRY = 3.7449E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 28, Simulated Time = 24.0000 hr ______ Node Number = 2 12 25 Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.37969 0.25752 0.25034 Head (cm) = 7.27959E+01 4.30136E+02 5.17324E+02Water Flow (cm) =  $2.48750E+00 \ 2.67461E-02 \ 3.11898E-03$ Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 23.8228+ 3.1760+ 2.0818 - 0.0000- 0.0000- 0.0031 = 26.9957 Versus 26.9959 Mass Balance = -2.2507E-04 cm; Time step attempts = 2393 and successes = 2393 Evaporation: Potential = 0.0000 cm, Actual = 0.0000 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 92.8 %; TMEAN = 277.3 K; HDRY = 1.0183E+05 cm; DAYUBC = 394 DAILY SUMMARY: Day = 29, Simulated Time = 24.0000 hr ______ Node Number Depth (cm) 12 = 2 25 Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.39278 0.26745 0.25038 = 3.40958E+01 3.33088E+02 5.16804E+02Head (cm) Water Flow (cm) = 3.99890E-01 9.71246E-02 3.12824E-03Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 26.9959+ 0.5225+ 1.0015 - 0.0300- 0.0000- 0.0031 = 27.4852 Versus 27.4847 Mass Balance = 4.8828E-04 cm; Time step attempts = 2096 and successes = 2096 Evaporation: Potential = 0.1226 cm, Actual = 0.0300 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 84.6 %; TMEAN = 277.9 K; HDRY = 2.2939E+05 cm; DAYUBC = 1640 DAILY SUMMARY: Day = 30, Simulated Time = 24.0000 hr ______ Node Number=21225Depth (cm)=5.0000030.7000091.50000Water (cm3/cm3)=0.376730.266960.25041

Head (cm) = 8.09186E+01 3.37379E+02 5.16321E+02Water Flow (cm) =-2.67512E-01 7.41075E-02 3.13673E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 27.4847+ 0.0000+ 0.0000 - 0.3811- 0.0000- 0.0031 = 27.1005 Versus 27.1005 Mass Balance = -3.8147E-06 cm; Time step attempts = 96 and successes = 96 Evaporation: Potential = 1.5245 cm, Actual = 0.3811 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 58.8 %; TMEAN = 280.7 K; HDRY = 7.2878E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 31, Simulated Time = 24.0000 hr _____ Node Number = Depth (cm) = 2 12 25 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.36388 0.26581 0.25046Head (cm) = 1.16034E+02 3.47625E+02 5.15695E+02 Water Flow (cm) =-2.52388E-01 4.65319E-02 3.14550E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 27.1005+ 0.0000+ 0.0000 - 0.3426- 0.0000- 0.0031 = 26.7547 Versus 26.7547 Mass Balance = 0.0000E+00 cm; Time step attempts = 96 and successes = 96 Evaporation: Potential = 1.3704 cm, Actual = 0.3426 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 52.9 %; TMEAN = 276.8 K; HDRY = 8.7361E+05 cm; DAYUBC = DAILY SUMMARY: Day = 32, Simulated Time = 24.0000 hr _____ Node Number -2 12 25 Note Number=21225Depth (cm)=5.0000030.7000091.50000Water (cm3/cm3)=0.352070.264660.25056 Head (cm) = 1.49373E+02 3.58071E+02 5.14386E+02Water Flow (cm) =-2.42942E-01 3.28752E-02 3.16089E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 26.7547+ 0.0000+ 0.0000 - 0.3257- 0.0000- 0.0032 = 26.4258 Versus 26.4258 Mass Balance = 1.9073E-06 cm; Time step attempts = 96 and successes = 96 Evaporation: Potential = 1.3029 cm, Actual = 0.3257 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cmRHMEAN = 59.6 %; TMEAN = 276.8 K; HDRY = 7.0964E+05 cm; DAYUBC = 0 

DAILY SUMMARY: Day = 33, Simulated Time = 24.0000 hr

Node Number=21225Depth (cm)=5.0000030.7000091.50000Water (cm3/cm3)=0.318260.263190.25075 Head (cm) = 2.61025E+02 3.71936E+02 5.11833E+02Water Flow (cm) =-5.89605E-01 2.29952E-02 3.19320E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 INFIL RUNOFF PRESTOR EVAPO TRANS DRAIN NEWSTOR STORAGE 26.4258+ 0.0000+ 0.0000 - 0.8340- 0.0000- 0.0032 = 25.5886 Versus 25.5886 Mass Balance = -3.4332E-05 cm; Time step attempts = 224 and successes = 224 Evaporation: Potential = 3.3361 cm, Actual = 0.8340 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 58.2 %; TMEAN = 272.3 K; HDRY = 7.4279E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 34, Simulated Time = 24.0000 hr _______ Node Number ----2 12 25 Depth (cm) Depth (cm) = 5.00000 30.70000 Water (cm3/cm3) = 0.30781 0.26159 91.50000 0.25103 Head (cm) = 3.03218E+02 3.87535E+02 5.08180E+02Water Flow (cm) =-3.64908E-01 1.25792E-02 3.24774E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 EVAPO TRANS PRESTOR INFIL RUNOFF DRAIN NEWSTOR STORAGE 25.5886+ 0.0000+ 0.0000 - 0.9782- 0.0000- 0.0032 = 24.6071 Versus 24.6067 Mass Balance = 4.6539E-04 cm; Time step attempts =17052 and successes =17052 Evaporation: Potential = 4.6481 cm, Actual = 0.9782 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 61.0 %; TMEAN = 279.5 K; HDRY = 6.7727E+05 cm; DAYUBC = 2943 DAILY SUMMARY: Day = 35, Simulated Time = 24.0000 hr _____ Node Number = 2 12 25 Depth (cm) = Depth (cm) = 5.00000 30.70000 Water (cm3/cm3) = 0.30854 0.26045 91.50000 0.25141 Head (cm) = 3.00098E+02 3.99019E+02 5.03192E+02Water Flow (cm) =-5.48281E-02 8.17758E-03 3.32366E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 24.6067+ 0.0000+ 0.0000 - 0.0524- 0.0000- 0.0033 = 24.5509 Versus 24.5508 Mass Balance = 1.5068E-04 cm; Time step attempts = 5094 and successes = 5094 Evaporation: Potential = 6.2724 cm, Actual = 0.0524 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 57.2 %; TMEAN = 280.7 K; HDRY = 7.6572E+05 cm; DAYUBC = 4410
DAILY SUMMARY: Day = 36, Simulated Time = 24.0000 hr Node Number -----2 12 - 25 Depth (cm) = 5.00000 30.70000 91.50000 = 0.30667 0.25968 0.25185 Water (cm3/cm3) = 3.08107E+02 4.06990E+02 4.97533E+02 Head (cm) Water Flow (cm) =-4.19422E-02 6.90713E-03 3.42239E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 24.5508+ 0.0000+ 0.0000 - 0.0504- 0.0000- 0.0034 = 24.4969 Versus 24.4972 Mass Balance = -2.4414E-04 cm; Time step attempts = 5094 and successes = 5094 Evaporation: Potential = 5.1168 cm, Actual = 0.0504 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 57.1 %; TMEAN = 279.5 K; HDRY = 7.6758E+05 cm; DAYUBC = 5081 DAILY SUMMARY: Day = 37, Simulated Time = 24.0000 hr ______ Node Number ----2 12 25 Depth (cm) = 5.00000 30.70000 Water (cm3/cm3) = 0.30466 0.25908 91.50000 0.25231 = 3.16885E+02 4.13333E+02 4.91703E+02 Head (cm) Water Flow (cm) =-4.21401E-02 6.22772E-03 3.53342E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 24.4972+ 0.0000+ 0.0000 - 0.0506- 0.0000- 0.0035 = 24.4431 Versus 24.4433 Mass Balance = -2.7275E-04 cm; Time step attempts = 3760 and successes = 3760 Evaporation: Potential = 4.4585 cm, Actual = 0.0506 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 59.9 %; TMEAN = 282.6 K; HDRY = 7.0145E+05 cm; DAYUBC = 3747 _____ _____ DAILY SUMMARY: Day = 38, Simulated Time = 24.0000 hr _____ Node Number 2011 2 12 2.5 Depth (cm) ..... 5.00000 30.70000 91.50000 = 0.29903 0.25856 0.25276 Water (cm3/cm3) Head (cm) =  $3.42658E+02 \ 4.18853E+02 \ 4.86072E+02$ Water Flow (cm) =- $8.02628E-02 \ 5.64724E-03 \ 3.64841E-03$ Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 24.4433+ 0.0000+ 0.0000 - 0.0881- 0.0000- 0.0036 = 24.3516 Versus 24.3516 Mass Balance = 1.3351E-05 cm; Time step attempts = 3157 and successes = 3157 Evaporation: Potential = 2.7264 cm, Actual = 0.0881 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm

RHMEAN = 89.7 %; TMEAN = 284.0 K; HDRY = 1.4970E+05 cm; DAYUBC = 3115

DAILY SUMMARY: Day = 39, Simulated Time = 24.0000 hr _____ Node Number = Depth (cm) = Node Number === 2 12 25 Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.29725 0.25810 0.25314 Head (cm) = 3.51142E+02 4.23785E+02 4.81267E+02Water Flow (cm) =-6.58003E-02 4.95932E-03 3.75272E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 24.3516+ 0.0000+ 0.0000 - 0.0832- 0.0000- 0.0038 = 24.2646 Versus 24.2649 Mass Balance = -3.0136E-04 cm; Time step attempts =20088 and successes =20088 Evaporation: Potential = 4.0214 cm, Actual = 0.0832 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 80.4 %; TMEAN = 292.6 K; HDRY = 2.9946E+05 cm; DAYUBC = 4891 DAILY SUMMARY: Day = 40, Simulated Time = 24.0000 hr _____ ..... 2 12 Node Number 25 Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.29693 0.25771 0.25348 Head (cm) = 3.52729E+02 4.28045E+02 4.77081E+02Water Flow (cm) =-3.70564E-02 4.38210E-03 3.84815E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 24.2649+ 0.0000+ 0.0000 - 0.0495- 0.0000- 0.0038 = 24.2115 Versus 24.2142 Mass Balance = -2.6627E-03 cm; Time step attempts =27585 and successes =27585 Evaporation: Potential = 2.7644 cm, Actual = 0.0495 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cmRHMEAN = 46.8 %; TMEAN = 281.5 K; HDRY = 1.0421E+06 cm; DAYUBC = 4388 DAILY SUMMARY: Day = 41, Simulated Time = 24.0000 hr _____ = Node Number 2 12 25 Depth (cm) = Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.29469 0.25734 0.25383 Head (cm) = 3.63732E+02 4.32190E+02 4.72900E+02Water Flow (cm) =-3.95282E-02 3.99201E-03 3.94648E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 24.2142+ 0.0000+ 0.0000 - 0.0446- 0.0000- 0.0039 = 24.1656 Versus 24.1656

Mass Balance = 3.8147E-06 cm; Time step attempts = 176 and successes = 176 Evaporation: Potential = 2.5036 cm, Actual = 0.0446 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 67.7 %; TMEAN = 271.5 K; HDRY = 5.3416E+05 cm; DAYUBC = 139 DAILY SUMMARY: Day = 42, Simulated Time = 24.0000 hr _____ Node Number Depth (cm) = 2 12 25 Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.29273 0.25700 0.25412 = 3.73599E+02 4.35921E+02 4.69348E+02 Head (cm) Water Flow (cm) =-4.19912E-02 3.65173E-03 4.03389E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 24.1656+ 0.0000+ 0.0000 - 0.0502- 0.0000- 0.0040 = 24.1114 Versus 24.1118 Mass Balance = -4.0436E-04 cm; Time step attempts = 3751 and successes = 3751Evaporation: Potential = 3.5437 cm, Actual = 0.0502 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 69.7 %; TMEAN = 275.4 K; HDRY = 4.9511E+05 cm; DAYUBC = 3734 DAILY SUMMARY: Day = 43, Simulated Time = 24.0000 hr ______ Node Number = 2 12 25 Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.26404 0.25669 0.25434 = 5.52987E+02 4.39459E+02 4.66650E+02Head (cm) Water Flow (cm) =-2.61586E-01 3.24699E-03 4.10431E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 24.1118+ 0.0254+ 0.0000 - 0.0000- 0.0000- 0.0041 = 24.1331 Versus 24.1336 Mass Balance = -4.4250E-04 cm; Time step attempts =25162 and successes =25162 Evaporation: Potential = 0.0000 cm, Actual = 0.0000 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 95.7 %; TMEAN = 277.9 K; HDRY = 6.0702E+04 cm; DAYUBC = 0 DAILY SUMMARY: Day = 44, Simulated Time = 24.0000 hr _____ Node Number Depth (cm) 2 = 12 25 Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.25983 0.25624 0.25455 Head (cm) = 5.86279E+02 4.44503E+02 4.64152E+02Water Flow (cm) =-2.11682E-01 2.19242E-03 4.17024E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00

PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 24.1336+ 0.0000+ 0.0000 - 0.5857- 0.0000- 0.0042 = 23.5437 Versus 23.5438 Mass Balance = -1.3351E-04 cm; Time step attempts =12558 and successes =12558 Evaporation: Potential = 2.6223 cm, Actual = 0.5857 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 98.2 %; TMEAN = 282.3 K; HDRY = 2.4333E+04 cm; DAYUBC = 3141 DAILY SUMMARY: Day = 45, Simulated Time = 24.0000 hr ______ Node Number = 2 12 25 Depth (cm) Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.24341 0.25579 0.25475 Head (cm) = 7.40656E+02 4.49683E+02 4.61857E+02Water Flow (cm) =-2.04487E-01 1.15841E-03 4.22821E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 23.5438+ 0.0000+ 0.0000 - 0.0000- 0.0000- 0.0042 = 23.5396 Versus 23.5395 Mass Balance = 8.7738E-05 cm; Time step attempts = 96 and successes = 96 Evaporation: Potential = 2.8630 cm, Actual = 0.0000 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = **** %; TMEAN = 282.0 K; HDRY = -1.2593E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 46, Simulated Time = 24.0000 hr = Node Number 2 12 Node Number = Depth (cm) = 25 Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.25315 0.25536 0.25491 Head (cm) = 6.43950E+02 4.54657E+02 4.59928E+02Water Flow (cm) =-1.13349E-01 4.02821E-04 4.28052E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 23.5395+ 0.0000+ 0.0000 - 0.0000- 0.0000- 0.0043 = 23.5352 Versus 23.5352 Mass Balance = 0.0000E+00 cm; Time step attempts = 128 and successes = 128 Evaporation: Potential = 0.0000 cm, Actual = 0.0000 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 94.0 %; TMEAN = 272.0 K; HDRY = 8.5107E+04 cm; DAYUBC = 0 DAILY SUMMARY: Day = 47, Simulated Time = 24.0000 hr ______ = 2 12 Node Number 25 Node Number-2-Depth (cm)=5.0000030.7000091.50000Water (cm3/cm3)=0.253640.255010.25503Head (cm) = 6.39489E+02 4.58749E+02 4.58475E+02

Water Flow (cm) = 5.48880E - 02 - 7.82888E - 054.32290E - 03Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 23.5352+ 0.3810+ 0.0000 - 0.6538- 0.0000- 0.0043 = 23.2581 Versus 23.2582 Mass Balance = -1.1444E-04 cm; Time step attempts = 9211 and successes = 9211 Evaporation: Potential = 2.6417 cm, Actual = 0.6538 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 84.5 %; TMEAN = 271.8 K; HDRY = 2.3075E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 48, Simulated Time = 24.0000 hr _____ Node Number=21225Depth (cm)=5.0000030.7000091.50000Water (cm3/cm3)=0.249590.254730.25513 Head (cm) = 6.77447E+02 4.62035E+02 4.57363E+02Water Flow (cm) =-1.33441E-01-1.67814E-04 4.35455E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 23.2582+ 0.1270+ 0.0000 - 0.3600- 0.0000- 0.0044 = 23.0208 Versus 23.0212 Mass Balance = -3.6430E-04 cm; Time step attempts =22001 and successes =22001 Evaporation: Potential = 3.0741 cm, Actual = 0.3600 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 61.6 %; TMEAN = 271.2 K; HDRY = 6.6480E+05 cm; DAYUBC = 3123 DAILY SUMMARY: Day = 49, Simulated Time = 24.0000 hr _____ Node Number Depth (cm) = 2 12 25 Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.25162 0.25448 0.25519 Head (cm) = 6.58117E+02 4.65031E+02 4.56610E+02Water Flow (cm) =-4.97482E-02-4.37798E-04 4.37619E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 23.0212+ 0.0508+ 0.0000 - 0.0883- 0.0000- 0.0044 = 22.9793 Versus 22.9812 Mass Balance = -1.8959E-03 cm; Time step attempts =42279 and successes =42279 Evaporation: Potential = 3.3203 cm, Actual = 0.0883 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 71.5 %; TMEAN = 274.8 K; HDRY = 4.5964E+05 cm; DAYUBC = 2899 

DAILY SUMMARY: Day = 50, Simulated Time = 24.0000 hr

Node Number=21225Depth (cm)=5.0000030.7000091.50000Water (cm3/cm3)=0.252330.254200.25526 Head (cm)  $= 6.51506E+02 \ 4.68333E+02 \ 4.55768E+02$ Water Flow (cm) =-2.76340E-02-6.76378E-04 4.39950E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 22.9812+ 0.0000+ 0.0000 - 0.0255- 0.0000- 0.0044 = 22.9513 Versus 22.9515 Mass Balance = -2.2507E-04 cm; Time step attempts = 5094 and successes = 5094Evaporation: Potential = 5.1699 cm, Actual = 0.0255 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 68.1 %; TMEAN = 282.9 K; HDRY = 5.2566E+05 cm; DAYUBC = 4073 DAILY SUMMARY: Day = 51, Simulated Time = 24.0000 hr _____ Node Number = 2 12 2.5 Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.24990 0.25395 0.25532 Depth (cm) Head (cm) = 6.74426E+02 4.71374E+02 4.55147E+02Water Flow (cm) =-3.17803E-02-8.07890E-04 4.41896E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 22.9515+ 0.0000+ 0.0000 - 0.0243- 0.0000- 0.0044 = 22.9228 Versus 22.9228 Mass Balance = -1.7166E-05 cm; Time step attempts = 841 and successes = 841 Evaporation: Potential = 2.5091 cm, Actual = 0.0243 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 91.4 %; TMEAN = 281.5 K; HDRY = 1.2308E+05 cm; DAYUBC = 777 DAILY SUMMARY: Day = 52, Simulated Time = 24.0000 hr _____ 2 Node Number -----12 25 Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.24953 0.25378 0.25534 Head (cm) = 6.77967E+02 4.73451E+02 4.54860E+02Water Flow (cm) =-3.57919E-02-8.88740E-04 4.42914E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 22.9228+ 0.0254+ 0.0000 - 0.0792- 0.0000- 0.0044 = 22.8646 Versus 22.8719 Mass Balance = -7.3166E-03 cm; Time step attempts =53090 and successes =53090 Evaporation: Potential = 3.7411 cm, Actual = 0.0792 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 73.1 %; TMEAN = 275.9 K; HDRY = 4.2865E+05 cm; DAYUBC = 4960 

DAILY SUMMARY: Day = 53, Simulated Time = 24.0000 hr -----Node Number = Depth (cm) = 2 12 25 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.24946 0.25357 0.25536Head (cm) =  $6.78648E+02 \ 4.76036E+02 \ 4.54612E+02$ Water Flow (cm) = $-2.27236E-02-9.72135E-04 \ 4.43741E-03$ Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 22.8719+ 0.0000+ 0.0000 - 0.0251- 0.0000- 0.0044 = 22.8424 Versus 22.8431 Mass Balance = -7.4005E-04 cm; Time step attempts = 7593 and successes = 7593 Evaporation: Potential = 3.2177 cm, Actual = 0.0251 cmTranspiration: Potential = 0.0000 cm, Actual = 0.0000 cmRHMEAN = 67.3 %; TMEAN = 275.7 K; HDRY = 5.4240E+05 cm; DAYUBC = 4535 _____ DAILY SUMMARY: Day = 54, Simulated Time = 24.0000 hr ______ Node Number = 2 12 25 Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.34248 0.25645 0.25537 Head (cm) = 1.78095E+02 4.42081E+02 4.54536E+02 Water Flow (cm) = 2.26214E+00 9.98596E-03 4.44169E-03Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 22.8431+ 3.8444+ 1.5404 - 0.5326- 0.0000- 0.0044 = 26.1504 Versus 26.1512 Mass Balance = -7.6103E-04 cm; Time step attempts = 5431 and successes = 5431Evaporation: Potential = 2.2663 cm, Actual = 0.5326 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 96.0 %; TMEAN = 276.5 K; HDRY = 5.6282E+04 cm; DAYUBC = 519 DAILY SUMMARY: Day = 55, Simulated Time = 24.0000 hr Node Number = 2 12 25 Depth (cm) = 5.00000 30.70000 91.50000  $Water (cm3/cm3) = 0.31139 \quad 0.25692 \quad 0.25536$ Head (cm) = 2.88229E+024.36869E+024.54610E+02Water Flow (cm) =-5.28860E-011.60296E-024.44190E-03Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 26.1512+ 0.0000+ 0.0000 - 0.7538- 0.0000- 0.0044 = 25.3929 Versus 25.3930 Mass Balance = -3.0518E-05 cm; Time step attempts = 224 and successes = 224 Evaporation: Potential = 3.0153 cm, Actual = 0.7538 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm

RHMEAN = 77.6 %; TMEAN = 285.1 K; HDRY = 3.4778E+05 cm; DAYUBC = 0

DAILY SUMMARY: Day = 56, Simulated Time = 24.0000 hr _____ Node Number = 2 12 25 Depth (cm) Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.28376 0.25651 0.25535 Head (cm) = 4.22207E+02 4.41470E+02 4.54809E+02Water Flow (cm) =-4.43940E-01 9.68216E-03 4.43815E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 25.3930+ 0.0000+ 0.0000 - 0.6519- 0.0000- 0.0044 = 24.7367 Versus 24.7367 Mass Balance = -1.3351E-05 cm; Time step attempts = 442 and successes = 442 Evaporation: Potential = 2.6075 cm, Actual = 0.6519 cmTranspiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 60.8 %; TMEAN = 279.8 K; HDRY = 6.8096E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 57, Simulated Time = 24.0000 hr _____ Node Number=21225Depth (cm)=5.0000030.7000091.50000Water (cm3/cm3)=0.272060.255980.25533 Head (cm) = 4.95211E+02 4.47476E+02 4.54951E+02Water Flow (cm) =-3.28862E-01 5.63063E-03 4.43291E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 24.7367+ 0.0000+ 0.0000 - 0.7344- 0.0000- 0.0044 = 23.9979 Versus 23.9978 Mass Balance = 1.2207E-04 cm; Time step attempts = 9539 and successes = 9539 Evaporation: Potential = 2.9374 cm, Actual = 0.7344 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 61.5 %; TMEAN = 283.4 K; HDRY = 6.6580E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 58, Simulated Time = 24.0000 hr _____ ----Node Number Depth (cm) 2 12 2.5 Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.27247 0.25552 0.25532 Head (cm) = 4.92421E+02 4.52829E+02 4.55067E+02Water Flow (cm) =-2.15588E-01 3.23070E-03 4.43043E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 23.9978+ 0.0000+ 0.0000 - 0.3065- 0.0000- 0.0044 = 23.6868 Versus 23.6869

Mass Balance = -7.8201E-05 cm; Time step attempts =19848 and successes =19848 Evaporation: Potential = 2.5217 cm, Actual = 0.3065 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 90.6 %; TMEAN = 279.0 K; HDRY = 1.3578E+05 cm; DAYUBC = 3823 DAILY SUMMARY: Day = 59, Simulated Time = 24.0000 hr _____ Node Number = 2 12 25 Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.27001 0.25512 0.25532 Head (cm) = 5.09358E+02 4.57438E+02 4.55132E+02Water Flow (cm) =-9.59645E-02 1.86887E-03 4.42755E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 23.6869+ 0.0000+ 0.0000 - 0.0919- 0.0000- 0.0044 = 23.5906 Versus 23.5906 Mass Balance = 1.7166E-05 cm; Time step attempts = 3093 and successes = 3093 Evaporation: Potential = 1.8100 cm, Actual = 0.0919 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 95.4 %; TMEAN = 271.5 K; HDRY = 6.4075E+04 cm; DAYUBC = 623 DAILY SUMMARY: Day = 60, Simulated Time = 24.0000 hr Node Number = 2 12 2.5 Note A(m, b)=21223Depth (cm)=5.0000030.7000091.50000Water (cm3/cm3)=0.272480.254810.25532Head (cm) = 4.92417E+02 4.61125E+02 4.55083E+02Water Flow (cm) =-5.85026E-02 1.20860E-03 4.42732E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 23.5906+ 0.3810+ 0.0000 - 0.0000- 0.0000- 0.0044 = 23.9671 Versus 23.9672 Mass Balance = -7.4387E-05 cm; Time step attempts = 5093 and successes = 5093 Evaporation: Potential = 0.0000 cm, Actual = 0.0000 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 92.7 %; TMEAN = 275.7 K; HDRY = 1.0327E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 61, Simulated Time = 24.0000 hr _____ Node Number = 2 12 25 Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.27371 0.25455 0.25533 Head (cm) = 4.84198E+02 4.64193E+02 4.54958E+02 Water Flow (cm) =-1.12059E-02 8.24363E-04 4.42975E-03

Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00

INFIL RUNOFF EVAPO TRANS PRESTOR DRAIN NEWSTOR STORAGE 23.9672+ 0.0000+ 0.0000 - 0.0021- 0.0000- 0.0044 = 23.9607 Versus 23.9607 Mass Balance = 0.0000E+00 cm; Time step attempts = 96 and successes = 96 Evaporation: Potential = 0.0083 cm, Actual = 0.0021 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 95.3 %; TMEAN = 276.5 K; HDRY = 6.6547E+04 cm; DAYUBC = DAILY SUMMARY: Day = 62, Simulated Time = 24.0000 hr Node Number = 2 12 25 Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.25833 0.25434 0.25534 Head (cm) = 5.98669E+02 4.66726E+02 4.54826E+02Water Flow (cm) =-1.00796E-01 6.58817E-04 4.43324E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 23.9607+ 0.0000+ 0.0000 - 0.2246- 0.0000- 0.0044 = 23.7317 Versus 23.7317 Mass Balance = -3.8147E-06 cm; Time step attempts = 192 and successes = 192 Evaporation: Potential = 0.8982 cm, Actual = 0.2246 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 75.0 %; TMEAN = 279.8 K; HDRY = 3.9421E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 63, Simulated Time = 24.0000 hr _____ Node Number = Depth (cm) = 2 25 12 Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.24389 0.25411 0.25535 Head (cm) = 7.35481E+02 4.69455E+02 4.54769E+02Water Flow (cm) =-1.56598E-01 4.14336E-04 4.43594E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 EVAPO TRANS PRESTOR INFIL RUNOFF DRAIN NEWSTOR STORAGE 23.7317+ 0.0000+ 0.0000 - 0.4896- 0.0000- 0.0044 = 23.2376 Versus 23.2377 Mass Balance = -9.3460E-05 cm; Time step attempts = 4174 and successes = 4174 Evaporation: Potential = 1.9585 cm, Actual = 0.4896 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 57.4 %; TMEAN = 281.5 K; HDRY = 7.6038E+05 cm; DAYUBC = DAILY SUMMARY: Day = 64, Simulated Time = 24.0000 hr _____ Node Number=21225Depth (cm)=5.0000030.7000091.50000Water (cm3/cm3)=0.245840.253890.25535

= 7.14903E+02 4.72116E+02 4.54737E+02 Head (cm) Water Flow (cm) =-1.43081E-01-2.30364E-05 4.43678E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 23.2377+ 0.0000+ 0.0000 - 0.2762- 0.0000- 0.0044 = 22.9571 Versus 22.9582 Mass Balance = -1.0815E-03 cm; Time step attempts =28697 and successes =28697 Evaporation: Potential = 1.3641 cm, Actual = 0.2762 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 66.2 %; TMEAN = 278.7 K; HDRY = 5.6541E+05 cm; DAYUBC = 2672 DAILY SUMMARY: Day = 65, Simulated Time = 24.0000 hr _____ Node Number = Depth (cm) = 2 12 25 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.25081 0.25365 0.25535Head (cm) = 6.65679E+02 4.75042E+02 4.54747E+02Water Flow (cm) =-4.30699E-02-4.35280E-04 4.43730E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 22.9582+ 0.0000+ 0.0000 - 0.0223- 0.0000- 0.0044 = 22.9314 Versus 22.9316 Mass Balance = -1.4687E-04 cm; Time step attempts =12591 and successes =12591 Evaporation: Potential = 1.0253 cm, Actual = 0.0223 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 60.6 %; TMEAN = 279.0 K; HDRY = 6.8577E+05 cm; DAYUBC = 3468 DAILY SUMMARY: Day = 66, Simulated Time = 24.0000 hr _____ 2 Node Number = 12 25 Depth (cm) Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.25185 0.25343 0.25534 = 6.55953E+02 4.77696E+02 4.54847E+02 Head (cm) Water Flow (cm) =-2.52751E-02-6.97577E-04 4.43597E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 22.9316+ 0.0000+ 0.0000 - 0.0216- 0.0000- 0.0044 = 22.9056 Versus 22.9060 Mass Balance = -4.5395E-04 cm; Time step attempts = 7593 and successes = 7593 Evaporation: Potential = 1.5673 cm, Actual = 0.0216 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cmRHMEAN = 55.9 %; TMEAN = 281.5 K; HDRY = 7.9718E+05 cm; DAYUBC = 3921 

DAILY SUMMARY: Day = 67, Simulated Time = 24.0000 hr

Node Number=21225Depth (cm)=5.0000030.7000091.50000Water (cm3/cm3)=0.251510.253240.25533 Head (cm) = 6.59133E+024.80118E+024.55037E+02Water Flow (cm) =-2.13509E-02-8.38843E-04 4.43218E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 22.9060+ 0.0000+ 0.0000 - 0.0205- 0.0000- 0.0044 = 22.8810 Versus 22.8814 Mass Balance = -3.4523E-04 cm; Time step attempts = 3167 and successes = 3167Evaporation: Potential = 0.9421 cm, Actual = 0.0205 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 65.1 %; TMEAN = 279.5 K; HDRY = 5.8767E+05 cm; DAYUBC = 3141 DAILY SUMMARY: Day = 68, Simulated Time = 24.0000 hr ______ Node Number = 2 12 25 Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.25125 0.25307 0.25530 Head (cm)  $= 6.61579E+02 \ 4.82229E+02 \ 4.55301E+02$ Water Flow (cm) =-1.85639E-02-9.10405E-04 4.42641E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 22.8814+ 0.0000+ 0.0000 - 0.0229- 0.0000- 0.0044 = 22.8541 Versus 22.8554 Mass Balance = -1.3142E-03 cm; Time step attempts =12591 and successes =12591 Evaporation: Potential = 1.6632 cm, Actual = 0.0229 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 52.1 %; TMEAN = 280.7 K; HDRY = 8.9332E+05 cm; DAYUBC = 2951 DAILY SUMMARY: Day = 69, Simulated Time = 24.0000 hr _____ 2 Node Number == 12 25 Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.25073 0.25290 0.25527 Head (cm)  $= 6.66476E+02 \ 4.84255E+02 \ 4.55655E+02$ Water Flow (cm) =-1.71480E-02-9.64718E-04 4.41831E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 22.8554+ 0.0000+ 0.0000 - 0.0195- 0.0000- 0.0044 = 22.8314 Versus 22.8321 Mass Balance = -6.4278E-04 cm; Time step attempts = 4976 and successes = 4976Evaporation: Potential = 2.5464 cm, Actual = 0.0195 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 52.1 %; TMEAN = 282.9 K; HDRY = 8.9255E+05 cm; DAYUBC = 4964

DAILY SUMMARY: Day = 70, Simulated Time = 24.0000 hr ------Node Number = Depth (cm) = 2 12 25 Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.27813 0.25290 0.25524 91.50000 Head (cm) = 4.55868E+02 4.84279E+02 4.56083E+02Water Flow (cm) = 5.90730E-01-6.92524E-04 4.40820E-03Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 22.8321+ 1.4986+ 0.0000 - 0.3125- 0.0000- 0.0044 = 24.0137 Versus 24.0144 Mass Balance = -6.1035E-04 cm; Time step attempts = 5326 and successes = 5326 Evaporation: Potential = 1.2756 cm, Actual = 0.3125 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 60.8 %; TMEAN = 287.0 K; HDRY = 6.8103E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 71, Simulated Time = 24.0000 hr Node Number=21225Depth (cm)=5.0000030.7000091.50000Water (cm3/cm3)=0.263940.253000.25519 Head (cm) = 5.53705E+02 4.83060E+02 4.56594E+02Water Flow (cm) =-4.72954E-02 5.53391E-04 4.39599E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 24.0144+ 0.1524+ 0.0000 - 0.3095- 0.0000- 0.0044 = 23.8528 Versus 23.8530 Mass Balance = -1.5068E-04 cm; Time step attempts = 2248 and successes = 2248 Evaporation: Potential = 1.2506 cm, Actual = 0.3095 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 77.7 %; TMEAN = 282.6 K; HDRY = 3.4503E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 72, Simulated Time = 24.0000 hr _____ Node Number = Depth (cm) = 2 12 25 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.25070 0.25296 0.25515Head (cm) =  $6.66742E+02 \ 4.83519E+02 \ 4.5/121E+02$ Water Flow (cm) =- $1.71203E-01 \ 7.65793E-04 \ 4.38197E-03$ Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 23.8530+ 0.0000+ 0.0000 - 0.5490- 0.0000- 0.0044 = 23.2996 Versus 23.2999 Mass Balance = -3.3760E-04 cm; Time step attempts = 5786 and successes = 5786 Evaporation: Potential = 2.1961 cm, Actual = 0.5490 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm

RHMEAN = 48.9 %; TMEAN = 286.2 K; HDRY = 9.8105E+05 cm; DAYUBC =

DAILY SUMMARY: Day = 73, Simulated Time = 24.0000 hr Node Number = 2 12 2.5 Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.25133 0.25285 0.25511 Depth (cm) Head (cm) = 6.60795E+02 4.84871E+02 4.57564E+02Water Flow (cm) =-1.46153E-01 3.60408E-04 4.36963E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 23.2999+ 0.0000+ 0.0000 - 0.2432- 0.0000- 0.0044 = 23.0523 Versus 23.0537 Mass Balance = -1.4038E-03 cm; Time step attempts =40592 and successes =40592 Evaporation: Potential = 1.0460 cm, Actual = 0.2432 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 77.5 %; TMEAN = 284.8 K; HDRY = 3.4858E+05 cm; DAYUBC = 4166 DAILY SUMMARY: Day = 74, Simulated Time = 24.0000 hr 2 Node Number = 12 25 Node Number-2-Depth (cm)=5.0000030.7000091.50000Water (cm3/cm3)=0.237130.252690.25505Head (cm) = 8.12540E+02 4.86916E+02 4.58222E+02Water Flow (cm) =-1.49842E-01-1.09319E-04 4.35613E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 INFIL RUNOFF PRESTOR EVAPO TRANS DRAIN NEWSTOR STORAGE 23.0537+ 0.2286+ 0.0000 - 0.1027- 0.0000- 0.0044 = 23.1753 Versus 23.1754 Mass Balance = -1.5450E-04 cm; Time step attempts = 7727 and successes = 7727 Evaporation: Potential = 0.4149 cm, Actual = 0.1027 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 83.2 %; TMEAN = 281.5 K; HDRY = 2.5259E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 75, Simulated Time = 24.0000 hr ______ Node Number = 2 12 25 Depth (cm) Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.23136 0.25251 0.25500 Head (cm)  $= 8.86504\pm024.89164\pm024.58909\pm02$ Water Flow (cm) =-1.23196E-01-4.92886E-04 4.33838E-03Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 23.1754+ 0.0000+ 0.0000 - 0.3805- 0.0000- 0.0043 = 22.7907 Versus 22.7908

Mass Balance = -1.4305E-04 cm; Time step attempts = 3826 and successes = 3826Evaporation: Potential = 1.5218 cm, Actual = 0.3805 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 61.3 %; TMEAN = 279.0 K; HDRY = 6.6982E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 76, Simulated Time = 24.0000 hr 2 12 Node Number ----25 Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.23601 0.25235 0.25495 = 8.26288E+02 4.91163E+02 4.59503E+02 Head (cm) Water Flow (cm) =-9.22291E-02-8.05696E-04 4.32247E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS STORAGE DRAIN NEWSTOR 22.7908+ 0.0000+ 0.0000 - 0.1860- 0.0000- 0.0043 = 22.6005 Versus 22.6017 Mass Balance = -1.2302E-03 cm; Time step attempts =27961 and successes =27961 Evaporation: Potential = 1.5011 cm, Actual = 0.1860 cmTranspiration: Potential = 0.0000 cm, Actual = 0.0000 cmRHMEAN = 60.3 %; TMEAN = 278.7 K; HDRY = 6.9271E+05 cm; DAYUBC = 4844 DAILY SUMMARY: Day = 77, Simulated Time = 24.0000 hr _____ Node Number = Depth (cm) = 2 12 25 5.00000 30.70000 91.50000 = 0.23981 0.25218 0.25488Water (cm3/cm3) = 7.80833E+02 4.93374E+02 4.60226E+02Head (cm) Water Flow (cm) =-3.53022E-02-1.05410E-03 4.30592E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 22.6017+ 0.0000+ 0.0000 - 0.0169- 0.0000- 0.0043 = 22.5805 Versus 22.5807 Mass Balance = -2.3842E-04 cm; Time step attempts = 4298 and successes = 4298Evaporation: Potential = 0.6794 cm, Actual = 0.0169 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 65.0 %; TMEAN = 280.1 K; HDRY = 5.9016E+05 cm; DAYUBC = 4278 DAILY SUMMARY: Day = 78, Simulated Time = 24.0000 hr _____ Node Number Depth (cm) 2 12 ----25 Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.24087 0.25202 0.25482 = 7.68726E+02 4.95413E+02 4.60966E+02 Head (cm) Water Flow (cm) =-2.34128E-02-1.21642E-03 4.28763E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00

PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 22.5807+ 0.0000+ 0.0000 - 0.0190- 0.0000- 0.0043 = 22.5574 Versus 22.5577 Mass Balance = -2.1553E-04 cm; Time step attempts = 5094 and successes = 5094 Evaporation: Potential = 1.1055 cm, Actual = 0.0190 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 63.4 %; TMEAN = 281.5 K; HDRY = 6.2473E+05 cm; DAYUBC = 2756 DAILY SUMMARY: Day = 79, Simulated Time = 24.0000 hr ______ Node Number 2100 2 12 25 Note Hanber=5.0000030.7000091.50000Depth (cm)=0.241070.251870.25476Water (cm3/cm3)=0.241070.251870.25476Head (cm)  $= 7.66504\pm024.97274\pm024.61714\pm02$ Water Flow (cm) =-1.87647E-02-1.30643E-03 4.26913E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 22.5577+ 0.0000+ 0.0000 - 0.0190- 0.0000- 0.0043 = 22.5344 Versus 22.5350 Mass Balance = -6.5613E-04 cm; Time step attempts = 7593 and successes = 7593 Evaporation: Potential = 1.6059 cm, Actual = 0.0190 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 58.8 %; TMEAN = 280.7 K; HDRY = 7.2878E+05 cm; DAYUBC = 3913 DAILY SUMMARY: Day = 80, Simulated Time = 24.0000 hr _____ 12 Node Number 2 25 Depth (cm) Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.24098 0.25174 0.25469 Head (cm) = 7.67519E+02 4.99020E+02 4.62481E+02Water Flow (cm) =-1.61128E-02-1.35958E-03 4.25038E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 22.5350+ 0.0000+ 0.0000 - 0.0184- 0.0000- 0.0043 = 22.5124 Versus 22.5131 Mass Balance = -7.6675E-04 cm; Time step attempts = 7593 and successes = 7593 Evaporation: Potential = 2.1500 cm, Actual = 0.0184 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 49.6 %; TMEAN = 284.3 K; HDRY = 9.6041E+05 cm; DAYUBC = 3437 DAILY SUMMARY: Day = 81, Simulated Time = 24.0000 hr _____ 2 12 Node Number = 25 Note Number = 2Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.24066 0.25161 0.25463 = 0.24066 0.25161 0.25463 Head (cm) = 7.71148E+02 5.00683E+02 4.63285E+02

Water Flow (cm) =-1.48041E-02-1.39652E-03 4.23083E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 22.5131+ 0.0000+ 0.0000 - 0.0165- 0.0000- 0.0042 = 22.4924 Versus 22.4930 Mass Balance = -5.7220E-04 cm; Time step attempts = 5094 and successes = 5094Evaporation: Potential = 0.7520 cm, Actual = 0.0165 cm Transpiration: Potential = 2.1508 cm, Actual = 0.0000 cm RHMEAN = 48.5 %; TMEAN = 290.4 K; HDRY = 9.9280E+05 cm; DAYUBC = 3291 DAILY SUMMARY: Day = 82, Simulated Time = 24.0000 hr _____ 2 Node Number = 12 25 

 Note Number
 2
 12
 23

 Depth (cm)
 =
 5.00000
 30.70000
 91.50000

 Water (cm3/cm3)
 =
 0.20922
 0.25148
 0.25455

Head (cm) = 1.26757E+03 5.02365E+02 4.64148E+02Water Flow (cm) =-5.42692E-02-1.42232E-03 4.21000E-03 Plant Sink (cm) =  $4.04257E-02 \ 0.00000E+00 \ 0.00000E+00$ PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 22.4930+ 0.0000+ 0.0000 - 0.0110- 0.3364- 0.0042 = 22.1414 Versus 22.1414 Mass Balance = -4.0054E-05 cm; Time step attempts = 981 and successes = 981 Evaporation: Potential = 0.4704 cm, Actual = 0.0110 cm Transpiration: Potential = 1.3454 cm, Actual = 0.3364 cm RHMEAN = 61.3 %; TMEAN = 288.2 K; HDRY = 6.6988E+05 cm; DAYUBC = 955 DAILY SUMMARY: Day = 83, Simulated Time = 24.0000 hr ------Node Number Depth (cm) 2 = 12 25 Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.14821 0.25137 0.25450 Head (cm) = 4.97827E+03 5.03746E+02 4.64786E+02Water Flow (cm) =-5.67184E-02-1.51296E-03 4.19101E-03 Plant Sink (cm) =  $7.17160E-02 \ 0.00000E+00 \ 0.00000E+00$ PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 22.1414+ 0.0000+ 0.0000 - 0.0111- 0.6229- 0.0042 = 21.5033 Versus 21.5045 Mass Balance = -1.2074E-03 cm; Time step attempts =64976 and successes =64976 Evaporation: Potential = 0.9141 cm, Actual = 0.0111 cm Transpiration: Potential = 2.6147 cm, Actual = 0.6229 cm RHMEAN = 48.5 %; TMEAN = 281.8 K; HDRY = 9.9144E+05 cm; DAYUBC = 49864 DAILY SUMMARY: Day = 84, Simulated Time = 24.0000 hr _____ Node Number 2 12 = 25

Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.13996 0.25136 0.25450 Head (cm) = 6.52528E+03 5.03901E+02 4.64837E+02Water Flow (cm) =-3.84518E-02-1.60082E-03 4.18254E-03 Plant Sink (cm) = 1.53461E-02 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 21.5045+ 0.0000+ 0.0000 - 0.0056- 0.1839- 0.0042 = 21.3109 Versus 21.3165 Mass Balance = -5.6610E-03 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.3498 cm, Actual = 0.0056 cm Transpiration: Potential = 1.0004 cm, Actual = 0.1839 cm RHMEAN = 51.4 %; TMEAN = 276.2 K; HDRY = 9.1100E+05 cm; DAYUBC = ***** DAILY SUMMARY: Day = 85, Simulated Time = 24.0000 hr Node Number = Depth (cm) = 2 12 25 Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.13062 0.25136 0.25450 Head (cm) = 9.29712E+03 5.03903E+02 4.64837E+02Water Flow (cm) =-2.55907E-02-1.60262E-03 4.18246E-03 Plant Sink (cm) = 1.34982E-02 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 21.3165+ 0.0000+ 0.0000 - 0.0089- 0.2192- 0.0042 = 21.0843 Versus 21.0912 Mass Balance = -6.9046E-03 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.4877 cm, Actual = 0.0089 cm Transpiration: Potential = 1.3950 cm, Actual = 0.2192 cm RHMEAN = 41.1 %; TMEAN = 277.6 K; HDRY = 1.2170E+06 cm; DAYUBC = ***** DAILY SUMMARY: Day = 86, Simulated Time = 24.0000 hr _____ 2 12 Node Number = 25 
 Node Number
 =
 2
 12

 Depth (cm)
 =
 5.00000
 30.70000

 Water (cm3/cm3)
 =
 0.13259
 0.25136
91.50000 0.25136 0.25450 Head (cm) = 8.58570E+03 5.03903E+02 4.64837E+02Water Flow (cm) =-1.78957E-02-1.60347E-03 4.18246E-03 Plant Sink (cm) = 4.72602E-03 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 21.0912+ 0.2286+ 0.0000 - 0.0532- 0.0894- 0.0042 = 21.1730 Versus 21.1799 Mass Balance = -6.8111E-03 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.2151 cm, Actual = 0.0532 cmTranspiration: Potential = 0.6153 cm, Actual = 0.0894 cm RHMEAN = 49.6 %; TMEAN = 280.4 K; HDRY = 9.5991E+05 cm; DAYUBC = 0 

DAILY SUMMARY: Day = 87, Simulated Time = 24.0000 hr Node Number 22 2 12 25 Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.14776 0.25136 0.25450 Head (cm) = 5.04810E+03 5.03903E+02 4.64837E+02Water Flow (cm) =-2.24365E-02-1.60347E-03 4.18246E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 21.1799+ 0.1270+ 0.0000 - 0.0000- 0.0000- 0.0042 = 21.3027 Versus 21.3120 Mass Balance = -9.3155E-03 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.0000 cm, Actual = 0.0000 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 90.6 %; TMEAN = 279.0 K; HDRY = 1.3578E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 88, Simulated Time = 24.0000 hr _____ Node Number 2 == 25 12 Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.16398 0.25136 0.25450 Depth (cm) Head (cm) = 3.21064E+03 5.03904E+02 4.64837E+02Water Flow (cm) =-2.51996E-02-1.60371E-03 4.18246E-03 Plant Sink (cm) = 1.43161E-04 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 21.3120+ 0.0508+ 0.0000 - 0.0005- 0.0013- 0.0042 = 21.3567 Versus 21.3652 Mass Balance = -8.4610E-03 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.0021 cm, Actual = 0.0005 cm Transpiration: Potential = 0.0061 cm, Actual = 0.0013 cm RHMEAN = 88.1 %; TMEAN = 280.9 K; HDRY = 1.7350E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 89, Simulated Time = 24.0000 hr _____ Node Number 2222 2 12 25 Depth (cm) = 5.00000 Water (cm3/cm3) = 0.14537 30.70000 91.50000 0.25136 0.25450 Head (cm) = 5.44376E+03 5.03906E+02 4.64837E+02Water Flow (cm) =-1.82563E-02-1.60451E-03 4.18246E-03 Plant Sink (cm) = 2.44321E-02 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 21.3652+ 0.0000+ 0.0000 - 0.0959- 0.2349- 0.0042 = 21.0301 Versus 21.0361 Mass Balance = -5.9166E-03 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.3838 cm, Actual = 0.0959 cm Transpiration: Potential = 1.0977 cm, Actual = 0.2349 cm RHMEAN = 71.8 %; TMEAN = 284.3 K; HDRY = 4.5453E+05 cm; DAYUBC = Ο

DAILY SUMMARY: Day = 90, Simulated Time = 24.0000 hr 25 Node Number = 2 12 Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.13341 0.25136 0.25450 Head (cm) = 8.31095E+03 5.03912E+02 4.64837E+02Water Flow (cm) =-1.08255E-02-1.60784E-03 4.18246E-03 Plant Sink (cm) = 1.42647E-02 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 21.0361+ 0.0254+ 0.0000 - 0.1034- 0.1844- 0.0042 = 20.7695 Versus 20.7740 Mass Balance = -4.5185E-03 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.4179 cm, Actual = 0.1034 cmTranspiration: Potential = 1.1952 cm, Actual = 0.1844 cm RHMEAN = 66.5 %; TMEAN = 283.2 K; HDRY = 5.6000E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 91, Simulated Time = 24.0000 hr ______ Node Number ----2 12 25 Depth (cm) = 5.00000 30.70000Water (cm3/cm3) = 0.12770 0.2513691.50000 0.25450 Head (cm) = 1.05294E+04 5.03916E+02 4.64837E+02Water Flow (cm) =-6.39042E-03-1.61326E-03 4.18246E-03 Plant Sink (cm) = 6.64049E-03 0.00000E+00 0.00000E+00PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 20.7740+ 0.0000+ 0.0000 - 0.0619- 0.1184- 0.0042 = 20.5895 Versus 20.5986 Mass Balance = -9.0885E-03 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.3479 cm, Actual = 0.0619 cmTranspiration: Potential = 0.9951 cm, Actual = 0.1184 cmRHMEAN = 59.8 %; TMEAN = 283.7 K; HDRY = 7.0524E+05 cm; DAYUBC = ***** DAILY SUMMARY: Day = 92, Simulated Time = 24.0000 hr _____ Node Number = Depth (cm) = 2 12 25 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.12509 0.251360.25450 Head (cm) = 1.18384E+04 5.03916E+02 4.64837E+02Water Flow (cm) =-4.71530E-03-1.61374E-03 4.18246E-03 Plant Sink (cm) = 3.26181E-03 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 20.5986+ 0.0254+ 0.0000 - 0.0229- 0.0816- 0.0042 = 20.5153 Versus 20.5238 Mass Balance = -8.4057E-03 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.2923 cm, Actual = 0.0229 cm

Transpiration: Potential = 0.8362 cm, Actual = 0.0816 cm RHMEAN = 75.0 %; TMEAN = 292.0 K; HDRY = 3.9374E+05 cm; DAYUBC = ***** DAILY SUMMARY: Day = 93, Simulated Time = 24.0000 hr _____ Node Number Depth (cm) = 2 12 25 Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.12158 0.25136 0.25450 Head (cm) = 1.40242E+04 5.03928E+02 4.64837E+02Water Flow (cm) =-2.90231E-03-1.61843E-03 4.18246E-03 Plant Sink (cm) = 3.37386E-03 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 20.5238+ 0.0000+ 0.0000 - 0.0067- 0.1353- 0.0042 = 20.3775 Versus 20.3840 Mass Balance = -6.4754E-03 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.6417 cm, Actual = 0.0067 cmTranspiration: Potential = 1.8356 cm, Actual = 0.1353 cm RHMEAN = 68.0 %; TMEAN = 297.9 K; HDRY = 5.2825E+05 cm; DAYUBC = ***** DAILY SUMMARY: Day = 94, Simulated Time = 24.0000 hr _____ Node Number=21225Depth (cm)=5.0000030.7000091.50000Water (cm3/cm3)=0.120380.251160.25446 Head (cm) = 1.49115E+04 5.06429E+02 4.65312E+02Water Flow (cm) =-1.38565E-03-2.24772E-03 4.17444E-03 Plant Sink (cm) = 7.30103E-04 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 20.3840+ 0.0000+ 0.0000 - 0.0015- 0.0899- 0.0042 = 20.2884 Versus 20.2911 Mass Balance = -2.6188E-03 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.6055 cm, Actual = 0.0015 cm Transpiration: Potential = 1.7320 cm, Actual = 0.0899 cm RHMEAN = 73.3 %; TMEAN = 296.5 K; HDRY = 4.2502E+05 cm; DAYUBC = ***** DAILY SUMMARY: Day = 95, Simulated Time = 24.0000 hr _____ Node Number Depth (cm) ----2 12 25 Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.11992 0.25095 0.25438 Head (cm) = 1.52676E+04 5.09246E+02 4.66210E+02Water Flow (cm) =-9.03009E-04-2.43922E-03 4.16066E-03 Plant Sink (cm) = 2.14977E-06 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 20.2911+ 0.0000+ 0.0000 - 0.0000- 0.0464- 0.0042 = 20.2405 Versus 20.2407

Mass Balance = -1.9455E-04 cm; Time step attempts = 2595 and successes = 2595 Evaporation: Potential = 0.3736 cm, Actual = 0.0000 cm Transpiration: Potential = 1.0687 cm, Actual = 0.0464 cm RHMEAN = 81.9 %; TMEAN = 296.2 K; HDRY = 2.7302E+05 cm; DAYUBC = DAILY SUMMARY: Day = 96, Simulated Time = 24.0000 hr _____ Node Number ::::: 2 12 25 Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.11914 0.25095 0.25438 Head (cm) = 1.59071E+04 5.09246E+02 4.66210E+02Water Flow (cm) =-6.11866E-04-2.48280E-03 4.14929E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 20.2407+ 0.0000+ 0.0000 - 0.0110- 0.0907- 0.0041 = 20.1349 Versus 20.1424 Mass Balance = -7.4997E-03 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.9040 cm, Actual = 0.0110 cmTranspiration: Potential = 2.5857 cm, Actual = 0.0907 cm RHMEAN = 62.7 %; TMEAN = 298.4 K; HDRY = 6.3905E+05 cm; DAYUBC = ***** DAILY SUMMARY: Day = 97, Simulated Time = 24.0000 hr ______ Node Number = 2 12 25 Node Number = Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.11776 0.25095 0.25438Head (cm) = 1.71451E+04 5.09246E+02 4.66210E+02Water Flow (cm) =-3.56162E-04-2.48279E-03 4.14929E-03Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 20.1424+ 0.0000+ 0.0000 - 0.0122- 0.0913- 0.0041 = 20.0348 Versus 20.0439 Mass Balance = -9.1324E-03 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 1.4528 cm, Actual = 0.0122 cm Transpiration: Potential = 4.1554 cm, Actual = 0.0913 cm RHMEAN = 37.9 %; TMEAN = 297.3 K; HDRY = 1.3282E+06 cm; DAYUBC = ***** _____ DAILY SUMMARY: Day = 98, Simulated Time = 24.0000 hr 2 Node Number == 12 25 Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.11661 0.25076 0.25430 = 1.82945E+04 5.11686E+02 4.67135E+02 Head (cm) Water Flow (cm) =-3.30205E-04-2.52674E-03 4.13876E-03

Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00

INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR PRESTOR STORAGE 20.0439+ 0.0000+ 0.0000 - 0.0000- 0.0401- 0.0041 = 19.9996 Versus 19.9998 Mass Balance = -1.7166E-04 cm; Time step attempts = 2595 and successes = 2595 Evaporation: Potential = 0.8720 cm, Actual = 0.0000 cm Transpiration: Potential = 2.4941 cm, Actual = 0.0401 cm RHMEAN = 67.1 %; TMEAN = 297.6 K; HDRY = 5.4756E+05 cm; DAYUBC = DAILY SUMMARY: Day = 99, Simulated Time = 24.0000 hr Node Number = Depth (cm) = 2 12 25 Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.11647 0.25074 0.25429 Head (cm) = 1.84412E+045.12043E+024.67284E+02Water Flow (cm) =-3.50103E-04-2.54705E-03 4.12396E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 19.9998+ 0.0000+ 0.0000 - 0.0034- 0.0416- 0.0041 = 19.9507 Versus 19.9620 Mass Balance = -1.1278E-02 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 1.0269 cm, Actual = 0.0034 cm Transpiration: Potential = 2.9371 cm, Actual = 0.0416 cm RHMEAN = 67.0 %; TMEAN = 298.2 K; HDRY = 5.4966E+05 cm; DAYUBC = ***** DAILY SUMMARY: Day = 100, Simulated Time = 24.0000 hr _____ Node Number = 2 12 25 Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.11590 0.25060 0.25423 Head (cm) = 1.90491E+04 5.13792E+02 4.68040E+02Water Flow (cm) =-3.67366E-04-2.56141E-03 4.11325E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 19.9620+ 0.0000+ 0.0000 - 0.0006- 0.0169- 0.0041 = 19.9403 Versus 19.9433 Mass Balance = -3.0499E-03 cm; Time step attempts =50361 and successes =50361 Evaporation: Potential = 0.4196 cm, Actual = 0.0006 cmTranspiration: Potential = 1.2001 cm, Actual = 0.0169 cm RHMEAN = 74.4 %; TMEAN = 297.0 K; HDRY = 4.0514E+05 cm; DAYUBC = 47783 DAILY SUMMARY: Day = 101, Simulated Time = 24.0000 hr _____ Node Number = 2 12 25 Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.13674 0.25060 0.25423

Head (cm) = 7.32364E+03 5.13792E+02 4.68040E+02Water Flow (cm) = 6.36401E-02-2.56558E-03 4.10564E-03 Plant Sink (cm) = 1.07453E-01 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 19.9433+ 1.0693+ 0.1753 - 0.3629- 0.5825- 0.0041 = 20.0631 Versus 20.0706 Mass Balance = -7.4310E-03 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 1.4814 cm, Actual = 0.3629 cm Transpiration: Potential = 4.2374 cm, Actual = 0.5825 cm RHMEAN = 48.7 %; TMEAN = 291.2 K; HDRY = 9.8604E+05 cm; DAYUBC = 3949 DAILY SUMMARY: Day = 102, Simulated Time = 24.0000 hr ______ Node Number = Depth (cm) = 2 12 25 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.12589 0.25060 0.25423= 1.14156E+04 5.13792E+02 4.68040E+02Head (cm) Water Flow (cm) = 1.20182E-03-2.56558E-03 4.10564E-03Plant Sink (cm) = 9.77031E-03 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 20.0706+ 0.0000+ 0.0000 - 0.0729- 0.0686- 0.0041 = 19.9250 Versus 19.9329 Mass Balance = -7.8697E-03 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.5157 cm, Actual = 0.0729 cm Transpiration: Potential = 1.4751 cm, Actual = 0.0686 cm RHMEAN = 61.5 %; TMEAN = 285.9 K; HDRY = 6.6719E+05 cm; DAYUBC = ***** DAILY SUMMARY: Day = 103, Simulated Time = 24.0000 hr ______ Node Number Depth (cm) 2 12 25 Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.12156 0.25057 0.25421 = 1.40368E+04 5.14192E+02 4.68208E+02 Head (cm) Water Flow (cm) = 1.11864E-04-2.57253E-03 4.10228E-03Plant Sink (cm) = 3.34618E-03 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 19.9329+ 0.0000+ 0.0000 - 0.0035- 0.0408- 0.0041 = 19.8846 Versus 19.9017 Mass Balance = -1.7113E-02 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.5835 cm, Actual = 0.0035 cm Transpiration: Potential = 1.6689 cm, Actual = 0.0408 cm RHMEAN = 64.8 %; TMEAN = 288.4 K; HDRY = 5.9468E+05 cm; DAYUBC = **** 

DAILY SUMMARY: Day = 104, Simulated Time = 24.0000 hr

Node Number = 2 12 25 Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.12051 0.25042 0.25413 Head (cm) = 1.48114E+04 5.16248E+02 4.69189E+02Water Flow (cm) =-1.02729E-04-2.58187E-03 4.09066E-03 Plant Sink (cm) =  $2.65312E-04 \ 0.00000E+00 \ 0.00000E+00$ PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 19.9017+ 0.0000+ 0.0000 - 0.0000- 0.0101- 0.0041 = 19.8875 Versus 19.8877 Mass Balance = -1.6403E-04 cm; Time step attempts = 2595 and successes = 2595 Evaporation: Potential = 0.1971 cm, Actual = 0.0000 cmTranspiration: Potential = 0.5637 cm, Actual = 0.0101 cm RHMEAN = 87.1 %; TMEAN = 289.8 K; HDRY = 1.8918E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 105, Simulated Time = 24.0000 hr _____ Node Number ..... 2 12 2.5 Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.19513 0.25042 0.25413 Depth (cm) = Head (cm) = 1.63337E+03 5.16248E+02 4.69189E+02Water Flow (cm) = 2.71689E-01-2.58406E-03 4.07856E-03Plant Sink (cm) = 5.60533E-02 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 19.8877+ 1.3051+ 0.6507 - 0.1631- 0.3882- 0.0041 = 20.6375 Versus 20.6600 Mass Balance = -2.2522E-02 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.6655 cm, Actual = 0.1631 cm Transpiration: Potential = 1.9037 cm, Actual = 0.3882 cm RHMEAN = 72.9 %; TMEAN = 290.1 K; HDRY = 4.3325E+05 cm; DAYUBC = 12047 DAILY SUMMARY: Day = 106, Simulated Time = 24.0000 hr _____ 2 Node Number 12 25 = Depth (cm) = 5.00000 30.70000Water (cm3/cm3) = 0.14413 0.2504291.50000 0.25042 0.25413 Head (cm) = 5.66692E+03 5.16248E+02 4.69189E+02Water Flow (cm) = 2.43065E-02-2.58406E-03 4.07856E-03 Plant Sink (cm) = 4.68804E-02 0.00000E+00 0.00000E+00 INFIL RUNOFF DRAIN NEWSTOR PRESTOR EVAPO TRANS STORAGE 20.6600+ 0.0000+ 0.0000 - 0.1597- 0.3285- 0.0041 = 20.1677 Versus 20.1911 Mass Balance = -2.3458E-02 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.6388 cm, Actual = 0.1597 cm Transpiration: Potential = 1.8270 cm, Actual = 0.3285 cm RHMEAN = 60.2 %; TMEAN = 285.4 K; HDRY = 6.9554E+05 cm; DAYUBC = 0

DAILY SUMMARY: Day = 107, Simulated Time = 24.0000 hr _____ Node Number=21225Depth (cm)=5.0000030.7000091.50000Water (cm3/cm3)=0.125870.250420.25413 Head (cm) = 1.14268E+04 5.16248E+02 4.69189E+02Water Flow (cm) =-3.26557E-04-2.58406E-03 4.07856E-03 Plant Sink (cm) = 1.74880E-02 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 20.1911+ 0.0000+ 0.0000 - 0.1259- 0.1564- 0.0041 = 19.9048 Versus 19.9303 Mass Balance = -2.5482E-02 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.7307 cm, Actual = 0.1259 cmTranspiration: Potential = 2.0900 cm, Actual = 0.1564 cmRHMEAN = 53.5 %; TMEAN = 282.0 K; HDRY = 8.5655E+05 cm; DAYUBC = ***** DAILY SUMMARY: Day = 108, Simulated Time = 24.0000 hr Node Number Depth (cm) = 2 12 25 Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.12147 0.25039 0.25412 Head (cm) = 1.41039E+04 5.16652E+02 4.69387E+02Water Flow (cm) =-5.90899E-04-2.58344E-03 4.07453E-03 Plant Sink (cm) = 3.21530E-03 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 19.9303+ 0.0000+ 0.0000 - 0.0039- 0.0536- 0.0041 = 19.8687 Versus 19.8877 Mass Balance = -1.8932E-02 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.5602 cm, Actual = 0.0039 cmTranspiration: Potential = 1.6023 cm, Actual = 0.0536 cm RHMEAN = 58.9 %; TMEAN = 282.9 K; HDRY = 7.2567E+05 cm; DAYUBC = ***** DAILY SUMMARY: Day = 109, Simulated Time = 24.0000 hr ______ Node Number = Depth (cm) = 12 25 2 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.12013 0.25029 0.25406Head (cm) = 1.51003E+04 5.18053E+02 4.70117E+02 Water Flow (cm) =-4.03992E-04-2.58455E-03 4.06348E-03 Plant Sink (cm) = 4.99091E-04 0.00000E+00 0.00000E+00PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 19.8877+ 0.0000+ 0.0000 - 0.0010- 0.0358- 0.0041 = 19.8468 Versus 19.8532 Mass Balance = -6.3801E-03 cm; Time step attempts =67374 and successes =67374 Evaporation: Potential = 0.6179 cm, Actual = 0.0010 cmTranspiration: Potential = 1.7674 cm, Actual = 0.0358 cmRHMEAN = 69.7 %; TMEAN = 290.1 K; HDRY = 4.9529E+05 cm; DAYUBC = 64817

DAILY SUMMARY: Day = 110, Simulated Time = 24.0000 hr = Node Number Node Number = Depth (cm) = 2 12 25 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.11946 0.250160.25398 = 1.56429E+04 5.19678E+02 4.71017E+02 Head (cm) Water Flow (cm) =-2.75373E-04-2.58268E-03 4.04553E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 19.8532+ 0.0000+ 0.0000 - 0.0004- 0.0250- 0.0040 = 19.8237 Versus 19.8264 Mass Balance = -2.6016E-03 cm; Time step attempts =32967 and successes =32967 Evaporation: Potential = 0.5505 cm, Actual = 0.0004 cm Transpiration: Potential = 1.5745 cm, Actual = 0.0250 cm RHMEAN = 76.5 %; TMEAN = 295.9 K; HDRY = 3.6803E+05 cm; DAYUBC = 30381 DAILY SUMMARY: Day = 111, Simulated Time = 24.0000 hr ______ Node Number ----2 12 25 Depth (cm) = 5.00000 30.70000 Water (cm3/cm3) = 0.11882 0.25013 91.50000 0.25396 = 1.61890E+04 5.20196E+02 4.71312E+02Head (cm) Water Flow (cm) =-2.48003E-04-2.57652E-03 4.03029E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 19.8264+ 0.0000+ 0.0000 - 0.0019- 0.0174- 0.0040 = 19.8030 Versus 19.8176 Mass Balance = -1.4559E-02 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.4118 cm, Actual = 0.0019 cm Transpiration: Potential = 1.1778 cm, Actual = 0.0174 cm RHMEAN = 78.1 %; TMEAN = 294.8 K; HDRY = 3.3948E+05 cm; DAYUBC = ***** DAILY SUMMARY: Day = 112, Simulated Time = 24.0000 hr Node Number 2 12 25 = Depth (cm) = 5.00000 30.70000 91.50000 Dependence(cm)=5.0000030.7000091.50000Water (cm3/cm3)=0.233350.250130.25396Head (cm) = 8.59990E+02 5.20196E+02 4.71312E+02Water Flow (cm) = 8.97355E-01-2.57684E-03 4.02913E-03Plant Sink (cm) =  $4.54513E-02 \ 0.00000E+00 \ 0.00000E+00$ PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 19.8176+ 2.1170+ 0.8040 - 0.1322- 0.3718- 0.0040 = 21.4265 Versus 21.4445 Mass Balance = -1.7935E-02 cm; Time step attempts =***** and successes =*****

Evaporation: Potential = 0.5453 cm, Actual = 0.1322 cm Transpiration: Potential = 1.5597 cm, Actual = 0.3718 cm RHMEAN = 73.8 %; TMEAN = 294.8 K; HDRY = 4.1700E+05 cm; DAYUBC = 14908 DAILY SUMMARY: Day = 113, Simulated Time = 24.0000 hr _____ Node Number 2 25 = 12 Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.18611 0.25013 0.25396 Head (cm) = 1.94912E+03 5.20196E+02 4.71312E+02Water Flow (cm) = 1.13765E-01-2.57684E-03 4.02913E-03Plant Sink (cm)  $= 6.55760E-02 \ 0.00000E+00 \ 0.00000E+00$ PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 21.4445+ 0.2540+ 0.0000 - 0.1908- 0.5456- 0.0040 = 20.9581 Versus 20.9701 Mass Balance = -1.2051E-02 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.7707 cm, Actual = 0.1908 cm Transpiration: Potential = 2.2045 cm, Actual = 0.5456 cm RHMEAN = 55.9 %; TMEAN = 287.6 K; HDRY = 7.9775E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 114, Simulated Time = 24.0000 hr _____ 25 Node Number 2 12 === Node Number=21225Depth (cm)=5.0000030.7000091.50000Water (cm3/cm3)=0.148100.250130.25396 Head (cm) = 4.99480E+03 5.20196E+02 4.71312E+02Water Flow (cm) =-1.27283E-02-2.57684E-03 4.02913E-03 Plant Sink (cm) = 3.95183E-02 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 20.9701+ 0.0000+ 0.0000 - 0.1311- 0.3489- 0.0040 = 20.4861 Versus 20.4929 Mass Balance = -6.7959E-03 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.5244 cm, Actual = 0.1311 cm Transpiration: Potential = 1.5000 cm, Actual = 0.3489 cm RHMEAN = 62.6 %; TMEAN = 285.9 K; HDRY = 6.4209E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 115, Simulated Time = 24.0000 hr Node Number 2 12 25 === Depth (cm) = 5.00000 30.70000 Water (cm3/cm3) = 0.12830 0.25013 91.50000 0.25396 = 1.02570E+04 5.20196E+02 4.71312E+02Head (cm) Water Flow (cm) =-6.58045E-03-2.57684E-03 4.02913E-03 Plant Sink (cm) = 2.03572E-02 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE

20.4929+ 0.0000+ 0.0000 - 0.1208- 0.2568- 0.0040 = 20.1112 Versus 20.1161 Mass Balance = -4.8790E-03 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.6763 cm, Actual = 0.1208 cm Transpiration: Potential = 1.9345 cm, Actual = 0.2568 cm RHMEAN = 57.6 %; TMEAN = 290.1 K; HDRY = 7.5695E+05 cm; DAYUBC = ***** DAILY SUMMARY: Day = 116, Simulated Time = 24.0000 hr Node Number = 2 12 25 Depth (cm) = 5.00000 30.70000 Water (cm3/cm3) = 0.12162 0.25013 91.50000 0.25396 Head (cm) = 1.39985E+04 5.20196E+02 4.71312E+02Water Flow (cm) =-2.01562E-03-2.57684E-03 4.02913E-03 Plant Sink (cm)  $= 5.82130E-03 \ 0.00000E+00 \ 0.00000E+00$ PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 20.1161+ 0.0000+ 0.0000 - 0.0053- 0.1408- 0.0040 = 19.9660 Versus 19.9720 Mass Balance = -5.9776E-03 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.7755 cm, Actual = 0.0053 cm Transpiration: Potential = 2.2180 cm, Actual = 0.1408 cm RHMEAN = 57.4 %; TMEAN = 292.6 K; HDRY = 7.6090E+05 cm; DAYUBC = ***** DAILY SUMMARY: Day = 117, Simulated Time = 24.0000 hr ______ == Node Number 2 12 25 Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.11984 0.25013 0.25396 Depth (cm) Head (cm) = 1.53358E+04 5.20196E+02 4.71312E+02Water Flow (cm) =-7.63156E-04-2.57684E-03 4.02913E-03 Plant Sink (cm) = 5.66124E-04 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 19.9720+ 0.0000+ 0.0000 - 0.0058- 0.0774- 0.0040 = 19.8847 Versus 19.8932 Mass Balance = -8.4705E-03 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.8187 cm, Actual = 0.0058 cm Transpiration: Potential = 2.3417 cm, Actual = 0.0774 cm RHMEAN = 55.6 %; TMEAN = 293.2 K; HDRY = 8.0550E+05 cm; DAYUBC = ***** DAILY SUMMARY: Day = 118, Simulated Time = 24.0000 hr _____ == 2 Node Number 12 25 Note Handel=5.0000030.7000091.50000Depth (cm)=0.118940.250060.25391Water (cm3/cm3)=0.118940.250060.25391Head (cm) = 1.60832E+04 5.21079E+02 4.71831E+02

Water Flow (cm) =-4.24377E-04-2.58302E-03 4.02036E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 19.8932+ 0.0000+ 0.0000 - 0.0021- 0.0420- 0.0040 = 19.8450 Versus 19.8521 Mass Balance = -7.1850E-03 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.6457 cm, Actual = 0.0021 cm Transpiration: Potential = 1.8468 cm, Actual = 0.0420 cm RHMEAN = 65.0 %; TMEAN = 292.3 K; HDRY = 5.9033E+05 cm; DAYUBC = ***** DAILY SUMMARY: Day = 119, Simulated Time = 24.0000 hr ______ Node Number == 2 12 25 Depth (cm) = Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.11829 0.25006 0.25391 Head (cm) = 1.66599E+04 5.21079E+02 4.71831E+02Water Flow (cm) =-3.26322E-04-2.57839E-03 4.01714E-03Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 19.8521+ 0.0000+ 0.0000 - 0.0058- 0.0407- 0.0040 = 19.8017 Versus 19.8195 Mass Balance = -1.7818E-02 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.8094 cm, Actual = 0.0058 cm Transpiration: Potential = 2.3151 cm, Actual = 0.0407 cm RHMEAN = 61.1 %; TMEAN = 292.6 K; HDRY = 6.7459E+05 cm; DAYUBC = ***** DAILY SUMMARY: Day = 120, Simulated Time = 24.0000 hr 2 Node Number = 12 25 Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.11765 0.25006 0.25391 = 1.72477E+04 5.21079E+02 4.71831E+02 Head (cm) Water Flow (cm) =-2.92454E-04-2.57839E-03 4.01714E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 19.8195+ 0.0000+ 0.0000 - 0.0048- 0.0297- 0.0040 = 19.7809 Versus 19.7975 Mass Balance = -1.6592E-02 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.7380 cm, Actual = 0.0048 cm Transpiration: Potential = 2.1109 cm, Actual = 0.0297 cm RHMEAN = 60.9 %; TMEAN = 292.6 K; HDRY = 6.7937E+05 cm; DAYUBC = ***** DAILY SUMMARY: Day = 121, Simulated Time = 24.0000 hr _____ Node Number = 2 12 25

Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.11735 0.25002 0.25389 Head (cm) = 1.75459E+04 5.21617E+02 4.72151E+02Water Flow (cm) =-2.96403E-04-2.57170E-03 4.01116E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 19.7975+ 0.0000+ 0.0000 - 0.0028- 0.0316- 0.0040 = 19.7591 Versus 19.7727 Mass Balance = -1.3618E-02 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.9391 cm, Actual = 0.0028 cm Transpiration: Potential = 2.6861 cm, Actual = 0.0316 cm RHMEAN = 66.0 %; TMEAN = 295.7 K; HDRY = 5.7002E+05 cm; DAYUBC = ***** DAILY SUMMARY: Day = 122, Simulated Time = 24.0000 hr _____ Depth (cm) = Water ( 2 12 25 Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.11700 0.24997 0.25385 Head (cm) = 1.78897E+04 5.22358E+02 4.72609E+02Water Flow (cm) =-2.99690E-04-2.57311E-03 4.00182E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 19.7727+ 0.0000+ 0.0000 - 0.0020- 0.0253- 0.0040 = 19.7414 Versus 19.7519 Mass Balance = -1.0574E-02 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.8881 cm, Actual = 0.0020 cm Transpiration: Potential = 2.5403 cm, Actual = 0.0253 cm RHMEAN = 70.9 %; TMEAN = 296.2 K; HDRY = 4.7140E+05 cm; DAYUBC = ***** DAILY SUMMARY: Day = 123, Simulated Time = 24.0000 hr _____ Node Number = 2 12 Depth (cm) = 5.00000 30.70000 Water (cm3/cm3) = 0.11694 0.24996 25 91.50000 0.25385 Head (cm) = 1.79491E+04 5.22445E+02 4.72662E+02Water Flow (cm) =-3.04155E-04-2.57255E-03 3.99807E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 19.7519+ 0.0000+ 0.0000 - 0.0032- 0.0189- 0.0040 = 19.7258 Versus 19.7467 Mass Balance = -2.0845E-02 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.7102 cm, Actual = 0.0032 cm Transpiration: Potential = 2.0314 cm, Actual = 0.0189 cm RHMEAN = 71.6 %; TMEAN = 296.2 K; HDRY = 4.5775E+05 cm; DAYUBC = *****

DAILY SUMMARY: Day = 124, Simulated Time = 24.0000 hr _____ Node Number == 2 12 25 91.50000 Depth (cm) 5.00000 30.70000 = 0.11959 Water (cm3/cm3) 0.24996 0.25385 Head (cm) = 1.55386E+04 5.22445E+02 4.72662E+02=-1.41231E-04-2.57260E-03 3.99800E-03 Water Flow (cm) Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 19.7467+ 0.2540+ 0.0000 - 0.1443- 0.0155- 0.0040 = 19.8369 Versus 19.8567 Mass Balance = -1.9766E-02 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.5830 cm, Actual = 0.1443 cm Transpiration: Potential = 1.6675 cm, Actual = 0.0155 cm RHMEAN = 73.4 %; TMEAN = 295.4 K; HDRY = 4.2426E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 125, Simulated Time = 24.0000 hr _____ Node Number 2 25 -----12 Depth (cm) = 5.00000 Water (cm3/cm3) = 0.16521 30.70000 91.50000 0.24996 0.25385 Head (cm) = 3.11356E+03 5.22445E+02 4.72662E+02= 2.67933E-02-2.57260E-03 3.99800E-03 Water Flow (cm) Plant Sink (cm) = 4.21773E-02 0.00000E+00 0.00000E+00 INFIL RUNOFF PRESTOR EVAPO TRANS DRAIN NEWSTOR STORAGE 19.8567 + 0.6858 + 0.0000 - 0.1240 - 0.2103 - 0.0040 = 20.2041 Versus 20.2242 Mass Balance = -2.0149E-02 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.5012 cm, Actual = 0.1240 cm Transpiration: Potential = 1.4336 cm, Actual = 0.2103 cm RHMEAN = 79.8 %; TMEAN = 294.3 K; HDRY = 3.0910E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 126, Simulated Time = 24.0000 hr Node Number = 2 12 25 Depth (cm) 5.00000 30.70000 91.50000 = = 0.14044 Water (cm3/cm3) 0.24996 0.25385 Head (cm) = 6.41591E+03 5.22445E+02 4.72662E+02Water Flow (cm) = 9.74640E - 03 - 2.57260E - 03 3.99800E - 03Plant Sink (cm) = 2.43323E-020.00000E+000.00000E+00PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 20.2242+ 0.0000+ 0.0000 - 0.1051- 0.1323- 0.0040 = 19.9829 Versus 20.0021 Mass Balance = -1.9144E-02 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.4203 cm, Actual = 0.1051 cm Transpiration: Potential = 1.2020 cm, Actual = 0.1323 cm RHMEAN = 81.0 %; TMEAN = 293.4 K; HDRY = 2.8831E+05 cm; DAYUBC = 0

DAILY SUMMARY: Day = 127, Simulated Time = 24.0000 hr Node Number = 2 12 25  $\begin{array}{rcl} & - & 5.00000 & 30.70000 & 91.50000 \\ \text{Water (cm3/cm3)} & = & 0.12552 & 0.24996 & 0.25385 \\ \text{Head (cm)} & - & 1.160707161 \\ \end{array}$ Head (cm) = 1.16078E+04 5.22445E+02 4.72662E+02Water Flow (cm) = 1.53516E-03-2.57260E-03 3.99800E-03Plant Sink (cm) = 1.39203E-02 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 20.0021+ 0.0000+ 0.0000 - 0.1268- 0.0935- 0.0040 = 19.7778 Versus 19.7985 Mass Balance = -2.0748E-02 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.6688 cm, Actual = 0.1268 cm Transpiration: Potential = 1.9130 cm, Actual = 0.0935 cm RHMEAN = 68.6 %; TMEAN = 290.9 K; HDRY = 5.1573E+05 cm; DAYUBC = 96890 DAILY SUMMARY: Day = 128, Simulated Time = 24.0000 hr 2 Node Number = 12 25 Depth (cm) = 5.00000 30.70000 Water (cm3/cm3) = 0.12139 0.24996 91.50000 0.25385 Head (cm) = 1.41602E+04 5.22445E+02 4.72662E+02Water Flow (cm) = 7.40822E-05-2.57260E-03 3.99800E-03 Plant Sink (cm) = 3.22607E-03 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 19.7985+ 0.0000+ 0.0000 - 0.0041- 0.0378- 0.0040 = 19.7526 Versus 19.7739 Mass Balance = -2.1305E-02 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.6159 cm, Actual = 0.0041 cm Transpiration: Potential = 1.7616 cm, Actual = 0.0378 cm RHMEAN = 68.3 %; TMEAN = 290.4 K; HDRY = 5.2286E+05 cm; DAYUBC = ***** DAILY SUMMARY: Day = 129, Simulated Time = 24.0000 hr = Node Number 2 12 25 Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.120060.24993 0.25382 = 1.51585E+04 5.22886E+02 4.72942E+02Head (cm) Water Flow (cm) =-9.17463E-05-2.57028E-03 3.99274E-03 Plant Sink (cm) = 5.13857E-04 0.00000E+00 0.00000E+00 INFIL RUNOFF EVAPO TRANS PRESTOR DRAIN NEWSTOR STORAGE 19.7739+ 0.0000+ 0.0000 - 0.0026- 0.0268- 0.0040 = 19.7406 Versus 19.7533 Mass Balance = -1.2690E-02 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.7665 cm, Actual = 0.0026 cm

Transpiration: Potential = 2.1923 cm, Actual = 0.0268 cm RHMEAN = 71.2 %; TMEAN = 294.0 K; HDRY = 4.6601E+05 cm; DAYUBC = ***** DAILY SUMMARY: Day = 130, Simulated Time = 24.0000 hr _____ Node Number Depth (cm) = 2 12 25 Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.12051 0.24993 0.25382 Head (cm) = 1.48110E+04 5.22886E+02 4.72942E+02Water Flow (cm) =-5.55997E-05-2.56954E-03 3.99175E-03 Plant Sink (cm) =  $2.73822E-04 \ 0.00000E+00 \ 0.00000E+00$ PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 19.7533+ 0.1778+ 0.0000 - 0.1223- 0.0157- 0.0040 = 19.7891 Versus 19.8090 Mass Balance = -1.9920E-02 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.4942 cm, Actual = 0.1223 cm Transpiration: Potential = 1.4137 cm, Actual = 0.0157 cm RHMEAN = 72.5 %; TMEAN = 295.7 K; HDRY = 4.4154E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 131, Simulated Time = 24.0000 hr _____ Node Number = 2 12 25 Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.21189 0.24993 0.25382 = 1.21127E+03 5.22886E+02 4.72942E+02 Head (cm) Water Flow (cm) = 2.37992E-01-2.56954E-03 3.99175E-03Plant Sink (cm) = 2.47565E-02 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 19.8090+ 1.1105+ 0.2103 - 0.0720- 0.1587- 0.0040 = 20.6848 Versus 20.7044 Mass Balance = -1.9600E-02 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.2939 cm, Actual = 0.0720 cm Transpiration: Potential = 0.8408 cm, Actual = 0.1587 cm RHMEAN = 80.4 %; TMEAN = 294.5 K; HDRY = 2.9934E+05 cm; DAYUBC = 4769 DAILY SUMMARY: Day = 132, Simulated Time = 24.0000 hr 12 Node Number Depth (cm) 25 = 2 Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.14924 0.24993 0.25382 Head (cm) = 4.82352E+03 5.22886E+02 4.72942E+02Water Flow (cm) = 4.47160E-02-2.56954E-03 3.99175E-03Plant Sink (cm) = 5.61017E-02 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 20.7044+ 0.0000+ 0.0000 - 0.1756- 0.3978- 0.0040 = 20.1269 Versus 20.1476

Mass Balance = -2.0615E-02 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.7025 cm, Actual = 0.1756 cm Transpiration: Potential = 2.0095 cm, Actual = 0.3978 cm RHMEAN = 72.2 %; TMEAN = 294.3 K; HDRY = 4.4650E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 133, Simulated Time = 24.0000 hr ______ Node Number 2 12 ----25 Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.12918 0.24993 0.25382 Head (cm) = 9.87742E+03 5.22886E+02 4.72942E+02Water Flow (cm) =-4.13585E-04-2.56954E-03 3.99175E-03 Plant Sink (cm)  $= 1.95131E-02 \ 0.00000E+00 \ 0.00000E+00$ PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 20.1476+ 0.0000+ 0.0000 - 0.1387- 0.1628- 0.0040 = 19.8420 Versus 19.8620 Mass Balance = -1.9978E-02 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.6193 cm, Actual = 0.1387 cm Transpiration: Potential = 1.7713 cm, Actual = 0.1628 cm RHMEAN = 78.8 %; TMEAN = 294.3 K; HDRY = 3.2732E+05 cm; DAYUBC = 77991 DAILY SUMMARY: Day = 134, Simulated Time = 24.0000 hr _____ = 12 Node Number 2 25 Depth (cm) = Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.12166 0.24993 0.25382 Head (cm) = 1.39708E+04 5.22886E+02 4.72942E+02Water Flow (cm) =-6.34368E-04-2.56954E-03 3.99175E-03 Plant Sink (cm) = 6.83850E-03 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 19.8620+ 0.0000+ 0.0000 - 0.0076- 0.0900- 0.0040 = 19.7604 Versus 19.7793 Mass Balance = -1.8930E-02 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.8694 cm, Actual = 0.0076 cm Transpiration: Potential = 2.4868 cm, Actual = 0.0900 cm RHMEAN = 70.4 %; TMEAN = 297.3 K; HDRY = 4.8077E+05 cm; DAYUBC = ***** DAILY SUMMARY: Day = 135, Simulated Time = 24.0000 hr _____ 2 12 Node Number == 25 Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.12001 0.24993 0.25382 Head (cm) = 1.51974E+04 5.22886E+02 4.72942E+02Water Flow (cm) =-2.88917E-04-2.56954E-03 3.99175E-03 Plant Sink (cm) = 7.95896E-04 0.00000E+00 0.00000E+00

PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 19.7793+ 0.0000+ 0.0000 - 0.0060- 0.0443- 0.0040 = 19.7250 Versus 19.7453 Mass Balance = -2.0281E-02 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 1.0597 cm, Actual = 0.0060 cm Transpiration: Potential = 3.0312 cm, Actual = 0.0443 cm RHMEAN = 65.2 %; TMEAN = 297.9 K; HDRY = 5.8719E+05 cm; DAYUBC = ***** DAILY SUMMARY: Day = 136, Simulated Time = 24.0000 hr ______ Node Number = 2 12 25 Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.11924 0.24981 0.25374 Head (cm) = 1.58271E+04 5.24449E+02 4.73962E+02Water Flow (cm) =-1.82879E-04-2.56645E-03 3.97952E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 INFIL RUNOFF PRESTOR EVAPO TRANS DRAIN NEWSTOR STORAGE 19.7453+ 0.0000+ 0.0000 - 0.0003- 0.0184- 0.0040 = 19.7227 Versus 19.7241 Mass Balance = -1.4210E-03 cm; Time step attempts =22526 and successes =22526 Evaporation: Potential = 0.6446 cm, Actual = 0.0003 cm Transpiration: Potential = 1.8436 cm, Actual = 0.0184 cm RHMEAN = 74.4 %; TMEAN = 298.2 K; HDRY = 4.0602E+05 cm; DAYUBC = 19933 DAILY SUMMARY: Day = 137, Simulated Time = 24.0000 hr ____ Node Number Depth (cm) 200 2 12 25 Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.18809 0.24981 0.25374 = 1.87301E+03 5.24449E+02 4.73962E+02 Head (cm) Water Flow (cm) = 9.99406E - 02 - 2.56081E - 03 3.96833E - 03Plant Sink (cm) = 4.45870E-02 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 19.7241+ 1.0037+ 0.1646 - 0.1297- 0.2427- 0.0040 = 20.3515 Versus 20.3719 Mass Balance = -2.0323E-02 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.5294 cm, Actual = 0.1297 cm Transpiration: Potential = 1.5142 cm, Actual = 0.2427 cm RHMEAN = 83.4 %; TMEAN = 297.0 K; HDRY = 2.4906E+05 cm; DAYUBC = 2048 _____ DAILY SUMMARY: Day = 138, Simulated Time = 24.0000 hr ______ Node Number = 2 12 25 Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.23440 0.24981 0.25374
Head (cm) = 8.46479E+02 5.24449E+02 4.73962E+02Water Flow (cm) = 9.96541E-01-2.56081E-03 3.96833E-03Plant Sink (cm) = 4.91252E-02 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 20.3719+ 1.6745+ 0.1542 - 0.1429- 0.4059- 0.0040 = 21.4936 Versus 21.5097 Mass Balance = -1.6029E-02 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.5833 cm, Actual = 0.1429 cm Transpiration: Potential = 1.6683 cm, Actual = 0.4059 cm RHMEAN = 77.3 %; TMEAN = 296.2 K; HDRY = 3.5370E+05 cm; DAYUBC = 6498 DAILY SUMMARY: Day = 139, Simulated Time = 24.0000 hr -----Node Number = Depth (cm) = 2 12 25 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.20294 0.24981 0.25374= 1.41516E+03 5.24449E+02 4.73962E+02Head (cm) Water Flow (cm) = 4.89808E - 02 - 2.56081E - 03 3.96833E - 03Plant Sink (cm) = 3.41507E-02 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 21.5097+ 0.0762+ 0.0000 - 0.0993- 0.2841- 0.0040 = 21.1984 Versus 21.2051 Mass Balance = -6.6662E-03 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.4014 cm, Actual = 0.0993 cm Transpiration: Potential = 1.1481 cm, Actual = 0.2841 cm RHMEAN = 84.8 %; TMEAN = 294.8 K; HDRY = 2.2653E+05 cm; DAYUBC = ______ DAILY SUMMARY: Day = 140, Simulated Time = 24.0000 hr Node Number = 2 12 25 Depth (cm) Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.24755 0.24981 0.25374 Head (cm) = 6.97510E+02 5.24449E+02 4.73962E+02Water Flow (cm) = 6.82931E-01-2.56081E-03 3.96833E-03 Plant Sink (cm) = 3.27326E-02 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 21.2051+ 1.2421+ 0.0279 - 0.0952- 0.2723- 0.0040 = 22.0756 Versus 22.0800 Mass Balance = -4.4250E-03 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.3886 cm, Actual = 0.0952 cm Transpiration: Potential = 1.1116 cm, Actual = 0.2723 cmRHMEAN = 83.2 %; TMEAN = 295.9 K; HDRY = 2.5128E+05 cm; DAYUBC = 2174 

DAILY SUMMARY: Day = 141, Simulated Time = 24.0000 hr

Node Number = 2 12 25 Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.22673 0.24981 0.25374 Head (cm) = 9.52182E+02 5.24449E+02 4.73962E+02Water Flow (cm) = 1.34366E-01-2.56081E-03 3.96833E-03 Plant Sink (cm) = 3.64481E-02 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 22.0800+ 0.2540+ 0.0000 - 0.1060- 0.3033- 0.0040 = 21.9208 Versus 21.9273 Mass Balance = -6.5327E-03 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.4284 cm, Actual = 0.1060 cm Transpiration: Potential = 1.2253 cm, Actual = 0.3033 cm RHMEAN = 73.2 %; TMEAN = 287.9 K; HDRY = 4.2667E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 142, Simulated Time = 24.0000 hr ______ Node Number ..... 2 12 25 Depth (cm) Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.15910 0.24981 0.25374 91.50000 Head (cm) = 3.64387E+03 5.24449E+02 4.73962E+02Water Flow (cm) =-2.86885E-02-2.56081E-03 3.96833E-03 Plant Sink (cm) =  $7.20058E-02 \ 0.00000E+00 \ 0.00000E+00$ PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 21.9273+ 0.0000+ 0.0000 - 0.2125- 0.6030- 0.0040 = 21.1078 Versus 21.1141 Mass Balance = -6.2122E-03 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.8498 cm, Actual = 0.2125 cm Transpiration: Potential = 2.4307 cm, Actual = 0.6030 cm RHMEAN = 58.4 %; TMEAN = 289.3 K; HDRY = 7.3747E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 143, Simulated Time = 24.0000 hr _____ Node Number 2 12 25 == Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.13033 0.24981 0.25374 Head (cm) = 9.41076E+03 5.24449E+02 4.73962E+02Water Flow (cm) =-2.02506E-02-2.56081E-03 3.96833E-03 Plant Sink (cm) = 3.30614E-02 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 21.1141+ 0.0508+ 0.0000 - 0.1973- 0.3969- 0.0040 = 20.5666 Versus 20.5740 Mass Balance = -7.3643E-03 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.8199 cm, Actual = 0.1973 cm Transpiration: Potential = 2.3452 cm, Actual = 0.3969 cm RHMEAN = 59.4 %; TMEAN = 291.8 K; HDRY = 7.1289E+05 cm; DAYUBC = 30490 *****

DAILY SUMMARY: Day = 144, Simulated Time = 24.0000 hr _____ Node Number ----2 12 25 Depth (cm) === 5.00000 30.70000 91.50000  $= 0.12247 \quad 0.24981 \quad 0.25374$ Water (cm3/cm3) Head (cm) = 1.34166E+04 5.24449E+02 4.73962E+02Water Flow (cm) =-4.66654E-03-2.56081E-03 3.96833E-03 Plant Sink (cm) = 7.65474E-03 0.00000E+00 0.00000E+00PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 20.5740+ 0.0000+ 0.0000 - 0.0063- 0.2002- 0.0040 = 20.3636 Versus 20.3687 Mass Balance = -5.0926E-03 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.7553 cm, Actual = 0.0063 cm Transpiration: Potential = 2.1603 cm, Actual = 0.2002 cm RHMEAN = 55.8 %; TMEAN = 288.2 K; HDRY = 7.9910E+05 cm; DAYUBC = **** DAILY SUMMARY: Day = 145, Simulated Time = 24.0000 hr _____ 12 Node Number -2 25 Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.12056 0.24978 0.25371 = 1.47664E+04 5.24861E+02 4.74328E+02Head (cm) Water Flow (cm) =-1.78577E-03-2.48834E-03 3.96185E-03 Plant Sink (cm) = 1.33994E-03 0.00000E+00 0.00000E+00 INFIL RUNOFF PRESTOR EVAPO TRANS DRAIN NEWSTOR STORAGE 20.3687+ 0.0000+ 0.0000 - 0.0030- 0.1090- 0.0040 = 20.2528 Versus 20.2590 Mass Balance = -6.2561E-03 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.6513 cm, Actual = 0.0030 cm Transpiration: Potential = 1.8629 cm, Actual = 0.1090 cm RHMEAN = 62.8 %; TMEAN = 288.7 K; HDRY = 6.3737E+05 cm; DAYUBC = ***** DAILY SUMMARY: Day = 146, Simulated Time = 24.0000 hr _____ Node Number = Depth (cm) = 12 2 25 30.70000 5.00000 91.50000 = 0.11988 0.24973 0.25366 Water (cm3/cm3) = 1.52980E+04 5.25570E+02 4.74877E+02Head (cm) Water Flow (cm) =-9.42377E-04-2.46623E-03 3.95111E-03 Plant Sink (cm) = 2.66243E-05 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 20.2590+ 0.0000+ 0.0000 - 0.0019- 0.0802- 0.0040 = 20.1730 Versus 20.1793 Mass Balance = -6.3400E-03 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.6410 cm, Actual = 0.0019 cm Transpiration: Potential = 1.8334 cm, Actual = 0.0802 cm RHMEAN = 69.4 %; TMEAN = 291.2 K; HDRY = 5.0032E+05 cm; DAYUBC = *****

DAILY SUMMARY: Day = 147, Simulated Time = 24.0000 hr ______ Node Number == 2 12 25 Depth (cm) Depth (cm) = 5.00000 30.70000 Water (cm3/cm3) = 0.25232 0.24973 91.50000 0.24973 0.25366 Head (cm) = 6.51543E+02 5.25570E+02 4.74877E+02Water Flow (cm) = 1.36870E+00-2.46300E-03 3.94785E-03Plant Sink (cm) = 5.00114E-02 0.00000E+00 0.00000E+00 INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR PRESTOR STORAGE 20.1793+ 2.7487+ 0.9851 - 0.1455- 0.4161- 0.0039 = 22.3625 Versus 22.3717 Mass Balance = -9.2068E-03 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.6062 cm, Actual = 0.1455 cmTranspiration: Potential = 1.7338 cm, Actual = 0.4161 cm RHMEAN = 73.7 %; TMEAN = 295.7 K; HDRY = 4.1877E+05 cm; DAYUBC = 23339 DAILY SUMMARY: Day = 148, Simulated Time = 24.0000 hr 2 Node Number === 12 25 Depth (cm) = 5.00000 Water (cm3/cm3) = 0.18933 = 30.70000 91.50000 0.24973 0.25366 Head (cm) = 1.82721E+03 5.25570E+02 4.74877E+02Water Flow (cm) =-2.10183E-02-2.46300E-03 3.94785E-03 Plant Sink (cm) = 6.58207E-02 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 22.3717+ 0.0000+ 0.0000 - 0.1915- 0.5477- 0.0039 = 21.6286 Versus 21.6351 Mass Balance = -6.4106E-03 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.7659 cm, Actual = 0.1915 cm Transpiration: Potential = 2.1906 cm, Actual = 0.5477 cm RHMEAN = 74.2 %; TMEAN = 295.7 K; HDRY = 4.0968E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 149, Simulated Time = 24.0000 hr _____ Node Number 12 25 -2 Depth (cm) Depth (cm) = 5.00000 Water (cm3/cm3) = 0.30066 30.70000 91.50000 30.7000091.500000.249740.25366 Head (cm) = 3.35008E+02 5.25408E+02 4.74877E+02Water Flow (cm) = 1.73955E+00-2.46217E-03 3.94785E-03Plant Sink (cm) = 6.06321E-03 0.00000E+00 0.00000E+00 INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR PRESTOR STORAGE 21.6351+ 2.5885+ 0.4341 - 0.0176- 0.0504- 0.0039 = 24.1515 Versus 24.1572

Mass Balance = -5.7068E-03 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.0735 cm, Actual = 0.0176 cm Transpiration: Potential = 0.2102 cm, Actual = 0.0504 cm RHMEAN = 91.3 %; TMEAN = 293.7 K; HDRY = 1.2484E+05 cm; DAYUBC = 17203 DAILY SUMMARY: Day = 150, Simulated Time = 24.0000 hr 2 12 Node Number = 2.5 Depth (cm) = 5.00000 30.70000 91.50000Water (cm3/cm3) = 0.35535 0.25410 0.25357Head (cm)  $= 1.39922E+02 \ 4.69585E+02 \ 4.76007E+02$ Water Flow (cm) =  $1.89118E+00 \ 1.30483E-02 \ 3.93578E-03$ Plant Sink (cm)  $= 1.87136E-02 \ 0.00000E+00 \ 0.00000E+00$ PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 24.1572+ 2.4077+ 1.3261 - 0.0544- 0.1557- 0.0039 = 26.3508 Versus 26.3511 Mass Balance = -3.0136E-04 cm; Time step attempts = 5997 and successes = 5997 Evaporation: Potential = 0.2268 cm, Actual = 0.0544 cm Transpiration: Potential = 0.6488 cm, Actual = 0.1557 cm RHMEAN = 88.9 %; TMEAN = 293.4 K; HDRY = 1.6116E+05 cm; DAYUBC = 767 DAILY SUMMARY: Day = 151, Simulated Time = 24.0000 hr _____ = Node Number 2 12 25 Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.35094 0.25572 0.25347 Head (cm) = 1.52666E+02 4.50437E+02 4.77216E+02Water Flow (cm) =-4.45114E-02 2.26787E-02 3.90905E-03 Plant Sink (cm) =  $1.01539E-02 \ 0.00000E+00 \ 0.00000E+00$ PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 26.3511+ 0.0000+ 0.0000 - 0.0295- 0.0845- 0.0039 = 26.2332 Versus 26.2332 Mass Balance = 1.9073E-06 cm; Time step attempts = 96 and successes = 96 Evaporation: Potential = 0.1181 cm, Actual = 0.0295 cmTranspiration: Potential = 0.3379 cm, Actual = 0.0845 cm RHMEAN = 86.8 %; TMEAN = 290.1 K; HDRY = 1.9340E+05 cm; DAYUBC = 0 _____ DAILY SUMMARY: Day = 152, Simulated Time = 24.0000 hr _____ 2 = 12 Node Number 25 Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.32718 0.25595 0.25337 Head (cm) = 2.28365E+02 4.47775E+02 4.78422E+02Water Flow (cm) =-2.48603E-01 1.97817E-02 3.88223E-03

Plant Sink (cm) = 5.61133E-02 0.00000E+00 0.00000E+00

PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 26.2332+ 0.0000+ 0.0000 - 0.1632- 0.4669- 0.0039 = 25.5992 Versus 25.5992 Mass Balance = -9.5367E-06 cm; Time step attempts = 96 and successes = 96 Evaporation: Potential = 0.6529 cm, Actual = 0.1632 cmTranspiration: Potential = 1.8675 cm, Actual = 0.4669 cm RHMEAN = 79.2 %; TMEAN = 291.8 K; HDRY = 3.2004E+05 cm; DAYUBC = Ω DAILY SUMMARY: Day = 153, Simulated Time = 24.0000 hr ______ Node Number = 2 12 25 Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.28686 0.25535 0.25328 Head (cm) = 4.04762E+02 4.54742E+02 4.79544E+02Water Flow (cm) =-3.25330E-01 1.23486E-02 3.85645E-03 Plant Sink (cm) = 8.29031E-02 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 25.5992+ 0.0000+ 0.0000 - 0.2412- 0.6898- 0.0039 = 24.6644 Versus 24.6645 Mass Balance = -4.9591E-05 cm; Time step attempts = 128 and successes = 128Evaporation: Potential = 0.9646 cm, Actual = 0.2412 cmTranspiration: Potential = 2.7591 cm, Actual = 0.6898 cm RHMEAN = 59.7 %; TMEAN = 295.9 K; HDRY = 7.0789E+05 cm; DAYUBC = Ο DAILY SUMMARY: Day = 154, Simulated Time = 24.0000 hr Node Number = Depth (cm) = 2 12 25 Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.25881 0.25455 0.25321 Head (cm) = 5.94646E+02 4.64155E+02 4.80448E+02Water Flow (cm) =-2.62777E-01 6.41092E-03 3.83416E-03 Plant Sink (cm) =  $6.27652E-02 \ 0.00000E+00 \ 0.00000E+00$ PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 24.6645+ 0.0000+ 0.0000 - 0.1826- 0.5222- 0.0038 = 23.9558 Versus 23.9559 Mass Balance = -1.7166E-05 cm; Time step attempts = 176 and successes = 176Evaporation: Potential = 0.7303 cm, Actual = 0.1826 cm Transpiration: Potential = 2.0889 cm, Actual = 0.5222 cm RHMEAN = 73.7 %; TMEAN = 298.4 K; HDRY = 4.1847E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 155, Simulated Time = 24.0000 hr ______ Node Number=21225Depth (cm)=5.0000030.7000091.50000Water (cm3/cm3)=0.207990.253860.25316

= 1.29491E+03 4.72474E+02 4.81029E+02 Head (cm) Water Flow (cm) =-2.19580E-01 3.13161E-03 3.81797E-03 Plant Sink (cm) = 8.28276E-02 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 23.9559+ 0.0000+ 0.0000 - 0.2409- 0.6892- 0.0038 = 23.0219 Versus 23.0219 Mass Balance = -7.6294E-06 cm; Time step attempts = 629 and successes = 629 Evaporation: Potential = 0.9638 cm, Actual = 0.2409 cm Transpiration: Potential = 2.7566 cm, Actual = 0.6892 cm RHMEAN = 71.2 %; TMEAN = 301.2 K; HDRY = 4.6613E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 156, Simulated Time = 24.0000 hr -----Node Number = Depth (cm) = 2 12 25 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.16774 0.25326 0.25315= 2.92629E+03 4.79869E+02 4.81236E+02Head (cm) Water Flow (cm) =-1.63578E-01 1.10916E-03 3.80953E-03 Plant Sink (cm) = 6.63390E-02 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 23.0219+ 0.0000+ 0.0000 - 0.1932- 0.5523- 0.0038 = 22.2726 Versus 22.2726 Mass Balance = 7.4387E-05 cm; Time step attempts = 3916 and successes = 3916 Evaporation: Potential = 0.7729 cm, Actual = 0.1932 cm Transpiration: Potential = 2.2107 cm, Actual = 0.5523 cm RHMEAN = 74.4 %; TMEAN = 299.8 K; HDRY = 4.0507E+05 cm; DAYUBC = DAILY SUMMARY: Day = 157, Simulated Time = 24.0000 hr _____ Node Number = 2 12 25 Depth (cm) Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.13954 0.25306 0.25314 = 6.62147E+03 4.82237E+02 4.81258E+02 Head (cm) Water Flow (cm) =-8.99954E-02 6.74206E-05 3.80743E-03 Plant Sink (cm) = 4.60047E-02 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 22.2726+ 0.0000+ 0.0000 - 0.1061- 0.4639- 0.0038 = 21.6988 Versus 21.7011 Mass Balance = -2.3193E-03 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.7824 cm, Actual = 0.1061 cm Transpiration: Potential = 2.2379 cm, Actual = 0.4639 cm RHMEAN = 72.6 %; TMEAN = 298.7 K; HDRY = 4.3979E+05 cm; DAYUBC = ***** 

DAILY SUMMARY: Day = 158, Simulated Time = 24.0000 hr

Node Number = 2 12 25 Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.12764 0.25291 0.25314 Head (cm)  $= 1.05576E+04 \ 4.84097E+02 \ 4.81266E+02$ Water Flow (cm) =-3.83102E-02-5.10868E-04 3.80728E-03 Plant Sink (cm) = 1.94121E-02 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 21.7011+ 0.0000+ 0.0000 - 0.0028- 0.3500- 0.0038 = 21.3444 Versus 21.3482 Mass Balance = -3.7651E-03 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.8019 cm, Actual = 0.0028 cm Transpiration: Potential = 2.2936 cm, Actual = 0.3500 cm RHMEAN = 74.2 %; TMEAN = 298.7 K; HDRY = 4.0867E+05 cm; DAYUBC = ***** DAILY SUMMARY: Day = 159, Simulated Time = 24.0000 hr _____ ----Node Number 2 12 25 Depth (cm) = Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.12489 0.25287 0.25314 Head (cm)  $= 1.19516E+04 \ 4.84599E+02 \ 4.81266E+02$ Water Flow (cm) =-1.64125E-02-9.83512E-04 3.80726E-03 Plant Sink (cm) = 7.56335E-03 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 21.3482+ 0.2032+ 0.0000 - 0.1564- 0.2262- 0.0038 = 21.1650 Versus 21.1711 Mass Balance = -6.1035E-03 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.6318 cm, Actual = 0.1564 cm Transpiration: Potential = 1.8070 cm, Actual = 0.2262 cm RHMEAN = 78.7 %; TMEAN = 298.7 K; HDRY = 3.2909E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 160, Simulated Time = 24.0000 hr _____ 2 Node Number 12 = 25 = 5.00000 30.70000 91.50000 Depth (cm) Water (cm3/cm3) =0.12227 0.25286 0.25314 Head (cm) = 1.35488E+04 4.84805E+02 4.81266E+02Water Flow (cm) =-8.76024E-03-1.30789E-03 3.80726E-03 Plant Sink (cm) = 4.15419E-03 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 21.1711+ 0.0000+ 0.0000 - 0.0426- 0.2000- 0.0038 = 20.9248 Versus 20.9326 Mass Balance = -7.7782E-03 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.6754 cm, Actual = 0.0426 cm Transpiration: Potential = 1.9318 cm, Actual = 0.2000 cm RHMEAN = 76.5 %; TMEAN = 299.0 K; HDRY = 3.6799E+05 cm; DAYUBC = ***** 

DAILY SUMMARY: Day = 161, Simulated Time = 24.0000 hr _____ = 2 Node Number 12 25 Depth (cm) = 5.00000 Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.12095 0.25281 0.25314 = 1.44794E+04 4.85460E+02 4.81266E+02 Head (cm) Water Flow (cm) =-4.41842E-03-1.32441E-03 3.80726E-03 Plant Sink (cm) =  $1.73491E-03 \ 0.00000E+00 \ 0.00000E+00$ PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 20.9326+ 0.0000+ 0.0000 - 0.0027- 0.1756- 0.0038 = 20.7505 Versus 20.7570 Mass Balance = -6.4602E-03 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.7337 cm, Actual = 0.0027 cm Transpiration: Potential = 2.0987 cm, Actual = 0.1756 cm RHMEAN = 76.9 %; TMEAN = 299.0 K; HDRY = 3.5917E+05 cm; DAYUBC = ***** DAILY SUMMARY: Day = 162, Simulated Time = 24.0000 hr 2 Node Number 12 = 25 Depth (cm) = 5.00000 30.70000 Water (cm3/cm3) = 0.12018 0.25281 91.50000 0.25314 = 1.50641E+04 4.85461E+02 4.81266E+02 Head (cm) Water Flow (cm) =-1.98768E-03-1.30320E-03 3.80726E-03 Plant Sink (cm) = 5.62133E-04 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 20.7570+ 0.0000+ 0.0000 - 0.0060- 0.1647- 0.0038 = 20.5825 Versus 20.5907 Mass Balance = -8.1463E-03 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.9175 cm, Actual = 0.0060 cm Transpiration: Potential = 2.6245 cm, Actual = 0.1647 cm RHMEAN = 71.5 %; TMEAN = 299.5 K; HDRY = 4.5902E+05 cm; DAYUBC = ***** DAILY SUMMARY: Day = 163, Simulated Time = 24.0000 hr _____ Node Number = 2 12 25 Depth (cm) = 5.00000 30.70000 91.50000 Depth (cm)=5.0000030./000091.50000Water (cm3/cm3)=0.119520.252800.25314 Head (cm) = 1.55905E+04 4.85467E+02 4.81266E+02Water Flow (cm) =-7.93289E-04-1.28602E-03 3.80726E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 20.5907+ 0.0000+ 0.0000 - 0.0050- 0.1448- 0.0038 = 20.4370 Versus 20.4453 Mass Balance = -8.3065E-03 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 1.1594 cm, Actual = 0.0050 cm Transpiration: Potential = 3.3163 cm, Actual = 0.1448 cm

RHMEAN = 68.5 %; TMEAN = 300.1 K; HDRY = 5.1897E+05 cm; DAYUBC = ****

DAILY SUMMARY: Day = 164, Simulated Time = 24.0000 hr -----Node Number = 2 12 25 Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.11879 0.25254 0.25314 Head (cm)  $= 1.62092E+04 \ 4.88743E+02 \ 4.81259E+02$ Water Flow (cm) =-3.78227E-04-1.68085E-03 3.80731E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 20.4453+ 0.0000+ 0.0000 - 0.0021- 0.0994- 0.0038 = 20.3400 Versus 20.3436 Mass Balance = -3.5133E-03 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 1.0579 cm, Actual = 0.0021 cm Transpiration: Potential = 3.0258 cm, Actual = 0.0994 cm RHMEAN = 72.3 %; TMEAN = 301.2 K; HDRY = 4.4425E+05 cm; DAYUBC = ***** DAILY SUMMARY: Day = 165, Simulated Time = 24.0000 hr _____ 2 12 Node Number = 25 Depth (cm) = 5.00000 30.70000 Water (cm3/cm3) = 0.21900 0.25254 91.50000 0.25314 Head (cm) = 1.07674E+03 4.88822E+02 4.81259E+02Water Flow (cm) = 4.68646E-01-1.81099E-03 3.80735E-03Plant Sink (cm) = 3.69065E-02 0.00000E+00 0.00000E+00INFIL RUNOFF DRAIN NEWSTOR PRESTOR EVAPO TRANS STORAGE 20.3436+ 1.5339+ 0.7266 - 0.1074- 0.2889- 0.0038 = 21.4774 Versus 21.4848 Mass Balance = -7.4234E-03 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.4427 cm, Actual = 0.1074 cm Transpiration: Potential = 1.2663 cm, Actual = 0.2889 cm RHMEAN = 81.4 %; TMEAN = 298.4 K; HDRY = 2.8139E+05 cm; DAYUBC = 12046 DAILY SUMMARY: Day = 166, Simulated Time = 24.0000 hr _____ Node Number = 2 12 25 Node Number = Depth (cm) = Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.15363 0.25253 0.25314 Head (cm) = 4.23895E+03 4.88904E+02 4.81259E+02Water Flow (cm) = 3.31411E-02-1.88007E-03 3.80735E-03Plant Sink (cm) = 6.36272E-02 0.00000E+00 0.00000E+00PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 21.4848+ 0.0254+ 0.0000 - 0.1920- 0.5356- 0.0038 = 20.7788 Versus 20.7857

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Mass Balance = -6.8569E-03 cm; Time step attempts =***** and successes =*****
  Evaporation: Potential = 0.7758 cm, Actual = 0.1920 cm
Transpiration: Potential = 2.2190 cm, Actual = 0.5356 cm
RHMEAN = 64.5 %; TMEAN = 297.0 K; HDRY = 6.0028E+05 cm; DAYUBC =
                                                              Ω
DAILY SUMMARY: Day = 167, Simulated Time = 24.0000 hr
-----
                     2
Node Number
              =
                               12
                                         25
Depth (cm) = 5.00000 30.70000
Water (cm3/cm3) = 0.12537 0.25252
                                       91.50000
                             0.25252
                                        0.25314
Head (cm) = 1.16868E+04 4.88977E+02 4.81259E+02
Water Flow (cm) =-3.61881E-03-1.94442E-03 3.80735E-03
Plant Sink (cm) = 2.83333E-02 0.00000E+00 0.00000E+00
                       EVAPO TRANS
        INFIL RUNOFF
 PRESTOR
                                     DRAIN NEWSTOR
                                                           STORAGE
 20.7857+ 0.0000+ 0.0000 - 0.1566- 0.3334- 0.0038 = 20.2919 Versus 20.2993
Mass Balance = -7.4043E-03 cm; Time step attempts =***** and successes =*****
  Evaporation: Potential = 0.9981 cm, Actual = 0.1566 cm
Transpiration: Potential = 2.8548 cm, Actual = 0.3334 cm
RHMEAN = 60.9 %; TMEAN = 297.6 K; HDRY = 6.8006E+05 cm; DAYUBC = ****
   DAILY SUMMARY: Day = 168, Simulated Time = 24.0000 hr
_____
Node Number =
Depth (cm) =
                    2
                              12
                                         25
                   5.00000
                                       91.50000
                             30.70000
Water (cm3/cm3) = 0.12079 0.25252 0.25314
Head (cm) = 1.45943E+04 4.89060E+02 4.81259E+02
Water Flow (cm) =-1.01799E-03-2.01499E-03 3.80735E-03
Plant Sink (cm) = 3.86899E-03 0.00000E+00 0.00000E+00
 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR
                                                           STORAGE
 20.2993+ 0.0000+ 0.0000 - 0.0048- 0.1227- 0.0038 = 20.1680 Versus 20.1767
Mass Balance = -8.6708E-03 cm; Time step attempts =***** and successes =*****
  Evaporation: Potential = 0.9129 cm, Actual = 0.0048 cm
Transpiration: Potential = 2.6112 cm, Actual = 0.1227 cm
RHMEAN = 61.0 %; TMEAN = 298.2 K; HDRY = 6.7703E+05 cm; DAYUBC = *****
DAILY SUMMARY: Day = 169, Simulated Time = 24.0000 hr
_____
Node Number
              ----
                     2
                              12
                                         25
Depth (cm)
Depth (cm) = 5.00000 30.70000 91.50000
Water (cm3/cm3) = 0.11980 0.25246 0.25314
              = 1.53659E+04 4.89857E+02 4.81257E+02
Head (cm)
Water Flow (cm) =-3.78508E-04-2.11183E-03 3.80750E-03
Plant Sink (cm) = 1.24052E-04 0.00000E+00 0.00000E+00
```

PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 20.1767+ 0.0000+ 0.0000 - 0.0041- 0.0807- 0.0038 = 20.0880 Versus 20.0946 Mass Balance = -6.5804E-03 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 1.0815 cm, Actual = 0.0041 cm Transpiration: Potential = 3.0935 cm, Actual = 0.0807 cm RHMEAN = 62.7 %; TMEAN = 299.0 K; HDRY = 6.4076E+05 cm; DAYUBC = ***** DAILY SUMMARY: Day = 170, Simulated Time = 24.0000 hr _____ 2 Node Number == 12 25 Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.11893 0.25232 0.25315 Head (cm) = 1.60879E+04 4.91588E+02 4.81233E+02Water Flow (cm) =-2.21690E-04-2.09964E-03 3.80780E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 20.0946+ 0.0000+ 0.0000 - 0.0029- 0.0517- 0.0038 = 20.0362 Versus 20.0434 Mass Balance = -7.1964E-03 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.9384 cm, Actual = 0.0029 cm Transpiration: Potential = 2.6841 cm, Actual = 0.0517 cm RHMEAN = 66.8 %; TMEAN = 299.5 K; HDRY = 5.5254E+05 cm; DAYUBC = ***** DAILY SUMMARY: Day = 171, Simulated Time = 24.0000 hr ______ Node Number Depth (cm) = 2 12 25 Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.11859 0.25232 0.25315 Head (cm) = 1.63840E+04 4.91588E+02 4.81233E+02Water Flow (cm) =-2.09570E-04-2.09284E-03 3.80787E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 20.0434+ 0.0508+ 0.0000 - 0.0484- 0.0286- 0.0038 = 20.0134 Versus 20.0222 Mass Balance = -8.7872E-03 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.5712 cm, Actual = 0.0484 cm Transpiration: Potential = 1.6338 cm, Actual = 0.0286 cm RHMEAN = 77.6 %; TMEAN = 298.2 K; HDRY = 3.4825E+05 cm; DAYUBC = ***** DAILY SUMMARY: Day = 172, Simulated Time = 24.0000 hr _____ Node Number 2 12 = 25 Node Number=21223Depth (cm)=5.000030.700091.50000Water (cm3/cm3)=0.122170.252320.25315Head (cm) = 1.36167E+04 4.91588E+02 4.81233E+02

Water Flow (cm) = 3.10951E-05-2.09284E-03 3.80787E-03Plant Sink (cm) = 1.15614E-03 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 20.0222+ 0.2794+ 0.0000 - 0.1062- 0.0275- 0.0038 = 20.1642 Versus 20.1734 Mass Balance = -9.2793E-03 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.4290 cm, Actual = 0.1062 cm Transpiration: Potential = 1.2270 cm, Actual = 0.0275 cm RHMEAN = 81.8 %; TMEAN = 294.8 K; HDRY = 2.7607E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 173, Simulated Time = 24.0000 hr _____ 2 Node Number -12 25 Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.12056 0.25232 0.25315Head (cm) = 1.47677E+04 4.91588E+02 4.81233E+02Water Flow (cm) = 4.33801E-05-2.09284E-03 3.80787E-03Plant Sink (cm) = 1.51658E-03 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 20.1734+ 0.0000+ 0.0000 - 0.1553- 0.0421- 0.0038 = 19.9722 Versus 19.9805 Mass Balance = -8.2569E-03 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.6783 cm, Actual = 0.1553 cm Transpiration: Potential = 1.9401 cm, Actual = 0.0421 cm RHMEAN = 68.0 %; TMEAN = 294.5 K; HDRY = 5.2865E+05 cm; DAYUBC = 73727 DAILY SUMMARY: Day = 174, Simulated Time = 24.0000 hr _____ Node Number ----2 12 25 Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.11966 0.25232 0.25315 Depth (cm) = 1.54751E+04 4.91588E+02 4.81233E+02 Head (cm) Water Flow (cm) =-7.25801E-05-2.09295E-03 3.80787E-03 Plant Sink (cm) = 1.75257E-05 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 19.9805+ 0.0000+ 0.0000 - 0.0043- 0.0367- 0.0038 = 19.9357 Versus 19.9448 Mass Balance = -9.0847E-03 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.7918 cm, Actual = 0.0043 cm Transpiration: Potential = 2.2646 cm, Actual = 0.0367 cm RHMEAN = 67.3 %; TMEAN = 296.5 K; HDRY = 5.4331E+05 cm; DAYUBC = ***** 

DAILY SUMMARY: Day = 175, Simulated Time = 24.0000 hr

Node Number=21225Depth (cm)=5.0000030.7000091.50000Water (cm3/cm3)=0.118790.252320.25315 Head (cm)  $= 1.62121E+04 \ 4.91588E+02 \ 4.81233E+02$ Water Flow (cm) =-1.36251E-04-2.09297E-03 3.80787E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 19.9448+ 0.0000+ 0.0000 - 0.0043- 0.0344- 0.0038 = 19.9023 Versus 19.9084 Mass Balance = -6.0825E-03 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.8570 cm, Actual = 0.0043 cm Transpiration: Potential = 2.4512 cm, Actual = 0.0344 cm RHMEAN = 66.7 %; TMEAN = 297.9 K; HDRY = 5.5541E+05 cm; DAYUBC = ***** DAILY SUMMARY: Day = 176, Simulated Time = 24.0000 hr _____ Node Number = Depth (cm) = 2 12 25 Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.11858 0.25232 0.25315 Head (cm) = 1.63931E+04 4.91588E+02 4.81233E+02 Water Flow (cm) =-1.79902E-04-2.09294E-03 3.80787E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 19.9084+ 0.0000+ 0.0000 - 0.0041- 0.0307- 0.0038 = 19.8698 Versus 19.8900 Mass Balance = -2.0214E-02 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.8655 cm, Actual = 0.0041 cm Transpiration: Potential = 2.4756 cm, Actual = 0.0307 cm RHMEAN = 66.7 %; TMEAN = 297.9 K; HDRY = 5.5541E+05 cm; DAYUBC = ***** DAILY SUMMARY: Day = 177, Simulated Time = 24.0000 hr _____ Node Number = Depth (cm) = 2 12 25 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.11830 0.252150.25315 Head (cm) = 1.66517E+04 4.93768E+02 4.81186E+02Water Flow (cm) =-1.99908E-04-2.17568E-03 3.80864E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 19.8900+ 0.0000+ 0.0000 - 0.0023- 0.0258- 0.0038 = 19.8581 Versus 19.8755 Mass Balance = -1.7418E-02 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.7743 cm, Actual = 0.0023 cm Transpiration: Potential = 2.2147 cm, Actual = 0.0258 cm RHMEAN = 70.6 %; TMEAN = 297.3 K; HDRY = 4.7625E+05 cm; DAYUBC = ***** 

DAILY SUMMARY: Day = 178, Simulated Time = 24.0000 hr Node Number = 2 12 25 Depth (cm) = 5.00000 30.70000 91.50000 = 0.11829 0.25214 0.25315 Water (cm3/cm3) = 1.66549E+04 4.93792E+02 4.81186E+02 Head (cm) Water Flow (cm) =-2.03811E-04-2.19070E-03 3.80881E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 19.8755+ 0.0000+ 0.0000 - 0.0036- 0.0251- 0.0038 = 19.8431 Versus 19.8718 Mass Balance = -2.8727E-02 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.7762 cm, Actual = 0.0036 cm Transpiration: Potential = 2.2201 cm, Actual = 0.0251 cmRHMEAN = 70.7 %; TMEAN = 298.7 K; HDRY = 4.7547E+05 cm; DAYUBC = ***** DAILY SUMMARY: Day = 179, Simulated Time = 24.0000 hr _____ Node Number 2 12 25 -----Depth (cm) = 5.00000 Water (cm3/cm3) = 0.26647 30.70000 91.50000 0.25214 0.25315 = 5.34712E+02 4.93792E+02 4.81186E+02 Head (cm) Water Flow (cm) = 1.42235E+00-2.19293E-03 3.80881E-03 Plant Sink (cm) = 1.84596E-02 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 19.8718+ 2.6782+ 1.1318 - 0.0537- 0.1533- 0.0038 = 22.3392 Versus 22.3568 Mass Balance = -1.7611E-02 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.2237 cm, Actual = 0.0537 cm Transpiration: Potential = 0.6400 cm, Actual = 0.1533 cmRHMEAN = 85.7 %; TMEAN = 295.4 K; HDRY = 2.1212E+05 cm; DAYUBC = 26291 DAILY SUMMARY: Day = 180, Simulated Time = 24.0000 hr _____ Node Number 12 25 2 = Depth (cm) = 5.00000 30.70000 91.50000 = 0.25972 0.25214 0.25315Water (cm3/cm3) Head (cm) = 5.87145E+02 4.93792E+02 4.81186E+02Water Flow (cm) = 8.23838E-01-2.19293E-03 3.80881E-03Plant Sink (cm) =  $5.39271E-02 \ 0.00000E+00 \ 0.00000E+00$ PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 22.3568+ 1.1684+ 0.0000 - 0.1569- 0.4487- 0.0038 = 22.9158 Versus 22.9233 Mass Balance = -7.4730E-03 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.6403 cm, Actual = 0.1569 cm Transpiration: Potential = 1.8314 cm, Actual = 0.4487 cm

RHMEAN = 79.5 %; TMEAN = 297.3 K; HDRY = 3.1495E+05 cm; DAYUBC = 0

DAILY SUMMARY: Day = 181, Simulated Time = 24.0000 hr _____ Node Number 2 25 == 12 Depth (cm) Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.21642 0.25214 0.25315 = 1.12322E+03 4.93793E+02 4.81186E+02 Head (cm) =-4.62051E-02-2.19409E-03 3.80881E-03 Water Flow (cm) Plant Sink (cm) =  $4.90442E-02 \ 0.00000E+00 \ 0.00000E+00$ PRESTOR INFIL RUNOFF DRAIN NEWSTOR EVAPO TRANS STORAGE 22.9233+ 0.0000+ 0.0000 - 0.1427- 0.4081- 0.0038 = 22.3687 Versus 22.3754 Mass Balance = -6.6986E-03 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.5707 cm, Actual = 0.1427 cm Transpiration: Potential = 1.6323 cm, Actual = 0.4081 cm RHMEAN = 79.4 %; TMEAN = 298.4 K; HDRY = 3.1659E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 182, Simulated Time = 24.0000 hr _____ Node Number 2 12 25 -----Depth (cm) = Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.16752 0.25214 0.25315 Head (cm) = 2.94196E+03 4.93793E+02 4.81186E+02 Water Flow (cm) =-6.52096E-02-2.19425E-03 3.80881E-03 Plant Sink (cm) = 5.85597E-02 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 22.3754+ 0.0000+ 0.0000 - 0.1703- 0.4872- 0.0038 = 21.7140 Versus 21.7201 Mass Balance = -6.0387E-03 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.6814 cm, Actual = 0.1703 cm Transpiration: Potential = 1.9490 cm, Actual = 0.4872 cm RHMEAN = 81.2 %; TMEAN = 298.7 K; HDRY = 2.8516E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 183, Simulated Time = 24.0000 hr -----Node Number 2 12 25 -Depth (cm) Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.13395 0.25214 0.25315 = 8.13856E+03 4.93793E+02 4.81186E+02 Head (cm) Water Flow (cm) =-3.67536E-02-2.19425E-03 3.80881E-03 Plant Sink (cm) = 3.99819E-02 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 21.7201+ 0.0000+ 0.0000 - 0.1629- 0.4354- 0.0038 = 21.1180 Versus 21.1243

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Mass Balance = -6.3419E-03 cm; Time step attempts =***** and successes =*****
  Evaporation: Potential = 0.7844 cm, Actual = 0.1629 cm
Transpiration: Potential = 2.2436 cm, Actual = 0.4354 cm
RHMEAN = 75.3 %; TMEAN = 299.3 K; HDRY = 3.8921E+05 cm; DAYUBC = 88137
DAILY SUMMARY: Day = 184, Simulated Time = 24.0000 hr
-----
Node Number
              =
                     2
                               12
                                         25
Depth (cm) = 5.00000 30.70000
Water (cm3/cm3) = 0.12458 0.25211
                                       91.50000
                                        0.25315
Head (cm) = 1.21246E+04 4.94187E+02 4.81169E+02
Water Flow (cm) =-1.09945E-02-1.94115E-03 3.80929E-03
Plant Sink (cm) = 1.10533E-02 0.00000E+00 0.00000E+00
 PRESTOR INFIL RUNOFF
                       EVAPO TRANS
                                     DRAIN NEWSTOR
                                                            STORAGE
 21.1243+ 0.0000+ 0.0000 - 0.0027- 0.2510- 0.0038 = 20.8668 Versus 20.8717
Mass Balance = -4.8904E-03 cm; Time step attempts =***** and successes =*****
  Evaporation: Potential = 0.7208 cm, Actual = 0.0027 cm
Transpiration: Potential = 2.0618 cm, Actual = 0.2510 cm
RHMEAN = 76.3 %; TMEAN = 299.3 K; HDRY = 3.7158E+05 cm; DAYUBC = *****
  DAILY SUMMARY: Day = 185, Simulated Time = 24.0000 hr
Node Number =
Depth (cm) =
                     2
                              12
                                         25
                   5.00000
                             30.70000
                                       91.50000
Water (cm3/cm3) = 0.12098 \quad 0.25211 \quad 0.25315
              = 1.44556E+04 4.94217E+02 4.81169E+02
Head (cm)
Water Flow (cm) =-3.83536E-03-1.94394E-03 3.80932E-03
Plant Sink (cm) = 3.70989E-03 0.00000E+00 0.00000E+00
 PRESTOR INFIL RUNOFF
                       EVAPO TRANS DRAIN NEWSTOR
                                                           STORAGE
 20.8717+ 0.0000+ 0.0000 - 0.0068- 0.2108- 0.0038 = 20.6503 Versus 20.6573
Mass Balance = -7.0076E-03 cm; Time step attempts =***** and successes =*****
  Evaporation: Potential = 0.9113 cm, Actual = 0.0068 cm
Transpiration: Potential = 2.6066 cm, Actual = 0.2108 cm
RHMEAN = 69.2 %; TMEAN = 301.5 K; HDRY = 5.0451E+05 cm; DAYUBC = *****
DAILY SUMMARY: Day = 186, Simulated Time = 24.0000 hr
_____
Node Number
              =
                     2
                              12
                                         25
Depth (cm)
Depth (cm) = 5.00000 30.70000 91.50000
Water (cm3/cm3) = 0.12019 0.25197 0.25316
              = 1.50583E+04 4.95983E+02 4.81099E+02
Head (cm)
Water Flow (cm) =-1.53655E-03-1.97311E-03 3.81049E-03
Plant Sink (cm) = 3.64569E-04 0.00000E+00 0.00000E+00
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EVAPO TRANS DRAIN NEWSTOR PRESTOR INFIL RUNOFF STORAGE 20.6573+ 0.0000+ 0.0000 - 0.0021- 0.1136- 0.0038 = 20.5379 Versus 20.5434 Mass Balance = -5.4703E-03 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.6792 cm, Actual = 0.0021 cm Transpiration: Potential = 1.9428 cm, Actual = 0.1136 cm RHMEAN = 72.8 %; TMEAN = 301.5 K; HDRY = 4.3456E+05 cm; DAYUBC = ***** DAILY SUMMARY: Day = 187, Simulated Time = 24.0000 hr ______ Node Number = 2 12 25 Depth (cm) = 5.00000 30.70000 Water (cm3/cm3) = 0.11961 0.25197 91.50000 0.25316 Head (cm) = 1.55216E+04 4.95983E+02 4.81099E+02Water Flow (cm) =-8.62326E-04-1.98032E-03 3.81082E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 20.5434+ 0.0000+ 0.0000 - 0.0039- 0.1251- 0.0038 = 20.4106 Versus 20.4172 Mass Balance = -6.5441E-03 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.9536 cm, Actual = 0.0039 cm Transpiration: Potential = 2.7274 cm, Actual = 0.1251 cm RHMEAN = 71.9 %; TMEAN = 302.6 K; HDRY = 4.5238E+05 cm; DAYUBC = ***** DAILY SUMMARY: Day = 188, Simulated Time = 24.0000 hr _____ Node Number = 2 12 25 Depth (cm) Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.11882 0.25197 0.25316 Head (cm) = 1.61837E+04 4.95983E+02 4.81099E+02Water Flow (cm) =-4.48546E-04-1.98032E-03 3.81082E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 20.4172+ 0.0000+ 0.0000 - 0.0057- 0.1065- 0.0038 = 20.3012 Versus 20.3079 Mass Balance = -6.6509E-03 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 1.0723 cm, Actual = 0.0057 cm Transpiration: Potential = 3.0672 cm, Actual = 0.1065 cm RHMEAN = 67.2 %; TMEAN = 303.4 K; HDRY = 5.4446E+05 cm; DAYUBC = ***** 

Node Number		2	12	25
Depth (cm)	==	5.00000	30.70000	91.50000
Water (cm3/cm3)	=	0.11859	0.25197	0.25316

DAILY SUMMARY: Day = 189, Simulated Time = 24.0000 hr

= 1.63876E+04 4.95983E+02 4.81099E+02 Head (cm) Water Flow (cm) =-2.96343E-04-1.98032E-03 3.81082E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 20.3079+ 0.0000+ 0.0000 - 0.0044- 0.0817- 0.0038 = 20.2180 Versus 20.2269 Mass Balance = -8.9226E-03 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 1.0409 cm, Actual = 0.0044 cm Transpiration: Potential = 2.9774 cm, Actual = 0.0817 cm RHMEAN = 66.3 %; TMEAN = 302.9 K; HDRY = 5.6432E+05 cm; DAYUBC = ***** DAILY SUMMARY: Day = 190, Simulated Time = 24.0000 hr Node Number = Depth (cm) = 2 12 2.5 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.11856 0.25197 0.25316Head (cm) = 1.64125E+04 4.95983E+02 4.81099E+02Water Flow (cm) =-2.24830E-04-1.98041E-03 3.81082E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 20.2269+ 0.0000+ 0.0000 - 0.0047- 0.0629- 0.0038 = 20.1554 Versus 20.1654 Mass Balance = -1.0021E-02 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.9530 cm, Actual = 0.0047 cm Transpiration: Potential = 2.7257 cm, Actual = 0.0629 cmRHMEAN = 64.8 %; TMEAN = 303.2 K; HDRY = 5.9364E+05 cm; DAYUBC = ***** DAILY SUMMARY: Day = 191, Simulated Time = 24.0000 hr ______ 2 Node Number = 12 25 Depth (cm) Depth (cm) = 5.00000 30.70000 Water (cm3/cm3) = 0.11825 0.25182 91.50000 0.25316 Head (cm) = 1.66906E+04 4.97913E+02 4.81024E+02Water Flow (cm) =-2.29770E-04-2.17490E-03 3.81201E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 20.1654+ 0.0000+ 0.0000 - 0.0024- 0.0492- 0.0038 = 20.1101 Versus 20.1169 Mass Balance = -6.8779E-03 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.8203 cm, Actual = 0.0024 cm Transpiration: Potential = 2.3463 cm, Actual = 0.0492 cm RHMEAN = 69.4 %; TMEAN = 302.3 K; HDRY = 5.0120E+05 cm; DAYUBC = ***** 

DAILY SUMMARY: Day = 192, Simulated Time = 24.0000 hr

Node Number = 2 12 25 Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.11813 0.25176 0.25316 Head (cm) = 1.68042E+04 4.98695E+02 4.80997E+02=-2.34489E-04-2.25077E-03 3.81286E-03 Water Flow (cm) Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 20.1169+ 0.0000+ 0.0000 - 0.0030- 0.0490- 0.0038 = 20.0611 Versus 20.0697 Mass Balance = -8.5621E-03 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.8920 cm, Actual = 0.0030 cm Transpiration: Potential = 2.5514 cm, Actual = 0.0490 cm RHMEAN = 71.0 %; TMEAN = 302.6 K; HDRY = 4.6970E+05 cm; DAYUBC = ***** DAILY SUMMARY: Day = 193, Simulated Time = 24.0000 hr _____ Node Number Depth (cm) 2 12 25 = Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.11751 0.25176 0.25316 Head (cm) = 1.73901E+04 4.98695E+02 4.80997E+02Water Flow (cm) =-2.48388E-04-2.25205E-03 3.81290E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 20.0697+ 0.0000+ 0.0000 - 0.0074- 0.0484- 0.0038 = 20.0100 Versus 20.0211 Mass Balance = -1.1101E-02 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 1.0013 cm, Actual = 0.0074 cm Transpiration: Potential = 2.8641 cm, Actual = 0.0484 cm RHMEAN = 62.0 %; TMEAN = 304.8 K; HDRY = 6.5517E+05 cm; DAYUBC = ***** DAILY SUMMARY: Day = 194, Simulated Time = 24.0000 hr _____ Node Number 2 12 25 = Node Number=21223Depth (cm)=5.0000030.7000091.50000Water (cm3/cm3)=0.116720.251420.25318Head (cm) = 1.81755E+04 5.03066E+02 4.80759E+02Water Flow (cm) =-3.04324E-04-2.31818E-03 3.81549E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 EVAPO TRANS PRESTOR INFIL RUNOFF DRAIN NEWSTOR STORAGE 20.0211+ 0.0000+ 0.0000 - 0.0000- 0.0215- 0.0038 = 19.9958 Versus 19.9959 Mass Balance = -5.7220E-05 cm; Time step attempts = 2595 and successes = 2595 Evaporation: Potential = 0.4491 cm, Actual = 0.0000 cm Transpiration: Potential = 1.2847 cm, Actual = 0.0215 cm RHMEAN = 82.3 %; TMEAN = 299.0 K; HDRY = 2.6705E+05 cm; DAYUBC = 0 

DAILY SUMMARY: Day = 195, Simulated Time = 24.0000 hr ______ Node Number -----12 2 25 Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.11668 0.25142 0.25318Head (cm) = 1.82139E+04 5.03067E+02 4.80759E+02Water Flow (cm) =-3.38378E-04-2.36434E-03 3.81808E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 19.9959+ 0.0000+ 0.0000 - 0.0068- 0.0349- 0.0038 = 19.9504 Versus 19.9635 Mass Balance = -1.3170E-02 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.7192 cm, Actual = 0.0068 cm Transpiration: Potential = 2.0572 cm, Actual = 0.0349 cm RHMEAN = 66.0 %; TMEAN = 298.7 K; HDRY = 5.6975E+05 cm; DAYUBC = ***** DAILY SUMMARY: Day = 196, Simulated Time = 24.0000 hr _____ Node Number 2 12 25 = Depth (cm) = 5.00000 30.70000 Water (cm3/cm3) = 0.11611 0.25119 91.50000 0.25119 0.25320 = 1.88234E+04 5.06102E+02 4.80603E+02 Head (cm) Water Flow (cm) =-3.63693E-04-2.39584E-03 3.82016E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 19.9635+ 0.0000+ 0.0000 - 0.0007- 0.0287- 0.0038 = 19.9303 Versus 19.9328 Mass Balance = -2.4662E-03 cm; Time step attempts =52128 and successes =52128 Evaporation: Potential = 0.6150 cm, Actual = 0.0007 cm Transpiration: Potential = 1.7592 cm, Actual = 0.0287 cm RHMEAN = 73.8 %; TMEAN = 297.9 K; HDRY = 4.1586E+05 cm; DAYUBC = 49556 DAILY SUMMARY: Day = 197, Simulated Time = 24.0000 hr _____ Node Number = Depth (cm) = 12 2 25 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.12454 0.25119 0.25320= 1.21492E+04 5.06102E+02 4.80603E+02Head (cm) Water Flow (cm) = 5.01107E-04-2.41094E-03 3.82155E-03Plant Sink (cm) = 1.00712E-02 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 19.9328+ 0.4572+ 0.0000 - 0.2145- 0.0822- 0.0038 = 20.0895 Versus 20.0969 Mass Balance = -7.3948E-03 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.8665 cm, Actual = 0.2145 cmTranspiration: Potential = 2.4785 cm, Actual = 0.0822 cm RHMEAN = 72.5 %; TMEAN = 302.0 K; HDRY = 4.4133E+05 cm; DAYUBC = 0

DAILY SUMMARY: Day = 198, Simulated Time = 24.0000 hr ______ Node Number = 2 12 25 Depth (cm) Depth (cm) = 5.00000 30.70000 Water (cm3/cm3) = 0.12050 0.25119 91.50000 0.25320 = 1.48167E+04 5.06102E+02 4.80603E+02Head (cm) Water Flow (cm) = 1.97183E-04-2.41095E-03 3.82155E-03Plant Sink (cm) = 3.85455E-03 0.00000E+00 0.00000E+00 INFIL RUNOFF PRESTOR EVAPO TRANS DRAIN NEWSTOR STORAGE 20.0969+ 0.0000+ 0.0000 - 0.1618- 0.0568- 0.0038 = 19.8745 Versus 19.8818 Mass Balance = -7.3547E-03 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 1.0942 cm, Actual = 0.1618 cm Transpiration: Potential = 3.1296 cm, Actual = 0.0568 cm RHMEAN = 67.2 %; TMEAN = 304.3 K; HDRY = 5.4522E+05 cm; DAYUBC = ***** DAILY SUMMARY: Day = 199, Simulated Time = 24.0000 hr ______ Node Number ----2 12 25 Depth (cm) = 5.00000 30.70000 Water (cm3/cm3) = 0.11961 0.25116 91.50000 0.25116 0.25320 Head (cm) = 1.55181E+04 5.06420E+02 4.80595E+02Water Flow (cm) =-4.42116E-05-2.42971E-03 3.82159E-03 Plant Sink (cm) = 1.37661E-05 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 19.8818+ 0.0000+ 0.0000 - 0.0035- 0.0326- 0.0038 = 19.8419 Versus 19.8581 Mass Balance = -1.6121E-02 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 1.0439 cm, Actual = 0.0035 cm Transpiration: Potential = 2.9859 cm, Actual = 0.0326 cm RHMEAN = 68.3 %; TMEAN = 303.7 K; HDRY = 5.2164E+05 cm; DAYUBC = ***** DAILY SUMMARY: Day = 200, Simulated Time = 24.0000 hr Node Number == 2 12 25 Depth (cm) Depth (cm) = 5.00000 Water (cm3/cm3) = 0.11870 30.70000 91.50000 0.25116 0.25320 Head (cm) = 1.62871E+04 5.06420E+02 4.80595E+02Water Flow (cm) =-1.21865E-04-2.42995E-03 3.82159E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 19.8581+ 0.0000+ 0.0000 - 0.0056- 0.0297- 0.0038 = 19.8190 Versus 19.8416 Mass Balance = -2.2663E-02 cm; Time step attempts =***** and successes =*****

Evaporation: Potential = 1.0635 cm, Actual = 0.0056 cm Transpiration: Potential = 3.0420 cm, Actual = 0.0297 cm RHMEAN = 64.5 %; TMEAN = 304.3 K; HDRY = 6.0147E+05 cm; DAYUBC = ***** DAILY SUMMARY: Day = 201, Simulated Time = 24.0000 hr Node Number 2 12 25 = Depth (cm) Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.11857 0.25115 0.25320 Head (cm) = 1.64076E+04 5.06658E+02 4.80591E+02Water Flow (cm) =-1.70009E-04-2.43670E-03 3.82164E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 19.8416+ 0.0000+ 0.0000 - 0.0039- 0.0255- 0.0038 = 19.8084 Versus 19.8324 Mass Balance = -2.3973E-02 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.9733 cm, Actual = 0.0039 cm Transpiration: Potential = 2.7839 cm, Actual = 0.0255 cm RHMEAN = 65.6 %; TMEAN = 302.6 K; HDRY = 5.7869E+05 cm; DAYUBC = ***** DAILY SUMMARY: Day = 202, Simulated Time = 24.0000 hr _____ Node Number 12 25 -2 Depth (cm) 5.00000 30.70000 91.50000 = Water (cm3/cm3) = 0.117940.25115 0.25320 Head (cm) = 1.69805E+04 5.06658E+02 4.80591E+02Water Flow (cm) =-1.97945E-04-2.43683E-03 3.82163E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 DRAIN NEWSTOR PRESTOR INFIL RUNOFF EVAPO TRANS STORAGE 19.8324+ 0.0000+ 0.0000 - 0.0052- 0.0247- 0.0038 = 19.7987 Versus 19.8232 Mass Balance = -2.4513E-02 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.9708 cm, Actual = 0.0052 cm Transpiration: Potential = 2.7767 cm, Actual = 0.0247 cm RHMEAN = 63.1 %; TMEAN = 303.4 K; HDRY = 6.3168E+05 cm; DAYUBC = ***** DAILY SUMMARY: Day = 203, Simulated Time = 24.0000 hr _____ Node Number = 2 12 25 Depth (cm) = 5.00000 30.70000 Water (cm3/cm3) = 0.11691 0.25115 91.50000 0.25320 Head (cm) = 1.79769E+04 5.06658E+02 4.80591E+02Water Flow (cm) =-2.62495E-04-2.43683E-03 3.82163E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00

PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 19.8232+ 0.0000+ 0.0000 - 0.0057- 0.0260- 0.0038 = 19.7877 Versus 19.8129 Mass Balance = -2.5251E-02 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 1.0739 cm, Actual = 0.0057 cm Transpiration: Potential = 3.0715 cm, Actual = 0.0260 cm RHMEAN = 60.0 %; TMEAN = 303.7 K; HDRY = 7.0047E+05 cm; DAYUBC = ***** DAILY SUMMARY: Day = 204, Simulated Time = 24.0000 hr _____ Node Number 25 222 2 12 Note Hamber21223Depth (cm)=5.0000030.7000091.50000Water (cm3/cm3)=0.116260.251140.25320Head (cm) = 1.86611E+04 5.06724E+02 4.80591E+02Water Flow (cm) =-3.24069E-04-2.44539E-03 3.82164E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 19.8129+ 0.0000+ 0.0000 - 0.0047- 0.0239- 0.0038 = 19.7805 Versus 19.8073 Mass Balance = -2.6773E-02 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 1.0302 cm, Actual = 0.0047 cm Transpiration: Potential = 2.9468 cm, Actual = 0.0239 cm RHMEAN = 60.5 %; TMEAN = 303.4 K; HDRY = 6.8878E+05 cm; DAYUBC = ***** DAILY SUMMARY: Day = 205, Simulated Time = 24.0000 hr ------Node Number Depth (cm) = 2 12 25 Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.11584 0.25108 0.25320 Head (cm) = 1.91168E+04 5.07573E+02 4.80559E+02Water Flow (cm) =-3.65398E-04-2.44988E-03 3.82234E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 INFIL RUNOFF PRESTOR EVAPO TRANS DRAIN NEWSTOR STORAGE 19.8073 + 0.0000 + 0.0000 - 0.0032 - 0.0208 - 0.0038 = 19.7795 Versus 19.8010 Mass Balance = -2.1507E-02 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.9000 cm, Actual = 0.0032 cm Transpiration: Potential = 2.5741 cm, Actual = 0.0208 cm RHMEAN = 64.6 %; TMEAN = 304.0 K; HDRY = 5.9844E+05 cm; DAYUBC = ***** DAILY SUMMARY: Day = 206, Simulated Time = 24.0000 hr _____ Node Number = Depth (cm) = 2 12 25 Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.11520 0.25108 0.25320

Head (cm) = 1.98495E+04 5.07573E+02 4.80559E+02Water Flow (cm) =-3.96037E-04-2.44959E-03 3.82244E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 19.8010+ 0.0000+ 0.0000 - 0.0058- 0.0227- 0.0038 = 19.7686 Versus 19.7957 Mass Balance = -2.7067E-02 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.9864 cm, Actual = 0.0058 cm Transpiration: Potential = 2.8213 cm, Actual = 0.0227 cm RHMEAN = 60.6 %; TMEAN = 304.8 K; HDRY = 6.8563E+05 cm; DAYUBC = ***** DAILY SUMMARY: Day = 207, Simulated Time = 24.0000 hr ______ Node Number = Depth (cm) = 2 12 25 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.11473 0.25093 0.25321Head (cm) = 2.04200E+04 5.09443E+02 4.80498E+02Water Flow (cm) =-4.34453E-04-2.46133E-03 3.82335E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 19.7957+ 0.0000+ 0.0000 - 0.0017- 0.0168- 0.0038 = 19.7734 Versus 19.7845 Mass Balance = -1.1101E-02 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.7183 cm, Actual = 0.0017 cm Transpiration: Potential = 2.0545 cm, Actual = 0.0168 cm RHMEAN = 68.7 %; TMEAN = 303.2 K; HDRY = 5.1525E+05 cm; DAYUBC = ***** DAILY SUMMARY: Day = 208, Simulated Time = 24.0000 hr ______ Node Number = 2 12 25 Depth (cm) = 5.00000 30.70000 91.50000Water (cm3/cm3) = 0.11472 0.25093 0.25321Head (cm) = 2.04322E+04 5.09443E+02 4.80498E+02Water Flow (cm) =-4.40419E-04-2.46608E-03 3.82365E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 19.7845+ 0.0000+ 0.0000 - 0.0043- 0.0168- 0.0038 = 19.7597 Versus 19.7867 Mass Balance = -2.7081E-02 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.6787 cm, Actual = 0.0043 cm Transpiration: Potential = 1.9414 cm, Actual = 0.0168 cm RHMEAN = 67.6 %; TMEAN = 303.4 K; HDRY = 5.3579E+05 cm; DAYUBC = ***** 

DAILY SUMMARY: Day = 209, Simulated Time = 24.0000 hr

Node Number = 2 12 25 Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.11508 0.25093 0.25321 Head (cm) = 1.99946E+04 5.09443E+02 4.80498E+02Water Flow (cm) =-4.21447E-04-2.46608E-03 3.82365E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 19.7867+ 0.1524+ 0.0000 - 0.1533- 0.0217- 0.0038 = 19.7604 Versus 19.7859 Mass Balance = -2.5557E-02 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.8766 cm, Actual = 0.1533 cm Transpiration: Potential = 2.5074 cm, Actual = 0.0217 cm RHMEAN = 66.0 %; TMEAN = 303.7 K; HDRY = 5.6930E+05 cm; DAYUBC = **** DAILY SUMMARY: Day = 210, Simulated Time = 24.0000 hr _____ Node Number = 2 12 25 Depth (cm) Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.11457 0.25076 0.25321 Head (cm) = 2.06186E+04 5.11701E+02 4.80471E+02Water Flow (cm) =-4.36332E-04-2.47995E-03 3.82418E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 19.7859+ 0.0000+ 0.0000 - 0.0009- 0.0192- 0.0038 = 19.7620 Versus 19.7683 Mass Balance = -6.3000E-03 cm; Time step attempts = 69692 and successes = 69692Evaporation: Potential = 0.7809 cm, Actual = 0.0009 cm Transpiration: Potential = 2.2337 cm, Actual = 0.0192 cm RHMEAN = 73.3 %; TMEAN = 302.6 K; HDRY = 4.2644E+05 cm; DAYUBC = 67137 DAILY SUMMARY: Day = 211, Simulated Time = 24.0000 hr ______ Node Number = 2 12 25 Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.11396 0.25076 0.25321 Head (cm) = 2.13892E+04 5.11701E+02 4.80471E+02Water Flow (cm) =-4.62685E-04-2.48005E-03 3.82425E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 INFIL RUNOFF EVAPO TRANS PRESTOR DRAIN NEWSTOR STORAGE 19.7683+ 0.0000+ 0.0000 - 0.0079- 0.0240- 0.0038 = 19.7326 Versus 19.7547 Mass Balance = -2.2156E-02 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 1.0298 cm, Actual = 0.0079 cm Transpiration: Potential = 2.9454 cm, Actual = 0.0240 cm RHMEAN = 63.1 %; TMEAN = 304.3 K; HDRY = 6.3189E+05 cm; DAYUBC = *****

DAILY SUMMARY: Day = 212, Simulated Time = 24.0000 hr _____ Node Number = 2 12 25 Depth (cm) = 5.00000 30.70000 91.50000  $U_{m}$ -0.000030.7000091.50000Water (cm3/cm3)=0.112940.250760.25321Head (cm) = 2.27922E+04 5.11701E+02 4.80471E+02Water Flow (cm) =-5.09722E-04-2.48005E-03 3.82425E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 19.7547+ 0.0000+ 0.0000 - 0.0057- 0.0234- 0.0038 = 19.7218 Versus 19.7427 Mass Balance = -2.0853E-02 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 1.0975 cm, Actual = 0.0057 cm Transpiration: Potential = 3.1390 cm, Actual = 0.0234 cm RHMEAN = 59.7 %; TMEAN = 304.3 K; HDRY = 7.0617E+05 cm; DAYUBC = ***** DAILY SUMMARY: Day = 213, Simulated Time = 24.0000 hr Node Number = 2 12 25 Depth (cm) = 5.00000 30.70000 Water (cm3/cm3) = 0.11229 0.25074 91.50000 0.25321 Head (cm) = 2.37665E+04 5.11950E+02 4.80471E+02Water Flow (cm) =-5.55003E-04-2.48684E-03 3.82426E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 19.7427+ 0.0000+ 0.0000 - 0.0042- 0.0197- 0.0038 = 19.7149 Versus 19.7352 Mass Balance = -2.0332E-02 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.9783 cm, Actual = 0.0042 cm Transpiration: Potential = 2.7983 cm, Actual = 0.0197 cm RHMEAN = 61.3 %; TMEAN = 302.9 K; HDRY = 6.6971E+05 cm; DAYUBC = ***** DAILY SUMMARY: Day = 214, Simulated Time = 24.0000 hr ______ Node Number = 2 12 25 Depth (cm) ----5.00000 30.70000 91.50000 = 0.11166 0.25068 0.25321 Water (cm3/cm3) = 2.47694E+04 5.12739E+02 4.80476E+02 Head (cm) Water Flow (cm) =-5.86752E-04-2.49789E-03 3.82423E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 19.7352+ 0.0000+ 0.0000 - 0.0030- 0.0182- 0.0038 = 19.7102 Versus 19.7264 Mass Balance = -1.6205E-02 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.9170 cm, Actual = 0.0030 cm Transpiration: Potential = 2.6229 cm, Actual = 0.0182 cmRHMEAN = 65.6 %; TMEAN = 302.9 K; HDRY = 5.7743E+05 cm; DAYUBC = *****

DAILY SUMMARY: Day = 215, Simulated Time = 24.0000 hr _____ 2 Node Number = 12 25 Depth (cm) = 5.00000 30.70000 91,50000 Water (cm3/cm3) = 0.11107 0.250680.25321 Head (cm) = 2.57496E+04 5.12739E+02 4.80476E+02Water Flow (cm) =-6.14362E-04-2.49588E-03 3.82419E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 19.7264+ 0.0000+ 0.0000 - 0.0049- 0.0155- 0.0038 = 19.7022 Versus 19.7248 Mass Balance = -2.2659E-02 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.7669 cm, Actual = 0.0049 cm Transpiration: Potential = 2.1934 cm, Actual = 0.0155 cm RHMEAN = 63.6 %; TMEAN = 301.5 K; HDRY = 6.1925E+05 cm; DAYUBC = ***** DAILY SUMMARY: Day = 216, Simulated Time = 24.0000 hr _____ ..... 2 Node Number 12 25 Depth (cm) = 5.00000 30.70000 Water (cm3/cm3) = 0.11006 0.25068 91.50000 0.25321 = 2.75846E+04 5.12739E+02 4.80476E+02Head (cm) Water Flow (cm) =-6.50441E-04-2.49588E-03 3.82419E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 19.7248+ 0.0000+ 0.0000 - 0.0058- 0.0168- 0.0038 = 19.6984 Versus 19.7198 Mass Balance = -2.1358E-02 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.8065 cm, Actual = 0.0058 cm Transpiration: Potential = 2.3068 cm, Actual = 0.0168 cm RHMEAN = 59.8 %; TMEAN = 302.9 K; HDRY = 7.0510E+05 cm; DAYUBC = ***** DAILY SUMMARY: Day = 217, Simulated Time = 24.0000 hr _____ Node Number 12 25 ----2 Depth (cm) = 5.00000 30.70000 91.50000 Depth (cm)=5.0000030.7000091.50000Water (cm3/cm3)=0.109040.250680.25321Head (cm) = 2.96427E+04 5.12739E+02 4.80476E+02Water Flow (cm) =-6.93830E-04-2.49588E-03 3.82419E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR PRESTOR STORAGE 19.7198+ 0.0000+ 0.0000 - 0.0051- 0.0187- 0.0038 = 19.6921 Versus 19.7136 Mass Balance = -2.1498E-02 cm; Time step attempts =***** and successes =*****

Evaporation: Potential = 0.9052 cm, Actual = 0.0051 cm Transpiration: Potential = 2.5892 cm, Actual = 0.0187 cm RHMEAN = 58.9 %; TMEAN = 304.0 K; HDRY = 7.2532E+05 cm; DAYUBC = ***** DAILY SUMMARY: Day = 218, Simulated Time = 24.0000 hr ______ Node Number Depth (cm) = 2 12 25 Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.10804 0.25068 0.25321 Head (cm) = 3.19054E+04 5.12739E+02 4.80476E+02Water Flow (cm) =-7.34359E-04-2.49588E-03 3.82419E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 INFIL RUNOFF PRESTOR EVAPO TRANS DRAIN NEWSTOR STORAGE 19.7136+ 0.0000+ 0.0000 - 0.0049- 0.0175- 0.0038 = 19.6874 Versus 19.7091 Mass Balance = -2.1627E-02 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.8497 cm, Actual = 0.0049 cmTranspiration: Potential = 2.4303 cm, Actual = 0.0175 cm RHMEAN = 58.9 %; TMEAN = 301.5 K; HDRY = 7.2635E+05 cm; DAYUBC = ***** DAILY SUMMARY: Day = 219, Simulated Time = 24.0000 hr _____ Node Number = Depth (cm) = 2 12 25 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.10763 0.25059 0.25320= 3.28866E+04 5.14032E+02 4.80505E+02 Head (cm) Water Flow (cm) =-7.61495E-04-2.49867E-03 3.82379E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 19.7091+ 0.0000+ 0.0000 - 0.0022- 0.0155- 0.0038 = 19.6876 Versus 19.7008 Mass Balance = -1.3195E-02 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.7381 cm, Actual = 0.0022 cm Transpiration: Potential = 2.1112 cm, Actual = 0.0155 cm RHMEAN = 66.2 %; TMEAN = 301.8 K; HDRY = 5.6489E+05 cm; DAYUBC = ***** DAILY SUMMARY: Day = 220, Simulated Time = 24.0000 hr _____ Node Number Depth (cm) == 2 12 25 Depth (cm) = 5.00000 30.70000 Water (cm3/cm3) = 0.10703 0.25059 91.50000 0.25320 = 3.44301E+04 5.14032E+02 4.80505E+02 Head (cm) Water Flow (cm) =-7.75447E-04-2.49564E-03 3.82357E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00

PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 19.7008+ 0.0000+ 0.0000 - 0.0058- 0.0170- 0.0038 = 19.6742 Versus 19.6965 Mass Balance = -2.2278E-02 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.7955 cm, Actual = 0.0058 cm Transpiration: Potential = 2.2753 cm, Actual = 0.0170 cm RHMEAN = 61.7 %; TMEAN = 302.9 K; HDRY = 6.6088E+05 cm; DAYUBC = ***** DAILY SUMMARY: Day = 221, Simulated Time = 24.0000 hr ____ Node Number Depth (cm) 2 = 12 25 Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.10592 0.25059 0.25320 Head (cm) = 3.75768E+04 5.14032E+02 4.80505E+02Water Flow (cm) =-8.06298E-04-2.49564E-03 3.82357E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 19.6965+ 0.0000+ 0.0000 - 0.0086- 0.0229- 0.0038 = 19.6611 Versus 19.6828 Mass Balance = -2.1667E-02 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 1.1436 cm, Actual = 0.0086 cm Transpiration: Potential = 3.2710 cm, Actual = 0.0229 cm RHMEAN = 49.5 %; TMEAN = 306.2 K; HDRY = 9.6246E+05 cm; DAYUBC = ***** DAILY SUMMARY: Day = 222, Simulated Time = 24.0000 hr _____ Node Number = 12 2 25 Depth (cm) = Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.13888 0.25059 0.25320 = 6.77754E+03 5.14032E+02 4.80505E+02Head (cm) Water Flow (cm) = 4.27166E-03-2.49564E-03 3.82357E-03Plant Sink (cm) = 3.38308E-02 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 19.6828+ 0.7112+ 0.0000 - 0.1772- 0.1676- 0.0038 = 20.0454 Versus 20.0674 Mass Balance = -2.2018E-02 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.7159 cm, Actual = 0.1772 cmTranspiration: Potential = 2.0477 cm, Actual = 0.1676 cmRHMEAN = 64.0 %; TMEAN = 303.2 K; HDRY = 6.1114E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 223, Simulated Time = 24.0000 hr ______ Node Number=21225Depth (cm)=5.0000030.7000091.50000Water (cm3/cm3)=0.131700.250590.25320

= 8.89712E+03 5.14032E+02 4.80505E+02 Head (cm) Water Flow (cm) = 2.51901E - 03 - 2.49564E - 03 3.82357E - 03Plant Sink (cm) = 9.34153E-03 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 20.0674+ 0.0000+ 0.0000 - 0.0818- 0.0509- 0.0038 = 19.9309 Versus 19.9525 Mass Balance = -2.1605E-02 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.3270 cm, Actual = 0.0818 cm Transpiration: Potential = 0.9354 cm, Actual = 0.0509 cm RHMEAN = 81.9 %; TMEAN = 300.1 K; HDRY = 2.7336E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 224, Simulated Time = 24.0000 hr _____ Node Number = Depth (cm) = 2 12 25 5.00000 30.70000 91.50000 20pcm (cm)-5.0000030.7000091.50000Water (cm3/cm3)=0.123610.250590.25320Head (cm) = 1.26935E+04 5.14032E+02 4.80505E+02Water Flow (cm) = 8.30656E - 04 - 2.49564E - 03 3.82357E - 03Plant Sink (cm) = 7.53435E-03 0.00000E+00 0.00000E+00PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 19.9525+ 0.0000+ 0.0000 - 0.1311- 0.0513- 0.0038 = 19.7663 Versus 19.7859 Mass Balance = -1.9564E-02 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.5990 cm, Actual = 0.1311 cm Transpiration: Potential = 1.7132 cm, Actual = 0.0513 cm RHMEAN = 73.1 %; TMEAN = 301.8 K; HDRY = 4.2926E+05 cm; DAYUBC = 81931 DAILY SUMMARY: Day = 225, Simulated Time = 24.0000 hr ______ Node Number == 2 12 25 Depth (cm) Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.12023 0.25059 0.25320 Head (cm) = 1.50226E+04 5.14032E+02 4.80505E+02 Water Flow (cm) = 9.06027E-05-2.49564E-03 3.82357E-03Plant Sink (cm) = 2.02600E-03 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 19.7859+ 0.0000+ 0.0000 - 0.0091- 0.0314- 0.0038 = 19.7415 Versus 19.7612 Mass Balance = -1.9699E-02 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.8285 cm, Actual = 0.0091 cm Transpiration: Potential = 2.3699 cm, Actual = 0.0314 cm RHMEAN = 59.8 %; TMEAN = 301.5 K; HDRY = 7.0395E+05 cm; DAYUBC = ***** 

DAILY SUMMARY: Day = 226, Simulated Time = 24.0000 hr

Node Number = 2 12 25 Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.11887 0.25059 0.25320 Head (cm) = 1.61407E+04 5.14032E+02 4.80505E+02Water Flow (cm) =-1.02665E-04-2.49564E-03 3.82357E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 19.7612+ 0.0000+ 0.0000 - 0.0058- 0.0204- 0.0038 = 19.7311 Versus 19.7513 Mass Balance = -2.0199E-02 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.8303 cm, Actual = 0.0058 cm Transpiration: Potential = 2.3748 cm, Actual = 0.0204 cm RHMEAN = 57.2 %; TMEAN = 300.1 K; HDRY = 7.6624E+05 cm; DAYUBC = ***** DAILY SUMMARY: Day = 227, Simulated Time = 24.0000 hr Node Number = 2 12 25 Depth (cm) Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.18263 0.25059 0.25320 Head (cm) = 2.09431E+03 5.14032E+02 4.80505E+02Water Flow (cm) = 2.10998E-01-2.49564E-03 3.82357E-03Plant Sink (cm) =  $7.11507E-02 \ 0.00000E+00 \ 0.00000E+00$ PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 19.7513+ 1.2757+ 0.5784 - 0.2070- 0.4411- 0.0038 = 20.3751 Versus 20.3958 Mass Balance = -2.0626E-02 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.8448 cm, Actual = 0.2070 cm Transpiration: Potential = 2.4164 cm, Actual = 0.4411 cm RHMEAN = 63.4 %; TMEAN = 301.2 K; HDRY = 6.2507E+05 cm; DAYUBC = 12000 DAILY SUMMARY: Day = 228, Simulated Time = 24.0000 hr ____ Node Number = 2 12 Depth (cm) = 5.00000 30.70000 Water (cm3/cm3) = 0.13797 0.25059 25 91.50000 0.25059 0.25320 Head (cm) = 7.00339E+03 5.14032E+02 4.80505E+02Water Flow (cm) = 1.54689E-02-2.49564E-03 3.82357E-03Plant Sink (cm) = 4.17831E-02 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 20.3958+ 0.0000+ 0.0000 - 0.1667- 0.2666- 0.0038 = 19.9587 Versus 19.9801 Mass Balance = -2.1389E-02 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.6666 cm, Actual = 0.1667 cm Transpiration: Potential = 1.9067 cm, Actual = 0.2666 cm RHMEAN = 69.7 %; TMEAN = 299.5 K; HDRY = 4.9455E+05 cm; DAYUBC = Ω 

DAILY SUMMARY: Day = 229, Simulated Time = 24.0000 hr Node Number = Depth (cm) = 2 12 25 Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.12451 0.25059 0.25320 Head (cm) = 1.21652E+04 5.14032E+02 4.80505E+02Water Flow (cm) = 6.50765E-05-2.49564E-03 3.82357E-03Plant Sink (cm)  $= 1.27606E-02 \ 0.00000E+00 \ 0.00000E+00$ PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 19.9801+ 0.0000+ 0.0000 - 0.0813- 0.1068- 0.0038 = 19.7882 Versus 19.8092 Mass Balance = -2.1040E-02 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.7197 cm, Actual = 0.0813 cm Transpiration: Potential = 2.0587 cm, Actual = 0.1068 cm RHMEAN = 70.9 %; TMEAN = 299.8 K; HDRY = 4.7128E+05 cm; DAYUBC = ***** DAILY SUMMARY: Day = 230, Simulated Time = 24.0000 hr _____ 2 Node Number = 12 25 Node Number=21225Depth (cm)=5.0000030.7000091.50000Water (cm3/cm3)=0.120880.250580.25320 = 1.45302E+04 5.14081E+02 4.80505E+02 Head (cm) Water Flow (cm) =-2.94772E-04-2.48877E-03 3.82357E-03 Plant Sink (cm) = 2.86554E-03 0.00000E+00 0.00000E+00PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 19.8092+ 0.0000+ 0.0000 - 0.0035- 0.0458- 0.0038 = 19.7561 Versus 19.7758 Mass Balance = -1.9669E-02 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.7474 cm, Actual = 0.0035 cmTranspiration: Potential = 2.1379 cm, Actual = 0.0458 cm RHMEAN = 71.2 %; TMEAN = 300.1 K; HDRY = 4.6583E+05 cm; DAYUBC = ***** DAILY SUMMARY: Day = 231, Simulated Time = 24.0000 hr _____ Node Number Depth (cm) = 2 12 25 Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.11946 0.25058 0.25320 Head (cm) = 1.56441E+04 5.14081E+02 4.80505E+02Water Flow (cm) =-2.24501E-04-2.48868E-03 3.82357E-03 Plant Sink (cm) = 8.11071E-05 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 19.7758+ 0.0000+ 0.0000 - 0.0104- 0.0293- 0.0038 = 19.7322 Versus 19.7531 Mass Balance = -2.0905E-02 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.9066 cm, Actual = 0.0104 cmTranspiration: Potential = 2.5930 cm, Actual = 0.0293 cm

RHMEAN = 53.6 %; TMEAN = 299.3 K; HDRY = 8.5417E+05 cm; DAYUBC = *****

DAILY SUMMARY: Day = 232, Simulated Time = 24.0000 hr -----Node Number === 2 12 25 Depth (cm) Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.11826 0.25054 0.25320 = 1.66836E+04 5.14669E+02 4.80523E+02 Head (cm) Water Flow (cm) =-2.13805E-04-2.50008E-03 3.82330E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 19.7531+ 0.0000+ 0.0000 - 0.0040- 0.0228- 0.0038 = 19.7225 Versus 19.7352 Mass Balance = -1.2619E-02 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.8985 cm, Actual = 0.0040 cm Transpiration: Potential = 2.5699 cm, Actual = 0.0228 cm RHMEAN = 58.6 %; TMEAN = 301.2 K; HDRY = 7.3246E+05 cm; DAYUBC = ***** DAILY SUMMARY: Day = 233, Simulated Time = 24.0000 hr _____ = Node Number 2 12 25 Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.11721 0.25054 0.25320Head (cm) = 1.76778E+04 5.14679E+02 4.80523E+02Water Flow (cm) =-2.57415E-04-2.50069E-03 3.82325E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 19.7352+ 0.0000+ 0.0000 - 0.0049- 0.0234- 0.0038 = 19.7030 Versus 19.7221 Mass Balance = -1.9091E-02 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 1.0832 cm, Actual = 0.0049 cm Transpiration: Potential = 3.0983 cm, Actual = 0.0234 cm RHMEAN = 58.9 %; TMEAN = 302.3 K; HDRY = 7.2602E+05 cm; DAYUBC = ***** DAILY SUMMARY: Day = 234, Simulated Time = 24.0000 hr _____ Node Number = 2 12 25 Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.11617 0.25054 0.25320 Head (cm) = 1.87595E+04 5.14679E+02 4.80523E+02Water Flow (cm) =-3.20134E-04-2.50069E-03 3.82325E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 19.7221+ 0.0000+ 0.0000 - 0.0060- 0.0184- 0.0038 = 19.6939 Versus 19.7153

Mass Balance = -2.1439E-02 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.9168 cm, Actual = 0.0060 cm Transpiration: Potential = 2.6223 cm, Actual = 0.0184 cm RHMEAN = 55.9 %; TMEAN = 303.7 K; HDRY = 7.9777E+05 cm; DAYUBC = ***** DAILY SUMMARY: Day = 235, Simulated Time = 24.0000 hr _____ 2 Node Number = 12 25 Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.17104 0.25054 0.25320 Head (cm) = 2.70520E+03 5.14679E+02 4.80523E+02Water Flow (cm) = 2.99450E-02-2.50069E-03 3.82325E-03Plant Sink (cm) = 3.69232E-02 0.00000E+00 0.00000E+00 DRAIN NEWSTOR PRESTOR INFIL RUNOFF EVAPO TRANS STORAGE 19.7153+ 0.8051+ 0.0077 - 0.1086- 0.1819- 0.0038 = 20.2261 Versus 20.2470 Mass Balance = -2.0918E-02 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.4389 cm, Actual = 0.1086 cm Transpiration: Potential = 1.2554 cm, Actual = 0.1819 cm RHMEAN = 67.6 %; TMEAN = 300.9 K; HDRY = 5.3764E+05 cm; DAYUBC = 119 DAILY SUMMARY: Day = 236, Simulated Time = 24.0000 hr Node Number = Depth (cm) = 12 2 25 91.50000 5.00000 30.70000 = 0.13806 0.25054 0.25320Water (cm3/cm3) = 6.97824E+03 5.14679E+02 4.80523E+02 Head (cm) Water Flow (cm) = 1.66224E-02-2.50069E-03 3.82325E-03Plant Sink (cm) = 5.05718E-02 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 20.2470+ 0.1778+ 0.0000 - 0.1934- 0.2673- 0.0038 = 19.9603 Versus 19.9808 Mass Balance = -2.0554E-02 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.7814 cm, Actual = 0.1934 cm Transpiration: Potential = 2.2352 cm, Actual = 0.2673 cm RHMEAN = 63.4 %; TMEAN = 301.2 K; HDRY = 6.2507E+05 cm; DAYUBC = DAILY SUMMARY: Day = 237, Simulated Time = 24.0000 hr _____ Node Number 200 2 12 25 Depth (cm) Depth (cm)=5.0000030.7000091.50000Water (cm3/cm3)=0.186890.250540.25320Head (cm)=1.91872E+035.14679E+024.80523E+02Water Flow (cm) = 2.29045E-01-2.50069E-03 3.82325E-03Plant Sink (cm) = 6.22032E-02 0.00000E+00 0.00000E+00

PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 19.9808+ 1.0328+ 0.2118 - 0.1809- 0.4069- 0.0038 = 20.4219 Versus 20.4415 Mass Balance = -1.9608E-02 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.7386 cm, Actual = 0.1809 cm Transpiration: Potential = 2.1125 cm, Actual = 0.4069 cm RHMEAN = 63.5 %; TMEAN = 302.6 K; HDRY = 6.2263E+05 cm; DAYUBC = 3297 DAILY SUMMARY: Day = 238, Simulated Time = 24.0000 hr ______ 2 Node Number = 12 25 Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.14064 0.25054 0.25320 Head (cm) = 6.37147E+03 5.14679E+02 4.80523E+02Water Flow (cm) = 1.77577E-02-2.50069E-03 3.82325E-03Plant Sink (cm) = 4.30854E-02 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 20.4415+ 0.0000+ 0.0000 - 0.1597- 0.2849- 0.0038 = 19.9930 Versus 20.0131 Mass Balance = -2.0103E-02 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.6389 cm, Actual = 0.1597 cm Transpiration: Potential = 1.8276 cm, Actual = 0.2849 cm RHMEAN = 72.6 %; TMEAN = 300.1 K; HDRY = 4.3936E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 239, Simulated Time = 24.0000 hr _____ Node Number Depth (cm) 2 12 25 Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.12548 0.25054 0.25320 Head (cm) = 1.16321E+04 5.14679E+02 4.80523E+02Water Flow (cm) =-1.25336E-04-2.50069E-03 3.82325E-03 Plant Sink (cm) = 1.45384E-02 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 20.0131+ 0.0000+ 0.0000 - 0.1005- 0.1225- 0.0038 = 19.7863 Versus 19.8065 Mass Balance = -2.0166E-02 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.6967 cm, Actual = 0.1005 cm Transpiration: Potential = 1.9926 cm, Actual = 0.1225 cm RHMEAN = 69.4 %; TMEAN = 301.2 K; HDRY = 5.0123E+05 cm; DAYUBC = ***** DAILY SUMMARY: Day = 240, Simulated Time = 24.0000 hr _____ Node Number = 2 12 25 Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.12126 0.25049 0.25320 Head (cm) = 1.42539E+04 5.15370E+02 4.80551E+02
Water Flow (cm) =-4.03525E-04-2.50272E-03 3.82264E-03 Plant Sink (cm) = 3.48849E-03 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 19.8065+ 0.0000+ 0.0000 - 0.0026- 0.0513- 0.0038 = 19.7488 Versus 19.7635 Mass Balance = -1.4713E-02 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.6974 cm, Actual = 0.0026 cmTranspiration: Potential = 1.9948 cm, Actual = 0.0513 cm RHMEAN = 71.9 %; TMEAN = 300.1 K; HDRY = 4.5259E+05 cm; DAYUBC = ***** DAILY SUMMARY: Day = 241, Simulated Time = 24.0000 hr Node Number -----2 12 25 Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.12007 0.25049 0.25320 Head (cm) = 1.51528E+04 5.15370E+02 4.80551E+02Water Flow (cm) =-2.69203E-04-2.50306E-03 3.82251E-03 Plant Sink (cm) = 4.25702E-04 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 19.7635+ 0.0000+ 0.0000 - 0.0038- 0.0282- 0.0038 = 19.7277 Versus 19.7485 Mass Balance = -2.0844E-02 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.7089 cm, Actual = 0.0038 cm Transpiration: Potential = 2.0276 cm, Actual = 0.0282 cmRHMEAN = 71.3 %; TMEAN = 299.3 K; HDRY = 4.6448E+05 cm; DAYUBC = ***** DAILY SUMMARY: Day = 242, Simulated Time = 24.0000 hr _____ Node Number = 2 12 25 Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.11930 0.25045 0.25320 = 1.57721E+04 5.15863E+02 4.80575E+02 Head (cm) Water Flow (cm) =-1.99243E-04-2.50707E-03 3.82219E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 INFIL RUNOFF TRANS PRESTOR EVAPO DRAIN NEWSTOR STORAGE 19.7485+ 0.0000+ 0.0000 - 0.0027- 0.0210- 0.0038 = 19.7210 Versus 19.7364 Mass Balance = -1.5400E-02 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.6901 cm, Actual = 0.0027 cm Transpiration: Potential = 1.9739 cm, Actual = 0.0210 cm RHMEAN = 72.9 %; TMEAN = 299.5 K; HDRY = 4.3246E+05 cm; DAYUBC = ***** DAILY SUMMARY: Day = 243, Simulated Time = 24.0000 hr _____ Node Number = 2 12 25

Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.11859 0.25045 0.25320 Head (cm) = 1.63845E+04 5.15863E+02 4.80575E+02Water Flow (cm) =-2.04136E-04-2.50699E-03 3.82213E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 19.7364+ 0.0000+ 0.0000 - 0.0053- 0.0173- 0.0038 = 19.7100 Versus 19.7310 Mass Balance = -2.1000E-02 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.6261 cm, Actual = 0.0053 cmTranspiration: Potential = 1.7909 cm, Actual = 0.0173 cm RHMEAN = 69.0 %; TMEAN = 298.4 K; HDRY = 5.0792E+05 cm; DAYUBC = ***** DAILY SUMMARY: Day = 244, Simulated Time = 24.0000 hr -----Node Number = Depth (cm) = 2 12 25 5.00000 30.70000 91.50000 Dependence(cm)=5.0000030.7000091.50000Water (cm3/cm3)=0.118390.250400.25320 Head (cm) = 1.65639E+04 5.16516E+02 4.80618E+02Water Flow (cm) =-2.14062E-04-2.50528E-03 3.82137E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 19.7310+ 0.0000+ 0.0000 - 0.0026- 0.0175- 0.0038 = 19.7071 Versus 19.7246 Mass Balance = -1.7414E-02 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.6631 cm, Actual = 0.0026 cm Transpiration: Potential = 1.8966 cm, Actual = 0.0175 cm RHMEAN = 71.6 %; TMEAN = 298.7 K; HDRY = 4.5761E+05 cm; DAYUBC = ***** DAILY SUMMARY: Day = 245, Simulated Time = 24.0000 hr Node Number = Depth (cm) = 2 12 25 Depth (cm) = 5.00000 30.70000 Water (cm3/cm3) = 0.11839 0.25040 91.50000 0.25320 = 1.65646E+04 5.16516E+02 4.80618E+02Head (cm) Water Flow (cm) =-2.02226E-04-2.50578E-03 3.82123E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 19.7246+ 0.0000+ 0.0000 - 0.0039- 0.0171- 0.0038 = 19.6998 Versus 19.7226 Mass Balance = -2.2873E-02 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.6680 cm, Actual = 0.0039 cm Transpiration: Potential = 1.9106 cm, Actual = 0.0171 cm RHMEAN = 70.8 %; TMEAN = 299.5 K; HDRY = 4.7233E+05 cm; DAYUBC = ***** 

DAILY SUMMARY: Day = 246, Simulated Time = 24.0000 hr _____ Node Number -2 12 25 Depth (cm) = 5.00000 30.70000 91.50000 = 0.11838 Water (cm3/cm3) 0.25040 0.25320 = 1.65773E+04 5.16516E+02 4.80618E+02Head (cm) Water Flow (cm) =-1.99467E-04-2.50578E-03 3.82123E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 19.7226+ 0.0000+ 0.0000 - 0.0048- 0.0149- 0.0038 = 19.6992 Versus 19.7234 Mass Balance = -2.4189E-02 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.5763 cm, Actual = 0.0048 cm Transpiration: Potential = 1.6483 cm, Actual = 0.0149 cm RHMEAN = 68.3 %; TMEAN = 300.4 K; HDRY = 5.2226E+05 cm; DAYUBC = ***** DAILY SUMMARY: Day = 247, Simulated Time = 24.0000 hr ______ Node Number 2 12 25 = 30.70000 Depth (cm) = 5.00000 91.50000 Water (cm3/cm3) = 0.118050.25032 0.25319 = 1.68732E+04 5.17608E+02 4.80712E+02Head (cm) Water Flow (cm) =-2.20066E-04-2.50864E-03 3.81958E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 19.7234+ 0.0000+ 0.0000 - 0.0019- 0.0117- 0.0038 = 19.7059 Versus 19.7190 Mass Balance = -1.3155E-02 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.4350 cm, Actual = 0.0019 cm Transpiration: Potential = 1.2442 cm, Actual = 0.0117 cm RHMEAN = 72.5 %; TMEAN = 299.5 K; HDRY = 4.4130E+05 cm; DAYUBC = ***** DAILY SUMMARY: Day = 248, Simulated Time = 24.0000 hr Node Number 2 12 25 = Depth (cm) 5.00000 30.70000 91.50000 = = 0.11805 0.25032 0.25319 Water (cm3/cm3) = 1.68732E+04 5.17608E+02 4.80712E+02 Head (cm) Water Flow (cm) =-2.24845E-04-2.50854E-03 3.81902E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 19.7190+ 0.0762+ 0.0000 - 0.0729- 0.0086- 0.0038 = 19.7099 Versus 19.7288 Mass Balance = -1.88888E-02 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.2947 cm, Actual = 0.0729 cm Transpiration: Potential = 0.8428 cm, Actual = 0.0086 cm RHMEAN = 76.3 %; TMEAN = 299.8 K; HDRY = 3.6994E+05 cm; DAYUBC = 0

DAILY SUMMARY: Day = 249, Simulated Time = 24.0000 hr Node Number 2 12 == 25 Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.11805 0.25032 0.25319 Head (cm) = 1.68735E+04 5.17608E+02 4.80712E+02Water Flow (cm) =-2.24833E-04-2.50854E-03 3.81902E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 19.7288+ 0.0254+ 0.0000 - 0.0313- 0.0200- 0.0038 = 19.6991 Versus 19.7205 Mass Balance = -2.1435E-02 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.6890 cm, Actual = 0.0313 cm Transpiration: Potential = 1.9707 cm, Actual = 0.0200 cm RHMEAN = 74.7 %; TMEAN = 299.8 K; HDRY = 4.0068E+05 cm; DAYUBC = ***** DAILY SUMMARY: Day = 250, Simulated Time = 24.0000 hr 2 Node Number = 12 25 Depth (cm) = 5.00000 30.70000 Water (cm3/cm3) = 0.15887 0.25032 91.50000 0.25319 Head (cm) = 3.66694E+03 5.17608E+02 4.80712E+02Water Flow (cm) = 3.73494E-02-2.50854E-03 3.81902E-03Plant Sink (cm)  $= 6.08350E-02 \ 0.00000E+00 \ 0.00000E+00$ PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 19.7205+ 0.8746+ 0.1668 - 0.1791- 0.3084- 0.0038 = 20.1038 Versus 20.1233 Mass Balance = -1.9480E-02 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.7309 cm, Actual = 0.1791 cmTranspiration: Potential = 2.0906 cm, Actual = 0.3084 cm RHMEAN = 77.2 %; TMEAN = 299.0 K; HDRY = 3.5476E+05 cm; DAYUBC = 2056 DAILY SUMMARY: Day = 251, Simulated Time = 24.0000 hr = Node Number 2 12 25 Depth (cm) Water (cm3/cm3) = 5.00000 30.70000 0.25032Head (cm) 91.50000 0.25032 0.25319 Head (cm) = 6.03013E+02 5.17608E+02 4.80712E+02Water Flow (cm) = 1.13837E+00-2.50854E-03 3.81902E-03Plant Sink (cm) = 1.39535E-02 0.00000E+00 0.00000E+00 INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR PRESTOR STORAGE 20.1233+ 1.9178+ 0.2666 - 0.0406- 0.1153- 0.0038 = 21.8813 Versus 21.8968 Mass Balance = -1.5530E-02 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.1674 cm, Actual = 0.0406 cm

Transpiration: Potential = 0.4788 cm, Actual = 0.1153 cm RHMEAN = 84.7 %; TMEAN = 294.0 K; HDRY = 2.2805E+05 cm; DAYUBC = 7990

DAILY SUMMARY: Day = 252, Simulated Time = 24.0000 hr Node Number Depth (cm) = 2 12 25 Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.20463 0.25032 0.25319 Head (cm) = 1.37317E+03 5.17608E+02 4.80712E+02Water Flow (cm) = 4.76769E-02-2.50854E-03 3.81902E-03Plant Sink (cm) = 4.94069E-02 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 21.8968+ 0.0254+ 0.0000 - 0.1437- 0.4111- 0.0038 = 21.3636 Versus 21.3698 Mass Balance = -6.1760E-03 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.5807 cm, Actual = 0.1437 cm Transpiration: Potential = 1.6610 cm, Actual = 0.4111 cm RHMEAN = 69.7 %; TMEAN = 292.6 K; HDRY = 4.9471E+05 cm; DAYUBC = Ω DAILY SUMMARY: Day = 253, Simulated Time = 24.0000 hr _____ Node Number = 2 12 25 Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.16308 0.25032 0.25319 = 3.28496E+03 5.17608E+02 4.80712E+02Head (cm) Water Flow (cm) =-2.52494E-02-2.50854E-03 3.81902E-03 Plant Sink (cm) = 4.49867E-02 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 21.3698+ 0.0000+ 0.0000 - 0.1316- 0.3752- 0.0038 = 20.8592 Versus 20.8639 Mass Balance = -4.7665E-03 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.5263 cm, Actual = 0.1316 cm Transpiration: Potential = 1.5052 cm, Actual = 0.3752 cm RHMEAN = 67.4 %; TMEAN = 291.8 K; HDRY = 5.4068E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 254, Simulated Time = 24.0000 hr _____ Node Number Depth (cm) 2 12 25 Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.13690 0.25032 0.25319 Head (cm) = 7.27986E+03 5.17608E+02 4.80712E+02Water Flow (cm) =-1.68107E-02-2.50854E-03 3.81902E-03 Plant Sink (cm) = 2.88412E-02 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 20.8639+ 0.0000+ 0.0000 - 0.1400- 0.3052- 0.0038 = 20.4148 Versus 20.4222

Mass Balance = -7.3185E-03 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.5602 cm, Actual = 0.1400 cm Transpiration: Potential = 1.6023 cm, Actual = 0.3052 cm RHMEAN = 68.9 %; TMEAN = 293.2 K; HDRY = 5.1119E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 255, Simulated Time = 24.0000 hr _____ 2 Node Number 12 2000 C 25 Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.12670 0.25032 0.25319 Head (cm) = 1.10028E+04 5.17608E+02 4.80712E+02Water Flow (cm) =-5.98272E-03-2.50854E-03 3.81902E-03 Plant Sink (cm)  $= 1.08642E-02 \ 0.00000E+00 \ 0.00000E+00$ PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 20.4222+ 0.0000+ 0.0000 - 0.0317- 0.1775- 0.0038 = 20.2092 Versus 20.2166 Mass Balance = -7.4902E-03 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.5374 cm, Actual = 0.0317 cm Transpiration: Potential = 1.5372 cm, Actual = 0.1775 cm RHMEAN = 67.8 %; TMEAN = 294.5 K; HDRY = 5.3329E+05 cm; DAYUBC = ***** DAILY SUMMARY: Day = 256, Simulated Time = 24.0000 hr Node Number = 2 12 25 Depth (cm) = Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.12249 0.25029 0.25319 Head (cm) = 1.34020E+04 5.17977E+02 4.80740E+02Water Flow (cm) =-2.65875E-03-2.51215E-03 3.81850E-03 Plant Sink (cm) = 4.00889E-03 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 20.2166+ 0.0000+ 0.0000 - 0.0032- 0.1118- 0.0038 = 20.0977 Versus 20.1042 Mass Balance = -6.4392E-03 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.5316 cm, Actual = 0.0032 cm Transpiration: Potential = 1.5206 cm, Actual = 0.1118 cm RHMEAN = 69.5 %; TMEAN = 295.1 K; HDRY = 4.9862E+05 cm; DAYUBC = ***** DAILY SUMMARY: Day = 257, Simulated Time = 24.0000 hr _____ 2 Node Number 12 = 25 Depth (cm) = 5.00000 30.70000 Water (cm3/cm3) = 0.12091 0.25021 Depth (cm) 91.50000 0.25318 Head (cm) = 1.45064E+04 5.19006E+02 4.80860E+02Water Flow (cm) =-1.46415E-03-2.50148E-03 3.81661E-03

Plant Sink (cm) = 1.13822E-03 0.00000E+00 0.00000E+00

PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 20.1042+ 0.0000+ 0.0000 - 0.0019- 0.0672- 0.0038 = 20.0313 Versus 20.0376 Mass Balance = -6.3152E-03 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.4499 cm, Actual = 0.0019 cmTranspiration: Potential = 1.2867 cm, Actual = 0.0672 cm RHMEAN = 73.3 %; TMEAN = 296.5 K; HDRY = 4.2502E+05 cm; DAYUBC = ***** DAILY SUMMARY: Day = 258, Simulated Time = 24.0000 hr Node Number=21225Depth (cm)=5.0000030.7000091.50000Water (cm3/cm3)=0.173210.250210.25318 Head (cm) = 2.57321E+03 5.19006E+02 4.80860E+02Water Flow (cm) = 2.04030E-02-2.50265E-03 3.81600E-03 Plant Sink (cm) = 1.72274E-02 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 20.0376+ 0.6604+ 0.0000 - 0.0514- 0.1044- 0.0038 = 20.5383 Versus 20.5559 Mass Balance = -1.7595E-02 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.2077 cm, Actual = 0.0514 cm Transpiration: Potential = 0.5941 cm, Actual = 0.1044 cm RHMEAN = 83.4 %; TMEAN = 294.8 K; HDRY = 2.4902E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 259, Simulated Time = 24.0000 hr _____ Node Number Depth (cm) = 2 12 25 Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.21000 0.25021 0.25318 = 1.25075E+03 5.19006E+02 4.80860E+02Head (cm) Water Flow (cm) = 1.74059E-01-2.50265E-03 3.81600E-03Plant Sink (cm) = 9.22315E-03 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 20.5559+ 0.4572+ 0.0000 - 0.0268- 0.0672- 0.0038 = 20.9153 Versus 20.9390 Mass Balance = -2.3737E-02 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.1084 cm, Actual = 0.0268 cm Transpiration: Potential = 0.3101 cm, Actual = 0.0672 cm RHMEAN = 87.7 %; TMEAN = 295.7 K; HDRY = 1.8050E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 260, Simulated Time = 24.0000 hr _____ Node Number=21225Depth (cm)=5.0000030.7000091.50000Water (cm3/cm3)=0.177760.250210.25318

= 2.32427E+03 5.19006E+02 4.80860E+02 Head (cm) Water Flow (cm) = 6.04837E-02-2.50265E-03 3.81600E-03 Plant Sink (cm) = 2.72269E-02 0.00000E+00 0.00000E+00PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 20.9390+ 0.0254+ 0.0000 - 0.0792- 0.2169- 0.0038 = 20.6644 Versus 20.6889 Mass Balance = -2.4416E-02 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.3200 cm, Actual = 0.0792 cm Transpiration: Potential = 0.9153 cm, Actual = 0.2169 cmRHMEAN = 79.8 %; TMEAN = 298.2 K; HDRY = 3.0846E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 261, Simulated Time = 24.0000 hr ____ Node Number = Depth (cm) = 2 12 25 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.27679 0.25021 0.25318Head (cm) = 4.64238E+02 5.19006E+02 4.80860E+02Water Flow (cm) = 1.60145E+00-2.50265E-03 3.81600E-03Plant Sink (cm) = 1.99619E-02 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 20.6889+ 2.4350+ 0.2066 - 0.0581- 0.1661- 0.0038 = 22.8959 Versus 22.9079 Mass Balance = -1.1984E-02 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.2395 cm, Actual = 0.0581 cm Transpiration: Potential = 0.6849 cm, Actual = 0.1661 cm RHMEAN = 88.2 %; TMEAN = 295.1 K; HDRY = 1.7238E+05 cm; DAYUBC = 7762 DAILY SUMMARY: Day = 262, Simulated Time = 24.0000 hr _____ 

 Node Number
 =
 2
 12
 25

 Depth (cm)
 =
 5.00000
 30.70000
 91.50000

 Water (cm3/cm3)
 =
 0.22977
 0.25021
 0.25318

 Node Number = 2 12 Head (cm) = 9.08392E+02 5.19006E+02 4.80860E+02Water Flow (cm) = 4.37563E-03-2.50265E-03 3.81600E-03 Plant Sink (cm) = 4.53700E-02 0.00000E+00 0.00000E+00PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 22.9079+ 0.0000+ 0.0000 - 0.1320- 0.3775- 0.0038 = 22.3946 Versus 22.4009 Mass Balance = -6.3400E-03 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.5279 cm, Actual = 0.1320 cm Transpiration: Potential = 1.5100 cm, Actual = 0.3775 cmRHMEAN = 70.0 %; TMEAN = 295.4 K; HDRY = 4.8833E+05 cm; DAYUBC = 0 

DAILY SUMMARY: Day = 263, Simulated Time = 24.0000 hr

Node Number = 2 12 25 Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.21961 0.25021 0.25318 Head (cm) = 1.06619E+03 5.19006E+02 4.80860E+02Water Flow (cm) =-3.78997E-02-2.50265E-03 3.81600E-03 Plant Sink (cm) =  $1.40584E-02 \ 0.00000E+00 \ 0.00000E+00$ PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 22.4009+ 0.0000+ 0.0000 - 0.0409- 0.1170- 0.0038 = 22.2393 Versus 22.2415 Mass Balance = -2.2106E-03 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.1636 cm, Actual = 0.0409 cm Transpiration: Potential = 0.4679 cm, Actual = 0.1170 cm RHMEAN = 86.0 %; TMEAN = 292.9 K; HDRY = 2.0731E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 264, Simulated Time = 24.0000 hr _____ Node Number Depth (cm) 2 25 = 12 Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.20096 0.25021 0.25318 Head (cm) = 1.46643E+03 5.19006E+02 4.80860E+02Water Flow (cm) =-3.94440E-02-2.50262E-03 3.81600E-03 Plant Sink (cm) =  $2.40620E-02 \ 0.00000E+00 \ 0.00000E+00$ PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 22.2415+ 0.0000+ 0.0000 - 0.0700- 0.2002- 0.0038 = 21.9674 Versus 21.9707 Mass Balance = -3.2864E-03 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.2800 cm, Actual = 0.0700 cm Transpiration: Potential = 0.8008 cm, Actual = 0.2002 cm RHMEAN = 83.9 %; TMEAN = 294.8 K; HDRY = 2.4002E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 265, Simulated Time = 24.0000 hr Node Number = Depth (cm) = 2 12 25 30.70000 5.00000 91.50000 Water (cm3/cm3) = 0.176210.25021 0.25318 = 2.40491E+03 5.19006E+02 4.80860E+02 Head (cm) Water Flow (cm) =-4.61988E-02-2.50262E-03 3.81600E-03 Plant Sink (cm) = 3.15925E-02 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 21.9707+ 0.0000+ 0.0000 - 0.0919- 0.2629- 0.0038 = 21.6122 Versus 21.6217 Mass Balance = -9.5444E-03 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.3676 cm, Actual = 0.0919 cm Transpiration: Potential = 1.0515 cm, Actual = 0.2629 cm RHMEAN = 81.6 %; TMEAN = 295.7 K; HDRY = 2.7872E+05 cm; DAYUBC = 0

DAILY SUMMARY: Day = 266, Simulated Time = 24.0000 hr _____ Node Number = 2 12 25 Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.141840.25318 0.25021 Head (cm) = 6.11525E+03 5.19006E+02 4.80860E+02Water Flow (cm) =-3.28080E-02-2.50262E-03 3.81600E-03 Plant Sink (cm) = 4.07488E-02 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 21.6217+ 0.0254+ 0.0000 - 0.1551- 0.3871- 0.0038 = 21.1011 Versus 21.1068 Mass Balance = -5.6534E-03 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.6267 cm, Actual = 0.1551 cm Transpiration: Potential = 1.7925 cm, Actual = 0.3871 cm RHMEAN = 72.8 %; TMEAN = 294.8 K; HDRY = 4.3533E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 267, Simulated Time = 24.0000 hr _____ Node Number -----2 12 25 Node Nambel=212Depth (cm)=5.0000030.70000Water (cm3/cm3)=0.128640.2502191.50000 0.25021 0.25318 Head (cm) = 1.01092E+04 5.19006E+02 4.80860E+02Water Flow (cm) =-1.35865E-02-2.50262E-03 3.81600E-03 Plant Sink (cm) = 1.54981E-02 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 21.1068+ 0.0000+ 0.0000 - 0.0738- 0.2445- 0.0038 = 20.7847 Versus 20.7921 Mass Balance = -7.4463E-03 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.5916 cm, Actual = 0.0738 cm Transpiration: Potential = 1.6922 cm, Actual = 0.2445 cm RHMEAN = 66.9 %; TMEAN = 288.4 K; HDRY = 5.5071E+05 cm; DAYUBC = ***** DAILY SUMMARY: Day = 268, Simulated Time = 24.0000 hr _____ Node Number Depth (cm) 25 -----2 12 Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.12469 0.25021 0.25318 Head (cm) = 1.20651E+04 5.19006E+02 4.80862E+02Water Flow (cm) =-6.67158E-03-2.46066E-03 3.81580E-03 Plant Sink (cm) = 4.91896E-03 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 20.7921+ 0.0000+ 0.0000 - 0.0039- 0.1310- 0.0038 = 20.6534 Versus 20.6635 Mass Balance = -1.0113E-02 cm; Time step attempts =***** and successes =*****

Evaporation: Potential = 0.4264 cm, Actual = 0.0039 cm Transpiration: Potential = 1.2197 cm, Actual = 0.1310 cmRHMEAN = 67.5 %; TMEAN = 286.8 K; HDRY = 5.3856E+05 cm; DAYUBC = ***** DAILY SUMMARY: Day = 269, Simulated Time = 24.0000 hr _____ Node Number 2 12 25 Depth (cm) = 5.00000 30.70000 91.50000Water (cm3/cm3) = 0.12261 0.25019 0.25317= 1.33237E+04 5.19388E+02 4.80960E+02 Head (cm) Water Flow (cm) =-4.66403E-03-2.30399E-03 3.81414E-03 Plant Sink (cm) = 2.42812E-03 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 20.6635+ 0.0000+ 0.0000 - 0.0025- 0.1044- 0.0038 = 20.5528 Versus 20.5581 Mass Balance = -5.2738E-03 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.4011 cm, Actual = 0.0025 cm Transpiration: Potential = 1.1474 cm, Actual = 0.1044 cm RHMEAN = 71.0 %; TMEAN = 287.9 K; HDRY = 4.7031E+05 cm; DAYUBC = ***** DAILY SUMMARY: Day = 270, Simulated Time = 24.0000 hr Node Number = 12 2 25 Depth (cm) Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.12129 0.25019 0.25317 Head (cm) = 1.42270E+04 5.19388E+02 4.80960E+02Water Flow (cm) =-3.23369E-03-2.28229E-03 3.81372E-03 Plant Sink (cm) =  $1.44252E-03 \ 0.00000E+00 \ 0.00000E+00$ PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 20.5581+ 0.0000+ 0.0000 - 0.0053- 0.1031- 0.0038 = 20.4458 Versus 20.4529 Mass Balance = -7.0210E-03 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.4716 cm, Actual = 0.0053 cm Transpiration: Potential = 1.3490 cm, Actual = 0.1031 cm RHMEAN = 67.3 %; TMEAN = 290.4 K; HDRY = 5.4202E+05 cm; DAYUBC = ***** DAILY SUMMARY: Day = 271, Simulated Time = 24.0000 hr ______ 2 12 25 Node Number === Depth (cm) = 5.00000 30.70000 91.50000Water (cm3/cm3) = 0.12059 0.25017 0.25317Head (cm) = 1.47495E+04 5.19586E+02 4.80983E+02Water Flow (cm) =-2.19032E-03-2.30708E-03 3.81341E-03 Plant Sink (cm) = 5.43306E-04 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE

20.4529 + 0.0000 + 0.0000 - 0.0035 - 0.0814 - 0.0038 = 20.3641 Versus 20.3702 Mass Balance = -6.0349E-03 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.4449 cm, Actual = 0.0035 cm Transpiration: Potential = 1.2724 cm, Actual = 0.0814 cm RHMEAN = 68.5 %; TMEAN = 291.5 K; HDRY = 5.1815E+05 cm; DAYUBC = ***** DAILY SUMMARY: Day = 272, Simulated Time = 24.0000 hr _____ 2 Node Number = 12 25 Depth (cm) = 5.00000 30.70000 Water (cm3/cm3) = 0.12010 0.25017 91.50000 0.25017 0.25317 Head (cm) = 1.51285E+04 5.19586E+02 4.80983E+02Water Flow (cm) =-1.54298E-03-2.30805E-03 3.81338E-03 Plant Sink (cm) =  $1.23074E-04 \ 0.00000E+00 \ 0.00000E+00$ PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 20.3702+ 0.0000+ 0.0000 - 0.0043- 0.0763- 0.0038 = 20.2857 Versus 20.2923 Mass Balance = -6.6166E-03 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.4884 cm, Actual = 0.0043 cm Transpiration: Potential = 1.3971 cm, Actual = 0.0763 cmRHMEAN = 67.5 %; TMEAN = 290.1 K; HDRY = 5.3845E+05 cm; DAYUBC = ***** DAILY SUMMARY: Day = 273, Simulated Time = 24.0000 hr _____ Node Number = Depth (cm) = 2 12 25 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.11954 0.25017 0.25317= 1.55782E+04 5.19586E+02 4.80983E+02Head (cm) Water Flow (cm) =-1.17225E-03-2.30805E-03 3.81338E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 20.2923+ 0.0000+ 0.0000 - 0.0041- 0.0573- 0.0038 = 20.2271 Versus 20.2345 Mass Balance = -7.3757E-03 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.4107 cm, Actual = 0.0041 cm Transpiration: Potential = 1.1747 cm, Actual = 0.0573 cm RHMEAN = 67.3 %; TMEAN = 289.3 K; HDRY = 5.4213E+05 cm; DAYUBC = ***** DAILY SUMMARY: Day = 274, Simulated Time = 24.0000 hr _____ 2 12 Node Number = 25 Depth (cm) = 5.00000 30.70000 Water (cm3/cm3) = 0.11890 0.25017 91.50000 0.25017 0.25317 Head (cm) = 1.61151E+04 5.19586E+02 4.80983E+02Water Flow (cm) =-1.00060E-03-2.30805E-03 3.81338E-03

Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 20.2345+ 0.0000+ 0.0000 - 0.0046- 0.0525- 0.0038 = 20.1736 Versus 20.1811 Mass Balance = -7.4329E-03 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.4071 cm, Actual = 0.0046 cm Transpiration: Potential = 1.1644 cm, Actual = 0.0525 cm RHMEAN = 66.0 %; TMEAN = 289.8 K; HDRY = 5.6861E+05 cm; DAYUBC = ***** DAILY SUMMARY: Day = 275, Simulated Time = 24.0000 hr -----Node Number = Depth (cm) = 2 12 25 5.00000 30.70000 91.50000 = 0.11859 0.25017 0.25317 Water (cm3/cm3) Head (cm) = 1.63843E+04 5.19586E+02 4.80983E+02Water Flow (cm) =-7.99225E-04-2.30805E-03 3.81338E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 20.1811+ 0.0254+ 0.0000 - 0.0289- 0.0644- 0.0038 = 20.1094 Versus 20.1154 Mass Balance = -5.9986E-03 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.5737 cm, Actual = 0.0289 cmTranspiration: Potential = 1.6410 cm, Actual = 0.0644 cm RHMEAN = 65.1 %; TMEAN = 290.9 K; HDRY = 5.8744E+05 cm; DAYUBC = ***** DAILY SUMMARY: Day = 276, Simulated Time = 24.0000 hr Node Number = 2 12 25 Depth (cm) = 5.00000 30.70000 Water (cm3/cm3) = 0.11858 0.25017 91.50000 0.25317 = 1.63955E+04 5.19586E+02 4.80983E+02 Head (cm) Water Flow (cm) =-5.38995E-04-2.30805E-03 3.81338E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 20.1154+ 0.0000+ 0.0000 - 0.0051- 0.0613- 0.0038 = 20.0451 Versus 20.0571 Mass Balance = -1.2007E-02 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.6695 cm, Actual = 0.0051 cm Transpiration: Potential = 1.9150 cm, Actual = 0.0613 cm RHMEAN = 63.0 %; TMEAN = 294.0 K; HDRY = 6.3405E+05 cm; DAYUBC = ***** DAILY SUMMARY: Day = 277, Simulated Time = 24.0000 hr Node Number = 2 12 25 = 5.00000 30.70000 91.50000 Depth (cm)

Water (cm3/cm3) = 0.11789 0.24999 0.25313Head (cm) = 1.70243E+04 5.21971E+02 4.81377E+02Water Flow (cm) =-4.23713E-04-2.44572E-03 3.80905E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 INFIL RUNOFF EVAPO TRANS PRESTOR DRAIN NEWSTOR STORAGE 20.0571+ 0.0000+ 0.0000 - 0.0003- 0.0454- 0.0038 = 20.0076 Versus 20.0087 Mass Balance = -1.1101E-03 cm; Time step attempts =23636 and successes =23636 Evaporation: Potential = 0.5866 cm, Actual = 0.0003 cm Transpiration: Potential = 1.6778 cm, Actual = 0.0454 cm RHMEAN = 73.2 %; TMEAN = 295.9 K; HDRY = 4.2692E+05 cm; DAYUBC = 21034 DAILY SUMMARY: Day = 278, Simulated Time = 24.0000 hr ______ Node Number 12 = 2 25 Depth (cm) Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.12076 0.24999 0.25313 Head (cm) = 1.46157E+04 5.21972E+02 4.81377E+02Water Flow (cm) =-2.57833E-04-2.47745E-03 3.80489E-03 Plant Sink (cm) = 1.16340E-05 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 20.0087+ 0.2286+ 0.0000 - 0.0605- 0.0183- 0.0038 = 20.1547 Versus 20.1654 Mass Balance = -1.0763E-02 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.2445 cm, Actual = 0.0605 cm Transpiration: Potential = 0.6992 cm, Actual = 0.0183 cm RHMEAN = 79.2 %; TMEAN = 286.2 K; HDRY = 3.2031E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 279, Simulated Time = 24.0000 hr _____ Node Number = Depth (cm) = 2 12 25 Depth (cm) = 5.00000 30.70000 Water (cm3/cm3) = 0.12084 0.24999 91.50000 0.24999 0.25313 = 1.45590E+04 5.21972E+02 4.81377E+02Head (cm) Water Flow (cm) =-1.54986E-04-2.47746E-03 3.80489E-03 Plant Sink (cm) =  $4.54854E-04 \ 0.00000E+00 \ 0.00000E+00$ DRAIN NEWSTOR PRESTOR INFIL RUNOFF EVAPO TRANS STORAGE 20.1654+ 0.0000+ 0.0000 - 0.0863- 0.0290- 0.0038 = 20.0464 Versus 20.0537 Mass Balance = -7.3776E-03 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.3450 cm, Actual = 0.0863 cm Transpiration: Potential = 0.9869 cm, Actual = 0.0290 cm RHMEAN = 63.6 %; TMEAN = 283.4 K; HDRY = 6.1977E+05 cm; DAYUBC = 0 

DAILY SUMMARY: Day = 280, Simulated Time = 24.0000 hr _____ Node Number = 2 12 25 5.00000 30.70000 Depth (cm) = 91.50000 = 0.12011 Water (cm3/cm3) 0.24999 0.25313 Head (cm) = 1.51175E+04 5.21972E+02 4.81377E+02 Water Flow (cm) =-1.76903E-04-2.47746E-03 3.80489E-03 Plant Sink (cm) = 2.92058E-04 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 20.0537+ 0.0000+ 0.0000 - 0.0723- 0.0350- 0.0038 = 19.9427 Versus 19.9521 Mass Balance = -9.4814E-03 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.4498 cm, Actual = 0.0723 cm Transpiration: Potential = 1.2865 cm, Actual = 0.0350 cm RHMEAN = 65.1 %; TMEAN = 284.5 K; HDRY = 5.8882E+05 cm; DAYUBC = ***** DAILY SUMMARY: Day = 281, Simulated Time = 24.0000 hr _____ Node Number _ 2 25 12 Depth (cm) Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.11923 0.24999 0.25313 Head (cm) = 1.58344E+04 5.21972E+02 4.81377E+02Water Flow (cm) =-1.96846E-04-2.47746E-03 3.80489E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 19.9521+ 0.0000+ 0.0000 - 0.0044- 0.0462- 0.0038 = 19.8977 Versus 19.9136 Mass Balance = -1.5821E-02 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.7227 cm, Actual = 0.0044 cm Transpiration: Potential = 2.0672 cm, Actual = 0.0462 cm RHMEAN = 64.8 %; TMEAN = 289.5 K; HDRY = 5.9414E+05 cm; DAYUBC = ***** DAILY SUMMARY: Day = 282, Simulated Time = 24.0000 hr Node Number ..... 2 12 25 Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.210780.24999 0.25313 Head (cm) = 1.23423E+03 5.21972E+02 4.81377E+02Water Flow (cm) = 2.69388E-01-2.47746E-03 3.80489E-03Plant Sink (cm) = 2.99061E-02 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 19.9136+ 1.2318+ 0.2668 - 0.0870- 0.2090- 0.0038 = 20.8456 Versus 20.8535 Mass Balance = -7.9155E-03 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.3551 cm, Actual = 0.0870 cm Transpiration: Potential = 1.0156 cm, Actual = 0.2090 cm RHMEAN = 76.1 %; TMEAN = 294.0 K; HDRY = 3.7383E+05 cm; DAYUBC = 8497

DAILY SUMMARY: Day = 283, Simulated Time = 24.0000 hr _____ Node Number == 2 12 25 Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.31845 0.25029 0.25313 Head (cm)  $= 2.60304\pm025.17940\pm024.81377\pm02$ Water Flow (cm) = 3.22834E+00-2.16070E-03 3.80489E-03Plant Sink (cm) = 1.21679E-03 0.00000E+00 0.00000E+00 INFIL RUNOFF EVAPO TRANS PRESTOR DRAIN NEWSTOR STORAGE 20.8535+ 3.9712+ 1.4644 - 0.0035- 0.0101- 0.0038 = 24.8072 Versus 24.8136 Mass Balance = -6.3496E-03 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.0151 cm, Actual = 0.0035 cm Transpiration: Potential = 0.0431 cm, Actual = 0.0101 cm RHMEAN = 87.4 %; TMEAN = 292.9 K; HDRY = 1.8457E+05 cm; DAYUBC = 38266 DAILY SUMMARY: Day = 284, Simulated Time = 24.0000 hr ______ Node Number 2 12 25 Depth (cm) = 5.00000 30.70000 Water (cm3/cm3) = 0.32694 0.25197 91.50000 0.25197 0.25310 Head (cm) = 2.29206E+02 4.95978E+02 4.81822E+02Water Flow (cm) = 5.52926E-01 5.40500E-03 3.80112E-03Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 24.8136+ 0.6096+ 0.0000 - 0.0000- 0.0000- 0.0038 = 25.4193 Versus 25.4199 Mass Balance = -5.5885E-04 cm; Time step attempts =40096 and successes =40096 Evaporation: Potential = 0.0000 cm, Actual = 0.0000 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm 0 RHMEAN = 97.3 %; TMEAN = 288.4 K; HDRY = 3.7054E+04 cm; DAYUBC = DAILY SUMMARY: Day = 285, Simulated Time = 24.0000 hr Node Number = 2 12 25 Depth (cm) Depth (cm) = 5.00000 Water (cm3/cm3) = 0.35766 5.00000 30.70000 91.50000 0.25584 0.25305 Head (cm) = 1.33373E+02 4.49129E+02 4.82427E+02Water Flow (cm)  $= 9.02154E-01 \ 2.24555E-02 \ 3.78940E-03$ Plant Sink (cm) = 2.61981E-02 0.00000E+00 0.00000E+00 TRANS DRAIN NEWSTOR PRESTOR INFIL RUNOFF EVAPO STORAGE 25.4199+ 1.3099+ 0.1379 - 0.0762- 0.2180- 0.0038 = 26.4318 Versus 26.4321 Mass Balance = -2.7466E-04 cm; Time step attempts = 2993 and successes = 2993 Evaporation: Potential = 0.3111 cm, Actual = 0.0762 cm

Transpiration: Potential = 0.8897 cm, Actual = 0.2180 cm RHMEAN = 82.4 %; TMEAN = 291.2 K; HDRY = 2.6565E+05 cm; DAYUBC = 369

DAILY SUMMARY: Day = 286, Simulated Time = 24.0000 hr ______ Node Number Depth (cm) 2 = 12 25 Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.35073 0.25690 0.25299 Head (cm) = 1.53294E+02 4.37022E+02 4.83147E+02Water Flow (cm) =-7.63307E-02 2.49499E-02 3.77483E-03 Plant Sink (cm) = 1.68280E-02 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 26.4321+ 0.0000+ 0.0000 - 0.0490- 0.1400- 0.0038 = 26.2393 Versus 26.2393 Mass Balance = 0.0000E+00 cm; Time step attempts = 96 and successes = 96 Evaporation: Potential = 0.1958 cm, Actual = 0.0490 cm Transpiration: Potential = 0.5601 cm, Actual = 0.1400 cm RHMEAN = 79.9 %; TMEAN = 287.6 K; HDRY = 3.0708E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 287, Simulated Time = 24.0000 hr ______ Node Number = Depth (cm) = 2 12 25 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.33774 0.25710 0.25293= 1.93008E+02 4.34806E+02 4.83882E+02Head (cm) Water Flow (cm) =-1.44349E-01 2.09732E-02 3.75923E-03 Plant Sink (cm) = 3.17625E-02 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 26.2393+ 0.0000+ 0.0000 - 0.0924- 0.2643- 0.0038 = 25.8789 Versus 25.8789 Mass Balance = -3.8147E-06 cm; Time step attempts = 96 and successes = 96Evaporation: Potential = 0.3696 cm, Actual = 0.0924 cm Transpiration: Potential = 1.0571 cm, Actual = 0.2643 cm RHMEAN = 64.0 %; TMEAN = 287.0 K; HDRY = 6.1142E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 288, Simulated Time = 24.0000 hr ______ 2 Node Number === 12 25 Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.32410 0.25682 0.25288 Head (cm) = 2.39328E+02 4.37902E+02 4.84497E+02Water Flow (cm) =-1.53809E-01 1.57459E-02 3.74471E-03 Plant Sink (cm) = 3.36597E-02 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE

25.8789+ 0.0000+ 0.0000 - 0.0979- 0.2801- 0.0037 = 25.4972 Versus 25.4972 Mass Balance = -1.9073E-06 cm; Time step attempts = 96 and successes = 96 Evaporation: Potential = 0.3917 cm, Actual = 0.0979 cm Transpiration: Potential = 1.1202 cm, Actual = 0.2801 cm RHMEAN = 68.6 %; TMEAN = 285.9 K; HDRY = 5.1747E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 289, Simulated Time = 24.0000 hr 2 = 12 Node Number 25 Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.31292 0.25639 0.25286 Head (cm) = 2.81997E+024.42820E+024.84836E+02Water Flow (cm) =-1.32650E-01 1.14962E-02 3.73444E-03 Plant Sink (cm)  $= 2.84645E-02 \ 0.00000E+00 \ 0.00000E+00$ PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 25.4972+ 0.0000+ 0.0000 - 0.0828- 0.2368- 0.0037 = 25.1738 Versus 25.1738 Mass Balance = 0.0000E+00 cm; Time step attempts = 96 and successes = 96 Evaporation: Potential = 0.3312 cm, Actual = 0.0828 cm Transpiration: Potential = 0.9473 cm, Actual = 0.2368 cm RHMEAN = 60.6 %; TMEAN = 282.3 K; HDRY = 6.8753E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 290, Simulated Time = 24.0000 hr Node Number = Depth (cm) = 12 2 25 Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.29207 0.25587 0.25286 Head (cm) = 3.76997E+02 4.48774E+02 4.84804E+02Water Flow (cm) =-1.82650E-01 8.38723E-03 3.73114E-03 Plant Sink (cm) = 4.46518E-02 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 25.1738+ 0.0000+ 0.0000 - 0.1299- 0.3715- 0.0037 = 24.6687 Versus 24.6687 Mass Balance = -9.5367E-06 cm; Time step attempts = 96 and successes = 96 Evaporation: Potential = 0.5196 cm, Actual = 0.1299 cm Transpiration: Potential = 1.4861 cm, Actual = 0.3715 cm RHMEAN = 57.2 %; TMEAN = 284.8 K; HDRY = 7.6505E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 291, Simulated Time = 24.0000 hr ______ 2 12 Node Number = 25 Note Namber212Depth (cm)=5.0000030.70000Water (cm3/cm3)=0.256580.2553591.50000 0.25288 = 6.13540E+02 4.54743E+02 4.84543E+02Head (cm) Water Flow (cm) =-2.27359E-01 5.41021E-03 3.73541E-03

Plant Sink (cm) =  $6.57576E-02 \ 0.00000E+00 \ 0.00000E+00$ PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 24.6687+ 0.0000+ 0.0000 - 0.1913- 0.5471- 0.0037 = 23.9265 Versus 23.9258 Mass Balance = 7.9536E-04 cm; Time step attempts =65907 and successes =65907 Evaporation: Potential = 0.7651 cm, Actual = 0.1913 cm Transpiration: Potential = 2.1885 cm, Actual = 0.5471 cm RHMEAN = 56.2 %; TMEAN = 290.9 K; HDRY = 7.9086E+05 cm; DAYUBC =  $\cap$ DAILY SUMMARY: Day = 292, Simulated Time = 24.0000 hr _____ = Node Number 2 12 25 Note Manbel=21223Depth (cm)=5.0000030.7000091.50000Water (cm3/cm3)=0.267210.255160.25288= 5.29324E+02 4.57022E+02 4.84543E+02Head (cm) Water Flow (cm) =-1.33649E-01 2.74390E-03 3.73766E-03 Plant Sink (cm) = 6.07836E-03 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 23.9258+ 0.0000+ 0.0000 - 0.0177- 0.0506- 0.0037 = 23.8538 Versus 23.8545 Mass Balance = -7.6866E-04 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.0707 cm, Actual = 0.0177 cm Transpiration: Potential = 0.2023 cm, Actual = 0.0506 cm RHMEAN = 85.6 %; TMEAN = 287.0 K; HDRY = 2.1259E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 293, Simulated Time = 24.0000 hr _____ Node Number Depth (cm) 12 = 2 25 Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.24934 0.25514 0.25288 Head (cm)  $= 6.79818E+02 \ 4.57178E+02 \ 4.84543E+02$ Water Flow (cm) =-1.07577E-01 2.27550E-03 3.73766E-03 Plant Sink (cm) =  $3.17445E-02 \ 0.00000E+00 \ 0.00000E+00$ PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 23.8545+ 0.0000+ 0.0000 - 0.0923- 0.2641- 0.0037 = 23.4943 Versus 23.4951 Mass Balance = -7.6103E-04 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.3694 cm, Actual = 0.0923 cm Transpiration: Potential = 1.0565 cm, Actual = 0.2641 cm RHMEAN = 77.8 %; TMEAN = 291.2 K; HDRY = 3.4484E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 294, Simulated Time = 24.0000 hr ______ Node Number 2 25 = 12 = 5.00000 30.70000 91.50000 Depth (cm)

Water (cm3/cm3) = 0.21860 0.25493 0.25288Head (cm) = 1.08381E+03 4.59713E+02 4.84543E+02Water Flow (cm) =-1.31542E-01 1.65239E-03 3.73766E-03 Plant Sink (cm) = 4.86715E-02 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 23.4951+ 0.0000+ 0.0000 - 0.1416- 0.4050- 0.0037 = 22.9448 Versus 22.9460 Mass Balance = -1.2054E-03 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.5663 cm, Actual = 0.1416 cm Transpiration: Potential = 1.6199 cm, Actual = 0.4050 cm RHMEAN = 70.5 %; TMEAN = 294.3 K; HDRY = 4.7883E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 295, Simulated Time = 24.0000 hr _____ Node Number = 2 12 25 Depth (cm) Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.20716 0.25471 0.25288 Head (cm) = 1.31355E+03 4.62342E+02 4.84543E+02Water Flow (cm) =-1.24115E-01 1.43830E-04 3.73766E-03 Plant Sink (cm) = 2.80896E-02 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 22.9460+ 0.0000+ 0.0000 - 0.0817- 0.2337- 0.0037 = 22.6268 Versus 22.6305 Mass Balance = -3.6850E-03 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.3268 cm, Actual = 0.0817 cm Transpiration: Potential = 0.9349 cm, Actual = 0.2337 cm RHMEAN = 81.6 %; TMEAN = 293.7 K; HDRY = 2.7855E+05 cm; DAYUBC = DAILY SUMMARY: Day = 296, Simulated Time = 24.0000 hr ______ Node Number=21225Depth (cm)=5.0000030.7000091.50000Water (cm3/cm3)=0.179220.254690.25288 Head (cm)  $= 2.25154\pm03 4.62480\pm02 4.84543\pm02$ Water Flow (cm) =-9.97489E-02 1.20427E-04 3.73766E-03 Plant Sink (cm) = 4.25804E-02 0.00000E+00 0.00000E+00 INFIL RUNOFF PRESTOR EVAPO TRANS DRAIN NEWSTOR STORAGE 22.6305+ 0.0000+ 0.0000 - 0.1239- 0.3543- 0.0037 = 22.1486 Versus 22.1542 Mass Balance = -5.5332E-03 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.4955 cm, Actual = 0.1239 cm Transpiration: Potential = 1.4172 cm, Actual = 0.3543 cm RHMEAN = 71.4 %; TMEAN = 295.1 K; HDRY = 4.6185E+05 cm; DAYUBC = 0 

DAILY SUMMARY: Day = 297, Simulated Time = 24.0000 hr

Node Number=21225Depth (cm)=5.0000030.7000091.50000Water (cm3/cm3)=0.151480.254570.25288 Head (cm) = 4.51070E+03 4.63954E+02 4.84543E+02 Water Flow (cm) =-7.18140E-02-1.52311E-04 3.73766E-03 Plant Sink (cm) =  $4.02590E-02 \ 0.00000E+00 \ 0.00000E+00$ PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 22.1542+ 0.0000+ 0.0000 - 0.1333- 0.3562- 0.0037 = 21.6610 Versus 21.6657 Mass Balance = -4.6825E-03 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.5331 cm, Actual = 0.1333 cm Transpiration: Potential = 1.5247 cm, Actual = 0.3562 cm RHMEAN = 65.4 %; TMEAN = 287.0 K; HDRY = 5.8172E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 298, Simulated Time = 24.0000 hr ______ = 2 Node Number 12 2.5 Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.14018 0.25431 0.25288 = 6.47349E+03 4.67080E+02 4.84543E+02 Head (cm) Water Flow (cm) =-4.54749E-02-8.65576E-04 3.73766E-03 Plant Sink (cm) = 2.01466E-02 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 21.6657+ 0.0000+ 0.0000 - 0.0722- 0.2305- 0.0037 = 21.3592 Versus 21.3632 Mass Balance = -4.0493E-03 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.4260 cm, Actual = 0.0722 cmTranspiration: Potential = 1.2184 cm, Actual = 0.2305 cm RHMEAN = 46.6 %; TMEAN = 283.4 K; HDRY = 1.0464E+06 cm; DAYUBC = ***** DAILY SUMMARY: Day = 299, Simulated Time = 24.0000 hr _____ Node Number = Depth (cm) = 2 25 12 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.13451 0.25419 0.25288= 7.96549E+03 4.68450E+02 4.84495E+02Head (cm) Water Flow (cm) =-3.09779E-02-1.08110E-03 3.73853E-03 Plant Sink (cm) = 1.11298E-02 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 21.3632+ 0.0000+ 0.0000 - 0.0053- 0.1647- 0.0037 = 21.1896 Versus 21.1936 Mass Balance = -4.0188E-03 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.3490 cm, Actual = 0.0053 cm Transpiration: Potential = 0.9981 cm, Actual = 0.1647 cm RHMEAN = 51.1 %; TMEAN = 282.6 K; HDRY = 9.2099E+05 cm; DAYUBC = *****

DAILY SUMMARY: Day = 300, Simulated Time = 24.0000 hr _____ Node Number = Depth (cm) = 2 12 25 5.00000 30.70000 91.50000 = 0.13045 0.25409 0.25289 Water (cm3/cm3) Head (cm) = 9.36335E+03 4.69711E+02 4.84466E+02Water Flow (cm) =-2.22298E-02-1.40532E-03 3.73918E-03 Plant Sink (cm) = 7.50277E-03 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 21.1936+ 0.0000+ 0.0000 - 0.0052- 0.1406- 0.0037 = 21.0441 Versus 21.0480 Mass Balance = -3.8643E-03 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.3283 cm, Actual = 0.0052 cm Transpiration: Potential = 0.9391 cm, Actual = 0.1406 cmRHMEAN = 53.9 %; TMEAN = 281.8 K; HDRY = 8.4799E+05 cm; DAYUBC = ***** DAILY SUMMARY: Day = 301, Simulated Time = 24.0000 hr ______ Node Number 2 12 25 = Depth (cm) = 5.00000 30.70000 91.50000Water (cm3/cm3) = 0.12372 0.25407 0.25289Head (cm) = 1.26302E+04 4.69883E+02 4.84466E+02Water Flow (cm) =-1.19866E-02-1.59293E-03 3.73922E-03 Plant Sink (cm) = 8.06684E-03 0.00000E+00 0.00000E+00 DRAIN NEWSTOR PRESTOR INFIL RUNOFF EVAPO TRANS STORAGE 21.0480+ 0.0000+ 0.0000 - 0.0072- 0.2254- 0.0037 = 20.8116 Versus 20.8207 Mass Balance = -9.0790E-03 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.6644 cm, Actual = 0.0072 cm Transpiration: Potential = 1.9003 cm, Actual = 0.2254 cmRHMEAN = 48.7 %; TMEAN = 285.7 K; HDRY = 9.8559E+05 cm; DAYUBC = ***** DAILY SUMMARY: Day = 302, Simulated Time = 24.0000 hr ______ Node Number ...... 2 12 25 Depth (cm) = 5.00000 30.70000 91.50000 SolutionSolutionSolutionSolutionSolutionWater (cm3/cm3)=0.120950.254060.25289Head (cm) = 1.44807E+04 4.70071E+02 4.84466E+02Water Flow (cm) =-4.58217E-03-1.79183E-03 3.73922E-03 Plant Sink (cm) = 2.41128E-03 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 20.8207+ 0.0000+ 0.0000 - 0.0064- 0.1606- 0.0037 = 20.6499 Versus 20.6573 Mass Balance = -7.3833E-03 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.6622 cm, Actual = 0.0064 cm Transpiration: Potential = 1.8941 cm, Actual = 0.1606 cm

RHMEAN = 47.9 %; TMEAN = 289.5 K; HDRY = 1.0097E+06 cm; DAYUBC = ****

DAILY SUMMARY: Day = 303, Simulated Time = 24.0000 hr _____ Node Number = 2 12 25 Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.11995 0.25389 0.25290 Head (cm) = 1.52409E+04 4.72153E+02 4.84340E+02Water Flow (cm) =-2.43350E-03-1.64477E-03 3.74146E-03 Plant Sink (cm) = 1.99365E-04 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 20.6573+ 0.0000+ 0.0000 - 0.0041- 0.0961- 0.0037 = 20.5534 Versus 20.5565 Mass Balance = -3.1414E-03 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.5063 cm, Actual = 0.0041 cm Transpiration: Potential = 1.4480 cm, Actual = 0.0961 cm RHMEAN = 55.3 %; TMEAN = 287.6 K; HDRY = 8.1282E+05 cm; DAYUBC = ***** DAILY SUMMARY: Day = 304, Simulated Time = 24.0000 hr ____ Node Number 2 12 25 = Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.11936 0.25350 0.25293 Depth (cm) Head (cm) = 1.57251E+04 4.76894E+02 4.83967E+02Water Flow (cm) =-1.66270E-03-1.76339E-03 3.74730E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 20.5565+ 0.0000+ 0.0000 - 0.0018- 0.0901- 0.0037 = 20.4608 Versus 20.4639 Mass Balance = -3.0842E-03 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.5523 cm, Actual = 0.0018 cm Transpiration: Potential = 1.5796 cm, Actual = 0.0901 cm RHMEAN = 65.8 %; TMEAN = 289.8 K; HDRY = 5.7344E+05 cm; DAYUBC = ***** DAILY SUMMARY: Day = 305, Simulated Time = 24.0000 hr ______ Node Number Depth (cm) = 2 12 25 Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.11885 0.25308 0.25297 91.50000 Head (cm) = 1.61627E+04 4.81998E+02 4.83395E+02Water Flow (cm) =-1.29711E-03-1.96088E-03 3.75723E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 20.4639+ 0.0000+ 0.0000 - 0.0010- 0.0590- 0.0038 = 20.4002 Versus 20.4024

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Mass Balance = -2.2240E-03 cm; Time step attempts =71742 and successes =71742
  Evaporation: Potential = 0.3968 cm, Actual = 0.0010 cm
Transpiration: Potential = 1.1349 cm, Actual = 0.0590 cm
RHMEAN = 73.2 %; TMEAN = 294.3 K; HDRY = 4.2807E+05 cm; DAYUBC = 69191
DAILY SUMMARY: Day = 306, Simulated Time = 24.0000 hr
_____
Node Number
                        2
                                   12
                                               25
                =
Depth (cm) = 5.00000 30.70000
Water (cm3/cm3) = 0.11854 0.25260
                                            91.50000
                                             0.25305
                = 1.64318E+04 4.88061E+02 4.82473E+02
Head (cm)
Water Flow (cm) =-1.54921E-03-2.11445E-03 3.77118E-03
Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00
 PRESTOR INFIL RUNOFF
                          EVAPO TRANS
                                          DRAIN NEWSTOR
                                                                    STORAGE
 20.4024+ 0.0000+ 0.0000 - 0.0000- 0.0000- 0.0038 = 20.3986 Versus 20.3986
Mass Balance = 1.5259E-05 cm; Time step attempts = 2595 and successes = 2595
  Evaporation: Potential = 0.0000 cm, Actual = 0.0000 cm
Transpiration: Potential = 0.0000 \text{ cm}, Actual = 0.0000 \text{ cm}
RHMEAN = 93.0 %; TMEAN = 291.5 K; HDRY = 9.9725E+04 cm; DAYUBC =
                                                                     0
 DAILY SUMMARY: Day = 307, Simulated Time = 24.0000 hr
------
Node Number =
Depth (cm) =
                       2
                                  12
                                               25
                      5.00000
                                 30.70000
                                             91.50000
               = 0.32045 0.25287 0.25305
Water (cm3/cm3)
                = 2.52717E+02 4.84606E+02 4.82473E+02
Head (cm)
Water Flow (cm) = 3.07540E+00-1.88572E-03 3.78138E-03
Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00
 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR
                                                                   STORAGE
 20.3986+ 4.5614+ 2.4997 - 0.0000- 0.0000- 0.0038 = 24.9563 Versus 24.9626
Mass Balance = -6.3648E-03 cm; Time step attempts =***** and successes =*****
  Evaporation: Potential = 0.0000 cm, Actual = 0.0000 cm
Transpiration: Potential = 0.0000 \text{ cm}, Actual = 0.0000 \text{ cm}
RHMEAN = 99.7 %; TMEAN = 289.5 K; HDRY = 4.5820E+03 cm; DAYUBC = 56227
DAILY SUMMARY: Day = 308, Simulated Time = 24.0000 hr
_____

      Node Number
      =
      2
      12
      12

      Depth (cm)
      =
      5.00000
      30.70000
      91.50000

      Water (cm3/cm3)
      =
      0.30601
      0.25362
      0.25312

      Head (cm)
      =
      3.10994E+02
      4.75371E+02
      4.81545E+02

Node Number
                =
                        2
                                  12
                                               25
Water Flow (cm) = 4.01518E-02 3.80322E-03 3.79112E-03
Plant Sink (cm) = 8.57159E-03 0.00000E+00 0.00000E+00
```

PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 24.9626+ 0.0000+ 0.0000 - 0.0249- 0.0713- 0.0038 = 24.8626 Versus 24.8626 Mass Balance = 0.0000E+00 cm; Time step attempts = 2595 and successes = 2595 Evaporation: Potential = 0.0997 cm, Actual = 0.0249 cm Transpiration: Potential = 0.2853 cm, Actual = 0.0713 cm RHMEAN = 86.9 %; TMEAN = 290.7 K; HDRY = 1.9254E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 309, Simulated Time = 24.0000 hr = Node Number 2 12 25 5.00000 30.70000 91.50000 Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.30082 0.25381 0.25319 = 3.34286E+02 4.73031E+02 4.80692E+02Head (cm) Water Flow (cm) =-4.36517E-02 5.19628E-03 3.81067E-03 Plant Sink (cm) = 1.08391E-02 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 24.8626+ 0.0000+ 0.0000 - 0.0315- 0.0902- 0.0038 = 24.7370 Versus 24.7370 Mass Balance = 0.0000E+00 cm; Time step attempts = 96 and successes = 96 Evaporation: Potential = 0.1261 cm, Actual = 0.0315 cmTranspiration: Potential = 0.3607 cm, Actual = 0.0902 cmRHMEAN = 78.9 %; TMEAN = 289.8 K; HDRY = 3.2513E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 310, Simulated Time = 24.0000 hr _____ Node Number Depth (cm) 2 = 12 25 Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.28998 0.25379 0.25325 Head (cm) = 3.87946E+02 4.73365E+02 4.80000E+02Water Flow (cm) =-8.85370E-02 4.75639E-03 3.82739E-03 Plant Sink (cm) = 2.23503E-02 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 24.7370+ 0.0000+ 0.0000 - 0.0650- 0.1860- 0.0038 = 24.4822 Versus 24.4822 Mass Balance = -5.7220E-06 cm; Time step attempts = 96 and successes = 96 Evaporation: Potential = 0.2601 cm, Actual = 0.0650 cmTranspiration: Potential = 0.7439 cm, Actual = 0.1860 cm RHMEAN = 74.9 %; TMEAN = 289.8 K; HDRY = 3.9616E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 311, Simulated Time = 24.0000 hr _____ Node Number=21225Depth (cm)=5.0000030.7000091.50000Water (cm3/cm3)=0.268790.253680.25328 Head (cm) = 5.17898E+02 4.74693E+02 4.79634E+02

Water Flow (cm) =-1.45191E-01 3.81178E-03 3.83999E-03 Plant Sink (cm) =  $4.01767E-02 \ 0.00000E+00 \ 0.00000E+00$ PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 24.4822+ 0.0000+ 0.0000 - 0.1169- 0.3343- 0.0038 = 24.0272 Versus 24.0274 Mass Balance = -1.5640E-04 cm; Time step attempts =82952 and successes =82952 Evaporation: Potential = 0.4675 cm, Actual = 0.1169 cm Transpiration: Potential = 1.3371 cm, Actual = 0.3343 cm RHMEAN = 59.4 %; TMEAN = 290.7 K; HDRY = 7.1452E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 312, Simulated Time = 24.0000 hr ______ Node Number = 2 12 25 Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.26533 0.25364 0.25328 = 5.43169E+02 4.75196E+02 4.79634E+02 Head (cm) Water Flow (cm) =-8.04452E-02 3.46438E-03 3.84251E-03 Plant Sink (cm) =  $2.40679E-02 \ 0.00000E+00 \ 0.00000E+00$ PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 24.0274+ 0.0762+ 0.0000 - 0.0700- 0.2003- 0.0038 = 23.8295 Versus 23.8300 Mass Balance = -5.1498E-04 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.2829 cm, Actual = 0.0700 cmTranspiration: Potential = 0.8091 cm, Actual = 0.2003 cm RHMEAN = 74.0 %; TMEAN = 286.2 K; HDRY = 4.1295E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 313, Simulated Time = 24.0000 hr _____ 2 Node Number = 12 25 Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.26055 0.25355 0.25328 Head (cm) = 5.80426E+02 4.76267E+02 4.79634E+02 Water Flow (cm) =-8.54803E-02 2.52998E-03 3.84251E-03 Plant Sink (cm) = 1.59358E-02 0.00000E+00 0.00000E+00 INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR PRESTOR STORAGE 23.8300+ 0.0000+ 0.0000 - 0.0464- 0.1326- 0.0038 = 23.6472 Versus 23.6514 Mass Balance = -4.1676E-03 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.1854 cm, Actual = 0.0464 cm Transpiration: Potential = 0.5304 cm, Actual = 0.1326 cm RHMEAN = 72.8 %; TMEAN = 282.6 K; HDRY = 4.3557E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 314, Simulated Time = 24.0000 hr _____ Node Number = 2 12 25

Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.24745 0.25348 0.25328 Head (cm) = 6.98553E+02 4.77105E+02 4.79634E+02Water Flow (cm) =-9.10942E-02 1.70997E-03 3.84251E-03 Plant Sink (cm) = 2.48757E-02 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 23.6514+ 0.0000+ 0.0000 - 0.0724- 0.2070- 0.0038 = 23.3682 Versus 23.3692 Mass Balance = -1.0414E-03 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.2895 cm, Actual = 0.0724 cm Transpiration: Potential = 0.8279 cm, Actual = 0.2070 cm RHMEAN = 63.9 %; TMEAN = 287.6 K; HDRY = 6.1356E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 315, Simulated Time = 24.0000 hr _____ Node Number = Depth (cm) = 2 12 Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.23926 0.25342 0.25328 Head (cm) = 7.87225E+024.77890E+024.79634E+02Water Flow (cm) =-9.02173E-02 9.76126E-04 3.84251E-03 Plant Sink (cm) = 1.99555E-02 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 23.3692+ 0.0000+ 0.0000 - 0.0581- 0.1660- 0.0038 = 23.1413 Versus 23.1459 Mass Balance = -4.6253E-03 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.2322 cm, Actual = 0.0581 cm Transpiration: Potential = 0.6642 cm, Actual = 0.1660 cm RHMEAN = 73.2 %; TMEAN = 286.2 K; HDRY = 4.2765E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 316, Simulated Time = 24.0000 hr _____ 2 Node Number=21225Depth (cm)=5.0000030.7000091.50000Water (cm3/cm3)=0.228290.253360.25328 Node Number = 12 25 Head (cm) = 9.29423E+02 4.78543E+02 4.79634E+02Water Flow (cm) =-8.56049E-02 3.45095E-04 3.84251E-03 Plant Sink (cm) = 2.22163E-02 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 23.1459+ 0.0000+ 0.0000 - 0.0646- 0.1848- 0.0038 = 22.8926 Versus 22.8961 Mass Balance = -3.4714E-03 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.2585 cm, Actual = 0.0646 cm Transpiration: Potential = 0.7394 cm, Actual = 0.1848 cm RHMEAN = 72.8 %; TMEAN = 286.8 K; HDRY = 4.3547E+05 cm; DAYUBC = 0

DAILY SUMMARY: Day = 317, Simulated Time = 24.0000 hr ______ Node Number _ 2 12 25 Depth (cm) = 5.00000 30.70000 91.50000 = 0.21644 0.25332 0.25328 Water (cm3/cm3) Head (cm) = 1.12282E+03 4.79133E+02 4.79634E+02 Water Flow (cm) =-8.35318E-02-1.94662E-04 3.84251E-03 Plant Sink (cm) = 2.28216E-02 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 22.8961+ 0.0000+ 0.0000 - 0.0664- 0.1899- 0.0038 = 22.6360 Versus 22.6387 Mass Balance = -2.7428E-03 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.2655 cm, Actual = 0.0664 cmTranspiration: Potential = 0.7595 cm, Actual = 0.1899 cm RHMEAN = 79.3 %; TMEAN = 288.7 K; HDRY = 3.1857E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 318, Simulated Time = 24.0000 hr _____ Node Number -----2 12 25 Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.19990 0.25327 0.25328 = 1.49501E+03 4.79751E+02 4.79634E+02 Head (cm) Water Flow (cm) =-8.14803E-02-6.33488E-04 3.84251E-03 Plant Sink (cm) = 2.76466E-02 0.00000E+00 0.00000E+00PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 22.6387+ 0.0000+ 0.0000 - 0.0804- 0.2300- 0.0038 = 22.3244 Versus 22.3281 Mass Balance = -3.7117E-03 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.3217 cm, Actual = 0.0804 cm Transpiration: Potential = 0.9201 cm, Actual = 0.2300 cmRHMEAN = 74.5 %; TMEAN = 290.7 K; HDRY = 4.0292E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 319, Simulated Time = 24.0000 hr Node Number 2 12 25 Depth (cm) = 5.00000 30.70000 91.50000 = 0.19291 0.25323 0.25328Water (cm3/cm3) = 1.70407E+03 4.80138E+02 4.79634E+02 Head (cm) Water Flow (cm) =-7.45687E-02-9.67725E-04 3.84251E-03 Plant Sink (cm) = 1.71746E-02 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 22.3281+ 0.0000+ 0.0000 - 0.0500- 0.1429- 0.0038 = 22.1314 Versus 22.1371 Mass Balance = -5.7030E-03 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.1998 cm, Actual = 0.0500 cm Transpiration: Potential = 0.5716 cm, Actual = 0.1429 cm RHMEAN = 75.9 %; TMEAN = 288.2 K; HDRY = 3.7753E+05 cm; DAYUBC = 0

DAILY SUMMARY: Day = 320, Simulated Time = 24.0000 hr Node Number = 2 12 25 Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.19791 0.25323 0.25328 Head (cm) = 1.55070E+03 4.80140E+02 4.79634E+02Water Flow (cm) =-6.48207E-02-8.82708E-04 3.84251E-03 Plant Sink (cm) = 3.32509E-03 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 22.1371+ 0.0000+ 0.0000 - 0.0097- 0.0277- 0.0038 = 22.0960 Versus 22.1049 Mass Balance = -8.9817E-03 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.0387 cm, Actual = 0.0097 cm Transpiration: Potential = 0.1107 cm, Actual = 0.0277 cm RHMEAN = 81.0 %; TMEAN = 287.0 K; HDRY = 2.8924E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 321, Simulated Time = 24.0000 hr 2 Node Number = 12 25 Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.19756 0.25323 0.25328 Head (cm) = 1.56076E+03 4.80140E+02 4.79634E+02Water Flow (cm) =-5.23524E-02-8.01642E-04 3.84251E-03 Plant Sink (cm) = 6.69267E-03 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 22.1049+ 0.0000+ 0.0000 - 0.0195- 0.0557- 0.0038 = 22.0259 Versus 22.0343 Mass Balance = -8.3694E-03 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.0779 cm, Actual = 0.0195 cm Transpiration: Potential = 0.2227 cm, Actual = 0.0557 cm RHMEAN = 75.4 %; TMEAN = 289.5 K; HDRY = 3.8754E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 322, Simulated Time = 24.0000 hr Node Number Depth (cm) = 2 12 25 Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.24781 0.25323 0.25328 = 91.50000 Head (cm) = 6.94961E+02 4.80140E+02 4.79634E+02Water Flow (cm) = 4.51619E-01-7.45974E-04 3.84251E-03Plant Sink (cm) =  $2.44652E-02 \ 0.00000E+00 \ 0.00000E+00$ INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR PRESTOR STORAGE 22.0343+ 1.0031+ 0.0383 - 0.0712- 0.2036- 0.0038 = 22.7588 Versus 22.7672 Mass Balance = -8.3656E-03 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.2905 cm, Actual = 0.0712 cm

Transpiration: Potential = 0.8309 cm, Actual = 0.2036 cm RHMEAN = 78.0 %; TMEAN = 291.2 K; HDRY = 3.4017E+05 cm; DAYUBC = 2558

DAILY SUMMARY: Day = 323, Simulated Time = 24.0000 hr _____ 2 Node Number = 12 25 Depth (cm) Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.25983 0.25323 0.25328 Head (cm) = 5.86244E+02 4.80142E+02 4.79634E+02Water Flow (cm) = 5.31391E-01-7.05902E-04 3.84251E-03Plant Sink (cm) =  $3.73024E-02 \ 0.00000E+00 \ 0.00000E+00$ PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 22.7672+ 0.8890+ 0.0000 - 0.1085- 0.3104- 0.0038 = 23.2335 Versus 23.2405 Mass Balance = -7.0686E-03 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.4384 cm, Actual = 0.1085 cm Transpiration: Potential = 1.2540 cm, Actual = 0.3104 cm RHMEAN = 63.1 %; TMEAN = 281.2 K; HDRY = 6.3139E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 324, Simulated Time = 24.0000 hr ______ Node Number = Depth (cm) = 2 12 25 91.50000 5.00000 30.70000 Water (cm3/cm3) = 0.24448 0.25323 0.25328= 7.29212E+02 4.80142E+02 4.79634E+02Head (cm) Water Flow (cm) =-2.09288E-02-6.70204E-04 3.84251E-03 Plant Sink (cm) = 1.75269E-02 0.00000E+00 0.00000E+00PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 23.2405+ 0.0000+ 0.0000 - 0.0510- 0.1458- 0.0038 = 23.0399 Versus 23.0406 Mass Balance = -7.6866E-04 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.2039 cm, Actual = 0.0510 cm Transpiration: Potential = 0.5833 cm, Actual = 0.1458 cm RHMEAN = 57.0 %; TMEAN = 278.2 K; HDRY = 7.7126E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 325, Simulated Time = 24.0000 hr _____ Node Number Depth (cm) = 2 12 25 Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.22982 0.25323 0.25328 = 9.07625E+02 4.80142E+02 4.79634E+02 Head (cm) Water Flow (cm) =-5.18062E-02-6.29609E-04 3.84251E-03 Plant Sink (cm) =  $2.15039E-02 \ 0.00000E+00 \ 0.00000E+00$ PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 23.0406+ 0.0000+ 0.0000 - 0.0626- 0.1789- 0.0038 = 22.7953 Versus 22.8042

Mass Balance = -8.8692E-03 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.2502 cm, Actual = 0.0626 cm Transpiration: Potential = 0.7157 cm, Actual = 0.1789 cm RHMEAN = 59.2 %; TMEAN = 279.5 K; HDRY = 7.1968E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 326, Simulated Time = 24.0000 hr ______ Node Number 2 12 = 25 Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.19989 0.25323 0.25328 Head (cm) = 1.49520E+03 4.80142E+02 4.79634E+02Water Flow (cm) =-7.51578E-02-5.94820E-04 3.84251E-03 Plant Sink (cm) = 4.07134E-02 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 22.8042+ 0.0000+ 0.0000 - 0.1184- 0.3387- 0.0038 = 22.3432 Versus 22.3494 Mass Balance = -6.2237E-03 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.4737 cm, Actual = 0.1184 cm Transpiration: Potential = 1.3550 cm, Actual = 0.3387 cmRHMEAN = 66.2 %; TMEAN = 285.4 K; HDRY = 5.6475E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 327, Simulated Time = 24.0000 hr ______ 12 Node Number = 2 25 Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.18376 0.25323 0.25328 Head (cm) = 2.04554E+03 4.80142E+02 4.79634E+02Water Flow (cm) =-7.81762E-02-5.62466E-04 3.84251E-03 Plant Sink (cm) = 2.73913E-02 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 22.3494+ 0.0000+ 0.0000 - 0.0797- 0.2279- 0.0038 = 22.0380 Versus 22.0436 Mass Balance = -5.6553E-03 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.3187 cm, Actual = 0.0797 cm Transpiration: Potential = 0.9116 cm, Actual = 0.2279 cmRHMEAN = 77.1 %; TMEAN = 290.4 K; HDRY = 3.5660E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 328, Simulated Time = 24.0000 hr ____ Node Number = 2 12 25 Depth (cm) = 5.00000 30.70000 Water (cm3/cm3) = 0.16885 0.25323 91.50000 0.25328 Head (cm) = 2.84909E+03 4.80153E+02 4.79634E+02 Water Flow (cm) =-6.71868E-02-5.35874E-04 3.84251E-03 Plant Sink (cm) = 2.53246E-02 0.00000E+00 0.00000E+00

PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 22.0436+ 0.0000+ 0.0000 - 0.0737- 0.2107- 0.0038 = 21.7554 Versus 21.7620 Mass Balance = -6.6319E-03 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.2947 cm, Actual = 0.0737 cm Transpiration: Potential = 0.8429 cm, Actual = 0.2107 cm RHMEAN = 65.7 %; TMEAN = 285.1 K; HDRY = 5.7608E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 329, Simulated Time = 24.0000 hr ________ Node Number = 2 12 25 Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.14865 0.25322 0.25328 Head (cm) = 4.91179E+03 4.80254E+02 4.79634E+02Water Flow (cm) =-4.88232E-02-5.52831E-04 3.84251E-03 Plant Sink (cm) = 2.89250E-02 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 21.7620+ 0.0000+ 0.0000 - 0.1050- 0.2681- 0.0038 = 21.3851 Versus 21.3920 Mass Balance = -6.8989E-03 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.4200 cm, Actual = 0.1050 cm Transpiration: Potential = 1.2013 cm, Actual = 0.2681 cm RHMEAN = 60.3 %; TMEAN = 284.0 K; HDRY = 6.9338E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 330, Simulated Time = 24.0000 hr Node Number Depth (cm) = 2 12 25 Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.13972 0.25321 0.25328 Head (cm) = 6.57977E+03 4.80418E+02 4.79634E+02 Water Flow (cm) =-3.16625E-02-6.50548E-04 3.84251E-03 Plant Sink (cm) = 1.48853E-02 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 21.3920+ 0.0000+ 0.0000 - 0.0625- 0.1772- 0.0038 = 21.1484 Versus 21.1533 Mass Balance = -4.8790E-03 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.3358 cm, Actual = 0.0625 cm Transpiration: Potential = 0.9605 cm, Actual = 0.1772 cm RHMEAN = 61.5 %; TMEAN = 283.4 K; HDRY = 6.6580E+05 cm; DAYUBC = ***** DAILY SUMMARY: Day = 331, Simulated Time = 24.0000 hr ______ Node Number=21225Depth (cm)=5.0000030.7000091.50000Water (cm3/cm3)=0.136580.253190.25328

= 7.36638E+03 4.80671E+02 4.79634E+02 Head (cm) Water Flow (cm) =-2.43849E-02-8.14580E-04 3.84251E-03 Plant Sink (cm) = 7.15831E-03 0.00000E+00 0.00000E+00PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 21.1533+ 0.0000+ 0.0000 - 0.0053- 0.1029- 0.0038 = 21.0413 Versus 21.0472 Mass Balance = -5.9433E-03 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.2153 cm, Actual = 0.0053 cm Transpiration: Potential = 0.6158 cm, Actual = 0.1029 cmRHMEAN = 60.6 %; TMEAN = 274.3 K; HDRY = 6.8619E+05 cm; DAYUBC = ***** DAILY SUMMARY: Day = 332, Simulated Time = 24.0000 hr Node Number = Depth (cm) = 2 12 25 5.00000 30.70000 91.50000  $= 0.14168 \quad 0.25250 \quad 0.25331$ Water (cm3/cm3) Head (cm) = 6.14926E+03 4.89267E+02 4.79199E+02Water Flow (cm) =-2.72189E-02-1.37741E-03 3.84726E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 21.0472+ 0.0000+ 0.0000 - 0.0000- 0.0000- 0.0038 = 21.0434 Versus 21.0433 Mass Balance = 4.1962E-05 cm; Time step attempts = 2595 and successes = 2595 Evaporation: Potential = 0.0000 cm, Actual = 0.0000 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cmRHMEAN = 91.7 %; TMEAN = 270.9 K; HDRY = 1.1831E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 333, Simulated Time = 24.0000 hr Node Number Depth (cm) = 2 12 25 Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.14828 0.25206 0.25334 = 4.96803E+03 4.94922E+02 4.78837E+02 Head (cm) Water Flow (cm) =-3.33029E-02-1.89695E-03 3.85611E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 21.0433+ 0.0000+ 0.0000 - 0.0000- 0.0000- 0.0039 = 21.0395 Versus 21.0395 Mass Balance = 1.9073E-06 cm; Time step attempts = 96 and successes = 96 Evaporation: Potential = 0.0000 cm, Actual = 0.0000 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cmRHMEAN = 90.8 %; TMEAN = 272.6 K; HDRY = 1.3263E+05 cm; DAYUBC = 0 

DAILY SUMMARY: Day = 334, Simulated Time = 24.0000 hr

Node Number = 2 12 25 Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.19785 0.25206 0.25334 Head (cm) = 1.55238E+03 4.94922E+02 4.78837E+02Water Flow (cm) = 8.75378E-02-2.01992E-03 3.85992E-03 Plant Sink (cm) = 2.60291E-02 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 21.0395+ 0.7874+ 0.0000 - 0.0757- 0.2166- 0.0039 = 21.5306 Versus 21.5396 Mass Balance = -9.0199E-03 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.3060 cm, Actual = 0.0757 cm Transpiration: Potential = 0.8753 cm, Actual = 0.2166 cm RHMEAN = 66.6 %; TMEAN = 279.5 K; HDRY = 5.5622E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 335, Simulated Time = 24.0000 hr _____ Node Number === 2 12 25 Depth (cm) Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.18872 0.25206 0.25334 Head (cm) = 1.84954E+03 4.94922E+02 4.78837E+02Water Flow (cm) = 1.29301E-02-2.01992E-03 3.85992E-03Plant Sink (cm) = 8.54885E-03 0.00000E+00 0.00000E+00 EVAPO TRANS PRESTOR INFIL RUNOFF DRAIN NEWSTOR STORAGE 21.5396+ 0.0000+ 0.0000 - 0.0249- 0.0711- 0.0039 = 21.4398 Versus 21.4503 Mass Balance = -1.0469E-02 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.0995 cm, Actual = 0.0249 cmTranspiration: Potential = 0.2845 cm, Actual = 0.0711 cm RHMEAN = 75.7 %; TMEAN = 280.4 K; HDRY = 3.8214E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 336, Simulated Time = 24.0000 hr -----Node Number 2 12 25 ----Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.18646 0.25206 0.25334 = 1.93520E+03 4.94922E+02 4.78837E+02Head (cm) Water Flow (cm) =-7.40476E-03-2.01992E-03 3.85992E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR PRESTOR STORAGE 21.4503+ 0.0000+ 0.0000 - 0.2655- 0.0000- 0.0039 = 21.1809 Versus 21.1995 Mass Balance = -1.8663E-02 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 1.0621 cm, Actual = 0.2655 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 76.1 %; TMEAN = 284.3 K; HDRY = 3.7493E+05 cm; DAYUBC = 0 

DAILY SUMMARY: Day = 337, Simulated Time = 24.0000 hr ____ Node Number 2 12 25 Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.18705 0.25169 0.25336 Head (cm) = 1.91232E+03 4.99568E+02 4.78561E+02 Water Flow (cm) =-1.16200E-02-2.15561E-03 3.86292E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 21.1995+ 0.0000+ 0.0000 - 0.0000- 0.0000- 0.0039 = 21.1957 Versus 21.1958 Mass Balance = -9.5367E-05 cm; Time step attempts = 2595 and successes = 2595 Evaporation: Potential = 0.0734 cm, Actual = 0.0000 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cmRHMEAN = 91.2 %; TMEAN = 287.3 K; HDRY = 1.2674E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 338, Simulated Time = 24.0000 hr ______ Node Number == 2 12 25 Depth (cm) = 5.00000 30.70000 Water (cm3/cm3) = 0.18705 0.25169 91.50000 0.25336 = 1.91231E+03 4.99568E+02 4.78561E+02 Head (cm) Water Flow (cm) =-1.21938E-02-2.21870E-03 3.86602E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 21.1958+ 0.0000+ 0.0000 - 0.0193- 0.0000- 0.0039 = 21.1726 Versus 21.1844 Mass Balance = -1.1835E-02 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 1.3539 cm, Actual = 0.0193 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cmRHMEAN = 78.3 %; TMEAN = 293.2 K; HDRY = 3.3486E+05 cm; DAYUBC = ***** DAILY SUMMARY: Day = 339, Simulated Time = 24.0000 hr ______ Node Number ..... 2 12 Node Number = Depth (cm) = 25 5.00000 30.70000 91.50000 = 0.18705 0.25169 0.25336Water (cm3/cm3) Head (cm) = 1.91231E+03 4.99568E+02 4.78561E+02 Water Flow (cm) =-1.21939E-02-2.21870E-03 3.86602E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 21.1844+ 0.0000+ 0.0000 - 0.0103- 0.0000- 0.0039 = 21.1702 Versus 21.1817 Mass Balance = -1.1463E-02 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 1.2723 cm, Actual = 0.0103 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 72.7 %; TMEAN = 290.4 K; HDRY = 4.3702E+05 cm; DAYUBC = *****

DAILY SUMMARY: Day = 340, Simulated Time = 24.0000 hr _____ Node Number === 2 12 25 Depth (cm) Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.18705 0.25169 0.25336 5.00000 = 1.91231E+03 4.99568E+02 4.78561E+02 Head (cm) Water Flow (cm) =-1.21939E-02-2.21870E-03 3.86602E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 21.1817+ 0.0000+ 0.0000 - 0.0106- 0.0000- 0.0039 = 21.1672 Versus 21.1787 Mass Balance = -1.1517E-02 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.4185 cm, Actual = 0.0106 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 64.1 %; TMEAN = 284.3 K; HDRY = 6.1043E+05 cm; DAYUBC = ***** DAILY SUMMARY: Day = 341, Simulated Time = 24.0000 hr _ _ _ _ _ _ _ _ _ _ _ _ _ _ _ _ _ _ 2 12 Node Number = 25 Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.18773 0.25148 0.25337 = 1.88654E+03 5.02328E+02 4.78417E+02Head (cm) Water Flow (cm) = -1.20051E - 02 - 2.25789E - 03 3.86794E - 03Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 21.1787+ 0.0000+ 0.0000 - 0.0024- 0.0000- 0.0039 = 21.1725 Versus 21.1760 Mass Balance = -3.5648E-03 cm; Time step attempts =74271 and successes =74271 Evaporation: Potential = 0.4965 cm, Actual = 0.0024 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 77.8 %; TMEAN = 283.2 K; HDRY = 3.4344E+05 cm; DAYUBC = 71727 DAILY SUMMARY: Day = 342, Simulated Time = 24.0000 hr Node Number = 2 12 25 Depth (cm) Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.18773 0.25148 0.25337 = 1.88654E+03 5.02328E+02 4.78417E+02Head (cm) Water Flow (cm) =-1.19087E-02-2.27400E-03 3.86902E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 INFIL RUNOFF PRESTOR EVAPO TRANS DRAIN NEWSTOR STORAGE 21.1760+ 0.0000+ 0.0000 - 0.0141- 0.0000- 0.0039 = 21.1580 Versus 21.1698 Mass Balance = -1.1705E-02 cm; Time step attempts =***** and successes =*****
Evaporation: Potential = 1.3779 cm, Actual = 0.0141 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 60.8 %; TMEAN = 277.9 K; HDRY = 6.8150E+05 cm; DAYUBC = ***** DAILY SUMMARY: Day = 343, Simulated Time = 24.0000 hr _____ Node Number Depth (cm) = 2 12 25 Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.18773 0.25148 0.25337 Head (cm) = 1.88654E+03 5.02328E+02 4.78417E+02Water Flow (cm) =-1.19087E-02-2.27400E-03 3.86902E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 21.1698+ 0.0000+ 0.0000 - 0.0092- 0.0000- 0.0039 = 21.1567 Versus 21.1686 Mass Balance = -1.1887E-02 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.5743 cm, Actual = 0.0092 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 55.9 %; TMEAN = 276.8 K; HDRY = 7.9680E+05 cm; DAYUBC = ***** DAILY SUMMARY: Day = 344, Simulated Time = 24.0000 hr ______ 2 Node Number 12 25 = 

 Node Number
 =
 2
 12
 25

 Depth (cm)
 =
 5.00000
 30.70000
 91.50000

 Water (cm3/cm3)
 =
 0.18786
 0.25144
 0.25338

 Head (cm) = 1.88155E+03 5.02845E+02 4.78399E+02Water Flow (cm) =-1.18350E-02-2.29170E-03 3.86949E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 21.1686+ 0.0000+ 0.0000 - 0.0067- 0.0000- 0.0039 = 21.1580 Versus 21.1681 Mass Balance = -1.0033E-02 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.8985 cm, Actual = 0.0067 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 60.9 %; TMEAN = 276.5 K; HDRY = 6.7873E+05 cm; DAYUBC = ***** DAILY SUMMARY: Day = 345, Simulated Time = 24.0000 hr ______ Node Number Depth (cm) = 2 12 25 Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.21847 0.25144 0.25338 Head (cm) = 1.08617E+03 5.02845E+02 4.78399E+02Water Flow (cm) = 1.22004E-01-2.29143E-03 3.86953E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE

122 of 130

21.1681+ 0.6604+ 0.0000 - 0.0733- 0.0000- 0.0039 = 21.7513 Versus 21.7674 Mass Balance = -1.6039E-02 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.2961 cm, Actual = 0.0733 cmTranspiration: Potential = 0.0000 cm, Actual = 0.0000 cmRHMEAN = 85.5 %; TMEAN = 277.3 K; HDRY = 2.1495E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 346, Simulated Time = 24.0000 hr Node Number = 2 12 25 Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.23716 0.25144 0.25338 Head (cm) = 8.12104E+025.02845E+024.78399E+02Water Flow (cm) = 1.90820E-01-2.29143E-03 3.86953E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 21.7674+ 0.3302+ 0.0000 - 0.0000- 0.0000- 0.0039 = 22.0937 Versus 22.1017 Mass Balance = -8.0242E-03 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.0000 cm, Actual = 0.0000 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cmRHMEAN = 98.9 %; TMEAN = 282.6 K; HDRY = 1.4559E+04 cm; DAYUBC = 0 DAILY SUMMARY: Day = 347, Simulated Time = 24.0000 hr Node Number = Depth (cm) = 2 12 25 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.26284 0.25144 0.25338= 5.62204E+02 5.02845E+02 4.78399E+02Head (cm) Water Flow (cm) = 4.57964E-01-2.29143E-03 3.86953E-03Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 22.1017+ 0.6350+ 0.0000 - 0.0000- 0.0000- 0.0039 = 22.7328 Versus 22.7375 Mass Balance = -4.6635E-03 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.0000 cm, Actual = 0.0000 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cmRHMEAN = 94.8 %; TMEAN = 281.2 K; HDRY = 7.3844E+04 cm; DAYUBC = 0 DAILY SUMMARY: Day = 348, Simulated Time = 24.0000 hr Node Number 2 12 25 Depth (cm) = 5.00000 30.70000 Water (cm3/cm3) = 0.25216 0.2514491.50000 0.25338 = 6.53034E+02 5.02846E+02 4.78399E+02Head (cm) Water Flow (cm) = 7.67189E-02-2.29148E-03 3.86953E-03

Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 22.7375+ 0.0000+ 0.0000 - 0.0000- 0.0000- 0.0039 = 22.7336 Versus 22.7391 Mass Balance = -5.4417E-03 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.0000 cm, Actual = 0.0000 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 93.6 %; TMEAN = 279.0 K; HDRY = 9.0333E+04 cm; DAYUBC = 0 DAILY SUMMARY: Day = 349, Simulated Time = 24.0000 hr _____ = Node Number 2 12 25 Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.24667 0.25144 0.25338= 7.06448E+02 5.02846E+02 4.78399E+02Head (cm) Water Flow (cm) = 3.93032E - 02 - 2.29176E - 03 3.86953E - 03Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 22.7391+ 0.0000+ 0.0000 - 0.0000- 0.0000- 0.0039 = 22.7352 Versus 22.7396 Mass Balance = -4.3621E-03 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.0000 cm, Actual = 0.0000 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 92.0 %; TMEAN = 280.1 K; HDRY = 1.1449E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 350, Simulated Time = 24.0000 hr Node Number = 2 12 25 Depth (cm) Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.33165 0.25299 0.25338 = 2.13008E+02 4.83190E+02 4.78320E+02 Head (cm) Water Flow (cm) = 2.26373E+00 1.97940E-03 3.87000E-03Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 22.7396+ 2.8545+ 0.9301 - 0.0000- 0.0000- 0.0039 = 25.5902 Versus 25.5929 Mass Balance = -2.6779E-03 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.0000 cm, Actual = 0.0000 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 97.6 %; TMEAN = 283.2 K; HDRY = 3.3848E+04 cm; DAYUBC = 28581 DAILY SUMMARY: Day = 351, Simulated Time = 24.0000 hr 2 Node Number = 12 25 = 5.00000 30.70000 91.50000 Depth (cm)

Water (cm3/cm3) = 0.34818 0.25568 0.25339Head (cm) = 1.60791E+02 4.50900E+02 4.78262E+02Water Flow (cm) = 4.77745E-01 1.71852E-02 3.87202E-03Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 25.5929+ 0.7901+ 0.0481 - 0.1985- 0.0000- 0.0039 = 26.1807 Versus 26.1808 Mass Balance = -1.0300E-04 cm; Time step attempts = 3231 and successes = 3231Evaporation: Potential = 0.8018 cm, Actual = 0.1985 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 74.1 %; TMEAN = 280.9 K; HDRY = 4.1098E+05 cm; DAYUBC = 190 DAILY SUMMARY: Day = 352, Simulated Time = 24.0000 hr Node Number ----2 12 25 Depth (cm) = Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.33518 0.25636 0.25338 Head (cm)  $= 2.01291E+02 \ 4.43092E+02 \ 4.78322E+02$ Water Flow (cm) =-2.55658E-01 1.89764E-02 3.87209E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 26.1808+ 0.0000+ 0.0000 - 0.3478- 0.0000- 0.0039 = 25.8291 Versus 25.8291 Mass Balance = -3.8147E-06 cm; Time step attempts = 96 and successes = 96 Evaporation: Potential = 1.3912 cm, Actual = 0.3478 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 62.2 %; TMEAN = 282.3 K; HDRY = 6.5133E+05 cm; DAYUBC = DAILY SUMMARY: Day = 353, Simulated Time = 24.0000 hr _ _ _ _ _ _ _ _ _ _ _ _ _ _ _ _ _ _ _ 2 Node Number = 12 25 Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.32428 0.25637 0.25337 = 2.38686E+02 4.42999E+02 4.78470E+02 Head (cm) Water Flow (cm) =-2.27486E-01 1.48117E-02 3.86966E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 25.8291+ 0.0000+ 0.0000 - 0.3040- 0.0000- 0.0039 = 25.5212 Versus 25.5212 Mass Balance = -3.8147E-06 cm; Time step attempts = 96 and successes = 96 Evaporation: Potential = 1.2161 cm, Actual = 0.3040 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cmRHMEAN = 55.1 %; TMEAN = 277.0 K; HDRY = 8.1676E+05 cm; DAYUBC = 0 

DAILY SUMMARY: Day = 354, Simulated Time = 24.0000 hr

_____ Node Number = 2 12 25 Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.31490 0.25617 0.25336 Head (cm) = 2.74083E+024.45338E+024.78594E+02Water Flow (cm) =-1.98463E-01 1.14589E-02 3.86643E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 25.5212+ 0.0000+ 0.0000 - 0.2643- 0.0000- 0.0039 = 25.2530 Versus 25.2531 Mass Balance = -5.7220E-06 cm; Time step attempts = 96 and successes = 96 Evaporation: Potential = 1.0572 cm, Actual = 0.2643 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 55.2 %; TMEAN = 279.8 K; HDRY = 8.1379E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 355, Simulated Time = 24.0000 hr _____ Node Number Depth (cm) == 2 12 25 Depth (cm) = 5.00000 30.70000 Water (cm3/cm3) = 0.29530 0.25584 91.50000 0.25336 Head (cm) = 3.60704E+02 4.49134E+02 4.78570E+02Water Flow (cm) =-2.96655E-01 8.86728E-03 3.86514E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 25.2531+ 0.0000+ 0.0000 - 0.4404- 0.0000- 0.0039 = 24.8087 Versus 24.8088 Mass Balance = -2.0981E-05 cm; Time step attempts = 160 and successes = 160Evaporation: Potential = 1.7618 cm, Actual = 0.4404 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 59.1 %; TMEAN = 281.8 K; HDRY = 7.2201E+05 cm; DAYUBC = DAILY SUMMARY: Day = 356, Simulated Time = 24.0000 hr Node Number = 2 12 25 
 Node Number
 =
 2
 12

 Depth (cm)
 =
 5.00000
 30.70000

 Water (cm3/cm3)
 =
 0.28705
 0.25537
 91.50000 0.25338 Head (cm) = 4.03755E+02 4.54525E+02 4.78330E+02Water Flow (cm) =-2.27448E-01 6.21962E-03 3.86798E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 24.8088+ 0.0000+ 0.0000 - 0.2832- 0.0000- 0.0039 = 24.5217 Versus 24.5217 Mass Balance = -1.3351E-05 cm; Time step attempts = 96 and successes = 96 Evaporation: Potential = 1.1328 cm, Actual = 0.2832 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 76.1 %; TMEAN = 279.8 K; HDRY = 3.7365E+05 cm; DAYUBC = 0 

DAILY SUMMARY: Day = 357, Simulated Time = 24.0000 hr _____ Node Number = 2 12 25 Depth (cm) 5.00000 30.70000 91.50000 = = 0.27808 Water (cm3/cm3) 0.25494 0.25342 Head (cm) = 4.56137E+02 4.59580E+02 4.77873E+02=-1.89478E-01 4.35606E-03 3.87569E-03 Water Flow (cm) Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 24.5217+ 0.0000+ 0.0000 - 0.2536- 0.0000- 0.0039 = 24.2642 Versus 24.2642 Mass Balance = -1.3351E-05 cm; Time step attempts = 96 and successes = 96 Evaporation: Potential = 1.0145 cm, Actual = 0.2536 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 50.1 %; TMEAN = 273.2 K; HDRY = 9.4659E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 358, Simulated Time = 24.0000 hr Node Number = 2 12 25 Depth (cm) = 5.00000 30.70000 Water (cm3/cm3) = 0.27114 0.25486 Depth (cm) 91.50000 0.25343 Head (cm) = 5.01495E+02 4.60566E+02 4.77761E+02 Water Flow (cm) =-1.62175E-01 3.43103E-03 3.88330E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 INFIL RUNOFF PRESTOR EVAPO TRANS DRAIN NEWSTOR STORAGE 24.2642+ 0.0000+ 0.0000 - 0.2110- 0.0000- 0.0039 = 24.0494 Versus 24.0509 Mass Balance = -1.5450E-03 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.8438 cm, Actual = 0.2110 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 56.3 %; TMEAN = 272.3 K; HDRY = 7.8801E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 359, Simulated Time = 24.0000 hr ______ Node Number == 2 12 25 Depth (cm) 5.00000 30.70000 91.50000 ----Water (cm3/cm3) = 0.271620.25484 0.25343 = 4.98227E+02 4.60701E+02 4.77761E+02 Head (cm) Water Flow (cm) =-1.01493E-01 3.36369E-03 3.88355E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 24.0509+ 0.0000+ 0.0000 - 0.0921- 0.0000- 0.0039 = 23.9549 Versus 23.9589 Mass Balance = -3.9940E-03 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.3685 cm, Actual = 0.0921 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm

RHMEAN = 71.5 %; TMEAN = 270.7 K; HDRY = 4.5959E+05 cm; DAYUBC =

DAILY SUMMARY: Day = 360, Simulated Time = 24.0000 hr = Node Number 2 12 25 Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.25856 0.25480 0.25343 Head (cm) = 5.96746E+02 4.61248E+02 4.77761E+02Water Flow (cm) =-1.34497E-01 2.97270E-03 3.88355E-03 Plant Sink (cm) =  $0.00000E+00 \ 0.00000E+00 \ 0.00000E+00$ PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 23.9589+ 0.0000+ 0.0000 - 0.2384- 0.0000- 0.0039 = 23.7166 Versus 23.7199 Mass Balance = -3.2787E-03 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.9538 cm, Actual = 0.2384 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 54.1 %; TMEAN = 272.0 K; HDRY = 8.4111E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 361, Simulated Time = 24.0000 hr 2 Node Number 12 = 25 Depth (cm) Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.25708 0.25474 0.25343 Head (cm) = 6.09232E+02 4.61942E+02 4.77761E+02Water Flow (cm) =-1.17836E-01 2.34939E-03 3.88355E-03 Plant Sink (cm) =  $0.00000E+00 \ 0.00000E+00 \ 0.00000E+00$ INFIL RUNOFF PRESTOR EVAPO TRANS DRAIN NEWSTOR STORAGE 23.7199+ 0.0000+ 0.0000 - 0.1201- 0.0000- 0.0039 = 23.5959 Versus 23.6016 Mass Balance = -5.6877E-03 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.4802 cm, Actual = 0.1201 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 65.6 %; TMEAN = 275.9 K; HDRY = 5.7886E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 362, Simulated Time = 24.0000 hr _____ Node Number ----2 12 25 Depth (cm) Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.24959 0.25468 0.25343 Head (cm) = 6.77377E+02 4.62636E+02 4.77761E+02Water Flow (cm) =-1.11597E-01 1.75343E-03 3.88355E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 23.6016+ 0.0000+ 0.0000 - 0.1714- 0.0000- 0.0039 = 23.4263 Versus 23.4310

Mass Balance = -4.7512E-03 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.6857 cm, Actual = 0.1714 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 60.3 %; TMEAN = 277.6 K; HDRY = 6.9387E+05 cm; DAYUBC = Ο DAILY SUMMARY: Day = 363, Simulated Time = 24.0000 hr _____ Node Number = Depth (cm) = 2 12 25 Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.23697 0.25453 0.25343 Head (cm) = 8.14505E+02 4.64468E+02 4.77761E+02Water Flow (cm) =-1.34619E-01 1.13965E-03 3.88355E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 23.4310+ 0.0000+ 0.0000 - 0.2621- 0.0000- 0.0039 = 23.1651 Versus 23.1696 Mass Balance = -4.5071E-03 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 1.0483 cm, Actual = 0.2621 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 56.2 %; TMEAN = 271.2 K; HDRY = 7.8989E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 364, Simulated Time = 24.0000 hr ______ Node Number === 2 12 25 Node Number=21223Depth (cm)=5.0000030.7000091.50000Water (cm3/cm3)=0.243460.254480.25343Head (cm) = 7.40080E+024.65037E+024.77761E+02Water Flow (cm) =-1.00592E-01 3.55960E-04 3.88355E-03 Plant Sink (cm) =  $0.00000E+00 \ 0.00000E+00 \ 0.00000E+00$ PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 23.1696+ 0.0000+ 0.0000 - 0.0000- 0.0000- 0.0039 = 23.1657 Versus 23.1734 Mass Balance = -7.6523E-03 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.0000 cm, Actual = 0.0000 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 77.0 %; TMEAN = 267.3 K; HDRY = 3.5867E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 365, Simulated Time = 24.0000 hr ______ Node Number Depth (cm) 12 = 2 25 Depth (cm) = 5.00000 30.70000 91.50000 Water (cm3/cm3) = 0.24138 0.25445 0.25343 Head (cm) = 7.62939E+02 4.65370E+02 4.77761E+02Water Flow (cm) =-6.07147E-02-3.54064E-05 3.88355E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00

PRESTOR INFIL RUNOFF EVAPO 23.1734+ 0.0000+ 0.0000 - 0.0829-	Т О.	RANS DRAIN 0000- 0.0039	NEWSTOR STORAGE = 23.0866 Versus 23.0977							
Mass Balance = -1.1095E-02 cm; Ti Evaporation: Potential = 0.331 Transpiration: Potential = 0.000	me 7 c 0 c	step attempts m, Actual = m. Actual =	=***** and successes =***** 0.0829 cm 0.0000 cm							
RHMEAN = $69.3$ %; TMEAN = $269.5$ K;	H	DRY = 5.0354	E+05  cm;  DAYUBC = 0							
1										
UNSAT-H Version 2.03										
SIMULATION SUMMARY										
Title:										
Muskogee LF Subtitle D Final Cover										
Transpiration Scheme is:	=	1								
Potential Evapotranspiration	===	1.8318E+02	[cm]							
Potential Transpiration	=	4.3246E+02	[ Cm ]							
Actual Transpiration	=	3.9417E+01	[ Cm ]							
Potential Evaporation	=	3.0027E+02	[cm]							
Actual Evaporation	=	3.3072E+01	[cm]							
Evaporation during Growth	=	1.4518E+01	[cm]							
Total Runoff	=	1.8245E+01	[ Cm ]							
Total Infiltration		7.1315E+01	[cm]							
Total Drainage at Base of Profile	=	1.4040E+00	[cm]							
Total Applied Water	=	8.9560E+01	[ Cm ]							
Actual Rainfall	==	8.9560E+01	[ Cm ]							
Actual Irrigation	=	0.0000E+00	[ Cm ]							
Total Final Moisture Storage	=	2.3098E+01	[ Cm ]							
Mass Balance Error	=	-2.8770E+00	[ Cm ]							
Total Successful Time Steps	=	56469442								
Total Attempted Time Steps	=	56469442								
Total Time Step Reductions (DHMAX)	==	23089								
Total Changes in Surface Boundary	==	26577674								
Total Time Actually Simulated	=	3.6500E+02	[days]							

Total water flow (cm) across different depths at the end of 3.6500E+02 days:

DEPTH	FLOW	DEPTH	FLOW	DEPTH	FLOW
0.000	3.8242E+01	2.500	3.8240E+01	7.000	2.0170E+01
10.500	1.0334E+01	13.500	3.0502E+00	16.500	-1.1462E+00
19.500	-9.6836E-01	23.000	-6.6754E-01	27.500	-1.9679E-01
30.250	1.5486E-01	30.550	1.7398E-01	30.650	1.7398E-01
32.850	3.3387E-01	37.500	6.6553E-01	42.500	9.0986E-01
47.500	1.0860E+00	52.500	1.2110E+00	57.500	1.2964E+00
62.500	1.3514E+00	67.500	1.3839E+00	72.500	1.4010E+00
77.500	1.4075E+00	82.500	1.4085E+00	87.500	1.4064E+00
90.750	1.4044E+00	91.500	1.4040E+00		

## ET MONOLITHIC SOIL ALTERNATIVE FINAL COVER SYSTEM UNSAT-H ANALYSIS

Program DATAINH Version2.03 Input Filename: S:\Nevzat\WinUnsatH\project.inp Date Processed: 15-JUL-** Time Processed: 13:15: 9 Title: Muskogee AFC Alternative Final Cover Options chosen include: LOWER = 1 IPLANT = 1 NGRAV = 1ISWDIF = 1IHEAT = 0UPPERH = 0LOWERH = 1DAYEND = 365ICONVH = 0NPRINT = 0NDAYS = 365 NYEARS = 5 IRAIN = 1 1 NSURPE = NFHOUR = 2ITOPBC = 0ET OPT = 1 ICLOUD = 0KEST = 3KOPT = 4IVAPOR = 1 SH OPT = 1 INMAX = 2 INMAX = 2 INHMAX = 1 HIRRI = 1.00 HDRY = 1.000E-06 HTOP = 15.0 DHMAX = 10.0DMAXBA = 1.000E-03 DELMAX = 0.250 DELMIN = 1.000E-04 STOPHR = 24.0 OUTTIM = 0.250TORT = 0.660 TSOIL = 291. VAPDIF = 0.240 QHTOP = 0.00 IVAPOR = 1: This option allows vapor flow Saturated vapor density (g/cm3) of soil when soil temperature is a constant equal to TSOIL = 1.521E-05= 2.123E-04 = 5.702E+02 100*MOLAR*GRAV/GASCON (K/cm) VC (cm5/g/h)TGRAD = 0.00 TSMEAN = 0.00 TSAMP = 0.00 QHLEAK = 0.00WTF = 0.500 RFACT = 1.00RAINIF = 1.000E - 03 DHFACT = 0.100MATN = 3 NPT = 25 KOPT = 4: van Genuchten functions for soil hydraulic properties THETA vs H, MAT 1, AIRINT = 0.0000 THET = 0.40000THTR = 8.00000E - 02ALPHA = 5.00000E-03M = 0.32432N = 1.4800K vs H, MAT 1, AIRINK = 0.0000SK = 0.18000A = 5.00000E - 03N = 1.4800M = 0.32432KMODEL = 2.0000EPIT = 0.50000THETA vs H, MAT 2, AIRINT = 0.0000THET = 0.40000THTR = 0.15000ALPHA = 5.00000E-03

Ν	==	1.4800		М :		0.32432
K vs H,	MA	Τ 2,				
AIRINK	=	0.0000		SK :		7.20000E-02
A	=	5.00000E-03		N :		1.4800
М	=	0.32432	F	MODEL :	=	2.0000
EPIT	=	0.50000				
THETA vs H,	MA	т З,				
AIRINT	==	0.0000		THET :	=	0.40000
THTR	=	0.15000		ALPHA :		5.00000E-03
N		1.4800		M :	-	0.32432
K vs H,	MA	т З,				
AIRINK	=	0.0000		SK :		7.20000E-02
А	=	5.00000E-03		N :		1.4800
М	=	0.32432	F	MODEL :	=	2.0000
EPIT	=	0.50000				

Surface node hydraulic properties

HIRRI = 1.0 , THETA = 0.4000, K = 1.5281E-01, C = -6.0345E-05 HDRY = 1.00E-06, THETA = 0.4000, K = 1.8000E-01, C = -7.9592E-08 NDAY = 0

NODE	Z	MAT	HEAD	CONDUCTIVITY	CAPACITY	THETA	TEMP
1	0.00	1	1.0000E+04	6.8953E-08	-2.3395E-06	0.1289	291.0
2	5.00	1	1.0000E+04	6.8953E-08	-2.3395E-06	0.1289	291.0
3	9.00	1	1.0000E+04	6.8953E-08	-2.3395E-06	0.1289	291.0
4	12.00	1	1.0000E+04	6.8953E-08	-2.3395E-06	0.1289	291.0
5	15.00	1	1.0000E+04	6.8953E-08	-2.3395E-06	0.1289	291.0
6	18.00	2	1.0000E+04	2.7581E-08	-1.8278E-06	0.1882	291.0
7	21.00	2	1.0000E+04	2.7581E-08	-1.8278E-06	0.1882	291.0
8	25.00	2	1.0000E+04	2.7581E-08	-1.8278E-06	0.1882	291.0
9	30.00	2	1.0000E+04	2.7581E-08	-1.8278E-06	0.1882	291.0
10	35.00	2	1.0000E+04	2.7581E-08	-1.8278E-06	0.1882	291.0
11	40.00	2	1.0000E+04	2.7581E-08	-1.8278E-06	0.1882	291.0
12	50.00	2	1.0000E+04	2.7581E-08	-1.8278E-06	0.1882	291.0
13	60.00	2	1.0000E+04	2.7581E-08	-1.8278E-06	0.1882	291.0
14	70.00	2	1.0000E+04	2.7581E-08	-1.8278E-06	0.1882	291.0
15	80.00	2	1.0000E+04	2.7581E-08	-1.8278E-06	0.1882	291.0
16	85.00	2	1.0000E+04	2.7581E-08	-1.8278E-06	0.1882	291.0
17	88.00	2	1.0000E+04	2.7581E-08	-1.8278E-06	0.1882	291.0
18	91.50	2	1.0000E+04	2.7581E-08	-1.8278E-06	0.1882	291.0
19	92.00	3	1.0000E+04	2.7581E-08	-1.8278E-06	0.1882	291.0
20	96.00	3	1.0000E+04	2.7581E-08	-1.8278E-06	0.1882	291.0
21	100.00	3	1.0000E+04	2.7581E-08	-1.8278E-06	0.1882	291.0
22	110.00	3	1.0000E+04	2.7581E-08	-1.8278E-06	0.1882	291.0
23	115.00	3	1.0000E+04	2.7581E-08	-1.8278E-06	0.1882	291.0
24	118.00	3	1.0000E+04	2.7581E-08	-1.8278E-06	0.1882	291.0
25	122.00	3	1.0000E+04	2.7581E-08	-1.8278E-06	0.1882	291.0

Total Initial Storage = 21.9812 cm

IPLANT = 1

Total number of Growth Day - Leaf Area Index (LAI) data pairs = 5

Growth Day	LAI
0	2.000
81	4.500
320	4.500
335	3.500
364	2.000

BARE =	0.050										
DAY	LAI	DAY	LAI	DAY	LAI	DAY	LAI	DAY	LAI	DAY	LAI
1	2.031	2	2.062	3	2.093	4	2.123	5	2.154	6	2.185
7	2.216	8	2.247	9	2.278	10	2.309	11	2.340	12	2.370
13	2.401	14	2.432	15	2.463	16	2.494	17	2.525	18	2.556
19	2.586	20	2.617	21	2.648	22	2.679	23	2.710	24	2.741
25	2.772	26	2.802	27	2.833	28	2.864	29	2.895	30	2.926
31	2.957	32	2.988	33	3.019	34	3.049	35	3.080	36	3.111
37	3.142	38	3.173	39	3.204	40	3.235	41	3.265	42	3.296
43	3.327	44	3.358	45	3.389	46	3.420	47	3.451	48	3.481
49	3.512	50	3.543	51	3.574	52	3.605	53	3.636	54	3.667
55	3.698	56	3.728	57	3.759	58	3.790	59	3.821	60	3.852
61	3.883	62	3.914	63	3.944	64	3.975	65	4.006	66	4.037
67	4.068	68	4.099	69	4.130	70	4.160	71	4.191	72	4.222
73	4.253	74	4.284	75	4.315	76	4.346	77	4.377	78	4.407
79	4.438	80	4.469	81	4.500	82	4.500	83	4.500	84	4.500
85	4.500	86	4.500	87	4.500	88	4.500	89	4.500	90	4.500
91	4.500	92	4.500	93	4.500	94	4.500	95	4.500	96	4.500
97	4.500	98	4.500	99	4.500	100	4.500	101	4.500	102	4.500
103	4.500	104	4.500	105	4.500	106	4.500	107	4.500	108	4.500
109	4.500	110	4.500	111	4.500	112	4.500	113	4.500	114	4.500
115	4.500	116	4.500	117	4.500	118	4.500	119	4.500	120	4.500
121	4.500	122	4.500	123	4.500	124	4.500	125	4.500	126	4.500
127	4.500	128	4.500	129	4.500	130	4.500	131	4.500	132	4.500
133	4.500	134	4.500	135	4.500	136	4.500	137	4.500	138	4.500
139	4.500	140	4.500	141	4.500	142	4.500	143	4.500	144	4.500
145	4.500	146	4.500	147	4.500	148	4.500	149	4.500	150	4.500
151	4.500	152	4.500	153	4.500	154	4.500	155	4.500	156	4.500
157	4.500	158	4.500	159	4.500	160	4.500	161	4.500	162	4.500
163	4.500	164	4.500	165	4.500	166	4.500	167	4.500	168	4.500
169	4.500	170	4.500	171	4.500	172	4.500	173	4.500	174	4.500
175	4.500	176	4.500	177	4.500	178	4.500	179	4.500	180	4.500
181	4.500	182	4.500	183	4.500	184	4.500	185	4.500	186	4.500
187	4.500	188	4.500	189	4.500	190	4.500	191	4.500	192	4.500
193	4.500	194	4.500	195	4.500	196	4.500	197	4.500	198	4.500
199	4.500	200	4.500	201	4.500	202	4.500	203	4.500	204	4.500
205	4.500	206	4.500	207	4.500	208	4.500	209	4.500	210	4.500
211	4.500	212	4.500	213	4.500	214	4.500	215	4.500	216	4.500
217	4.500	218	4.500	219	4.500	220	4.500	221	4.500	222	4.500
223	4.500	224	4.500	225	4.500	226	4.500	227	4.500	228	4.500
229	4.500	230	4.500	231	4.500	232	4.500	233	4.500	234	4.500
235	4.500	236	4.500	237	4.500	238	4.500	239	4.500	240	4.500
241	4.500	242	4.500	243	4.500	244	4.500	245	4.500	246	4,500

247	4.500	248	4.500	249	4.500	250	4.500	251	4.500	252	4.500
253	4.500	254	4.500	255	4.500	256	4.500	257	4.500	258	4.500
259	4.500	260	4.500	261	4.500	262	4.500	263	4.500	264	4.500
265	4.500	266	4.500	267	4.500	268	4.500	269	4.500	270	4.500
271	4.500	272	4.500	273	4.500	274	4.500	275	4.500	276	4.500
277	4.500	278	4.500	279	4.500	280	4.500	281	4.500	282	4.500
283	4.500	284	4.500	285	4.500	286	4.500	287	4.500	288	4.500
289	4.500	290	4.500	291	4.500	292	4.500	293	4.500	294	4.500
295	4.500	296	4.500	297	4.500	298	4.500	299	4.500	300	4.500
301	4.500	302	4.500	303	4.500	304	4.500	305	4.500	306	4.500
307	4.500	308	4.500	309	4.500	310	4.500	311	4.500	312	4.500
313	4.500	314	4.500	315	4.500	316	4.500	317	4.500	318	4.500
319	4.500	320	4.500	321	4.433	322	4.367	323	4.300	324	4.233
325	4.167	326	4.100	327	4.033	328	3.967	329	3.900	330	3.833
331	3.767	332	3.700	333	3.633	334	3.567	335	3.500	336	3.448
337	3.397	338	3.345	339	3.293	340	3.241	341	3.190	342	3.138
343	3.086	344	3.034	345	2.983	346	2.931	347	2.879	348	2.828
349	2.776	350	2.724	351	2.672	352	2.621	353	2.569	354	2.517
355	2.466	356	2.414	357	2.362	358	2.310	359	2.259	360	2.207
361	2.155	362	2.103	363	2.052	364	2.000	365	0.000		

NFROOT = 1: Negative exponential representation of root growth

AA	(intersection	of the c	curve at	z=0 with abscissa	) =	1.300
В1	(coefficient d	lefining	degree c	of curvature)	==	0.13000

В2	(coefficient	that	determines	the	value	of	asymptote	=	0.020
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Root depth, density, and weight/node versus depth

DAY	MAX ROOT DEPTH	ROOT DENSITY	NORMALIZED DENSITY
		(Cm/Cm)	(1/Cm)
1	0.00	0.000	0.0000
1	5.00	0.699	0.1126
1	9.00	0.423	0.0682
1	12.00	0.293	0.0472
1	15.00	0.205	0.0330
1	18.00	0.145	0.0234
1	21.00	0.105	0.0169
1	25.00	0.070	0.0113
1	30.00	0.046	0.0075
1	35.00	0.034	0.0054
1	40.00	0.027	0.0044
1	50.00	0.022	0.0035

MXROOT (deepest node to which roots penetrate) = 12 NUPTAK = 1: Feddes et al. 1975 moisture dependent sink term

For Material No. 1

THETAW	(wiltir	ng poir	nt r	noisture	content)		=	0.1203
THETAD	(lower	limit	of	optimum	moisture	content)	=	0.1667
THETAN	(upper	limit	of	optimum	moisture	content)	=	0.3988

For Material No. 2

```
THETAW (wilting point moisture content)=0.1815THETAD (lower limit of optimum moisture content)=0.2177THETAN (upper limit of optimum moisture content)=0.3990
  For Material No. 3
  THETAW (wilting point moisture content) = 0.1815
THETAD (lower limit of optimum moisture content) = 0.2177
  THETAN (upper limit of optimum moisture content) =
                                                          0.3990
NFHOUR = 2: User subroutine for hourly PET distribution
                                                                  0.0440
   0.0100 0.0100 0.0100 0.0100 0.0100 0.0100 0.0150
   0.0699 0.0911 0.1061 0.1139 0.1139 0.1061 0.0911 0.0699
   0.0440 0.0150 0.0100 0.0100 0.0100 0.0100 0.0100 0.0100
 ET OPT = 1 and IHEAT = 0:
 PET calculated from meteorological data
 using subroutine CALPEN
ALBEDO = 2.000E-01
    ALT = 7.040E+02 (m)
```

ZU = 5.000E-01 (m) PMB = 1.000E+03 (mb)

ET OPT = 1: Meteorological Data

Temperature			ture	Solar	Wind	Cloud	
IDAY	Max	Min	Dew	Rad	Speed	Cover	Prec.
	F	F	F	ly/d	mph	tenth	in
1.	29.	12.	18.6	225.	3.9	0.0	0.03
2.	24.	Ο.	8.9	268.	2.5	0.0	0.01
3.	36.	10.	20.3	273.	4.1	0.0	0.04
4.	48.	14.	25.1	275.	2.4	0.0	0.31
5.	56.	22.	26.8	283.	1.6	0.0	0.42
6.	57.	25.	29.5	245.	3.4	0.0	0.20
7.	43.	33.	31.3	153.	7.5	0.0	0.00
8.	49.	25.	27.3	286.	3.0	0.0	0.00
9.	45.	22.	26.9	285.	4.9	0.0	0.00
10.	45.	28.	30.0	150.	5.3	0.0	0.08
11.	41.	32.	35.1	90.	3.0	0.0	0.16
12.	43.	27.	36.2	69.	6.1	0.0	0.01
13.	48.	37.	39.6	25.	13.5	0.0	0.02
14.	50.	28.	31.4	292.	6.0	0.0	0.00
15.	51.	23.	27.4	269.	1.9	0.0	0.00
16.	37.	27.	29.0	61.	7.2	0.0	0.00
17.	35.	29.	28.9	84.	9.2	0.0	0.04
18.	46.	28.	26.4	205.	5.2	0.0	0.01
19.	33.	21.	17.0	269.	8.9	0.0	0.00
20.	36.	12.	13.2	293.	3.2	0.0	0.00
21.	49.	24.	24.0	253.	2.4	0.0	0.00
22.	54.	22.	26.8	314.	2.8	0.0	0.00

23.	55.	33.	35.6	159.	4.0	0.0	0.00
24.	51.	31.	29.0	456.	5.6	0.0	0.00
25.	50.	20.	22.0	334.	8.1	0.0	0.00
26.	57.	28.	32.4	326.	11.5	0.0	0.00
27.	39.	24.	24.5	221.	7.1	0.0	0.40
28.	43.	36.	37.5	23.	5.6	0.0	2.07
29.	46.	35.	36.0	92.	7.0	0.0	0.60
30.	54.	37.	31.2	270.	9.1	0.0	0.00
31.	50.	27.	22.0	352.	6.6	0.0	0.00
32.	54.	23.	25.0	324.	7.9	0.0	0.00
33.	44.	17.	16.9	857.	20.6	0.0	0.00
34.	58.	29.	30.3	884.	23.4	0.0	0.00
35.	57.	34.	30.5	950.	31.2	0.0	0.00
36.	62.	25.	28.6	936.	23.9	0.0	0.00
37.	62.	36.	35.0	989.	14.8	0.0	0.00
38.	66.	37.	48.4	1036.	0.0	0.0	0.00
39.	73.	61.	60.4	1201.	0.0	0.0	0.00
40.	69.	25.	26.7	1065.	0.0	0.0	0.00
41.	38.	20.	19.2	943.	13.2	0.0	0.00
42.	46.	26.	26.6	943	22.5	0.0	0.00
43.	43.	38.	39.3	59	6.5	0.0	0.01
44	5.5	42.	48.0	1055	0.0	0.0	0.00
45	65	31	50 6	1160	0.0	0 0	0.00
46	31	29	28 4	25	5.8	0.0	0.00
47	34	25	25.2	927	33.4	0.0	0.00
48	38	19	16.4	872	20.8	0.0	0.15
49	29. 29	21	26.3	881	24.0	0.0	0.02
50	65	34	38 9	979	25.7	0.0	0.02
51	57	37	11 5	1029	23.7	0.0	0.00
52	16	28	28 8	102J.	25 5	0.0	0.00
52.	40.	25	26.2	972. 972	15 3	0.0	0.01
54	40.	32	369	955	32 7	0.0	2 12
55	44. 65	12.	163	1086	0.0	0.0	
56	52	36	30.5	1070	0.0	0.0	0.00
57	52. 65	36	30.7	1096	0.0	0.0	0.00
58	54	31	30 8	1103	0.0	0.0	0.00
50. 59	31	27	27.8	450 950	23.9	0.0	0.00
60.	13	30	34 5	107	6 1	0.0	0.00
61 61	43.	20.	36.7	161	2.9	0.0	0.15
62		25.	36.2	272	75	0.0	0.00
63	55. 50	35.	32 0	272.	7.5	0.0	0.00
61	56.	28	31 0	400.	7.4 5 0	0.0	0.00
65	56	20.	20.2	470.	13	0.0	0.00
66	50. 61	23.	23.2	133	4.5	0.0	0.00
67	55	30.	32.0	400.	4.9	0.0	0.00
68	55. 61	30	22.0	290. 191	5.1	0.0	0.00
60.	65	30.	20.1	404.	4.4	0.0	0.00
70	61	50	12 0	409.	9.5	0.0	0.00
70.	64. 60	20.	42.9	120	0./ E 0	0.0	0.59
/1.	59.	39.	42.0	420.	5.9	0.0	0.06
72.	73.	38.	35.6	517.	4.7	0.0	0.00
73.	66.	40.	45.8	216.	10.3	0.0	0.00
74.	53.	41.	41.9	73.	12.3	0.0	0.09
75.	52.	33.	29.5	378.	7.9	0.0	0.00
76.	54.	30.	28.6	420.	6.6	0.0	0.00
77.	50.	39.	32.9	188.	5.1	0.0	0.00
78.	55.	39.	34.6	323.	4.8	0.0	0.00

79	9. 62.	29.	31.2	544.	3.4	0.0	0.00
80	D. 72.	32.	32.8	454.	6.4	0.0	0.00
81	1. 76.	50.	42.2	437.	8.2	0.0	0.00
82	2. 74.	44.	45.0	439.	4.8	0.0	0.00
83	3. 59.	36.	28.1	376.	17.5	0.0	0.00
84	4. 45.	30.	20.4	277.	8.0	0.0	0.00
85	5. 48.	32.	17.2	463.	6.3	0.0	0.00
8 (	6. 50.	40.	26.4	183.	4.3	0.0	0.09
8.	7. 45.	40.	39.8	76.	5.0	0.0	0.05
88	3. 50.	42.	42.5	123.	2.9	0.0	0.02
80	9 65	39	42 7	513	3 1	0 0	0 00
9(	) 62	38	38 7	460	56	0.0	0.00
9-	1. 69.	33.	36.8	263	6.8	0.0	0.00
92	2 73	59	57.4	109	10.8	0.0	0.01
9.	3. 87.	66.	64.5	336.	9.4	0.0	0.00
97	4. 89.	59.	64.4	503.	5.7	0.0	0.00
9	5. 80.	67.	67.3	236.	10.7	0.0	0.00
91	6. 84.	71.	63.0	223.	15.9	0.0	0.00
91	7. 84.	67.	46.6	594	12.1	0.0	0.00
98	3. 86.	66.	63.6	425.	12.6	0.0	0.00
90	9. 86.	68.	64.5	700.	9.1	0.0	0.00
100	). <u>80</u> .	70.	65.8	163.	10.9	0.0	0.00
10	1. 75.	54.	43.7	609.	18.4	0.0	0.49
102	2. 68.	42.	41.3	349.	9.1	0.0	0.00
10:	3. 79.	40.	47.0	612.	4.1	0.0	0.00
104	4. 72.	52.	57.9	234.	7.8	0.0	0.00
. 105	5. 74.	51.	53.2	622.	7.2	0.0	0.77
100	6. 63.	45.	39.8	469.	9.7	0.0	0.00
10	7. 58.	38.	31.1	655.	8.6	0.0	0.00
108	3. 67.	32.	35.0	586.	5.8	0.0	0.00
109	9. 70.	55.	51.9	287.	14.6	0.0	0.00
11(	D. 76.	70.	64.7	203.	15.6	0.0	0.00
11:	1. 74.	68.	63.4	162.	13.7	0.0	0.00
112	2. 79.	63.	61.7	150.	16.1	0.0	1.15
113	3. 70.	46.	41.5	638.	7.3	0.0	0.10
114	4. 70.	40.	41.8	657.	2.1	0.0	0.00
115	5. 80.	45.	46.5	665.	3.7	0.0	0.00
110	5. 84.	50.	50.6	640.	5.4	0.0	0.00
11	7. 82.	54.	50.6	657.	5.5	0.0	0.00
118	3. 82.	51.	53.7	626.	3.8	0.0	0.00
119	9. 82.	52.	52.4	661.	6.3	0.0	0.00
120	). 81.	53.	52.3	535.	7.4	0.0	0.00
121	L. 84.	61.	59.8	620.	10.0	0.0	0.00
122	2. 81.	66.	62.9	494.	13.6	0.0	0.00
123	3. 82.	65.	63.2	483.	9.1	0.0	0.00
124	4. 80.	64.	62.5	356.	10.2	0.0	0.10
125	5. 76.	64.	63.1	390.	9.5	0.0	0.27
120	5. 77.	60.	62.1	353.	8.6	0.0	0.00
12	7. 76.	52.	52.9	579.	7.0	0.0	0.00
128	8. 81.	45.	51.8	694.	2.4	0.0	0.00
129	9. 84.	55.	59.2	680.	6.0	0.0	0.00
130	). 81.	64.	62.6	327.	7.9	0.0	0.07
131	L. 80.	61.	63.8	273.	5.6	0.0	0.52
132	2. 81.	59.	60.1	693.	3.7	0.0	0.00
133	3. 84.	56.	62.7	604.	5.2	0.0	0.00
134	4. 85.	66.	64.6	636.	8.2	0.0	0.00

135.	86.	67.	63.2	683.	9.9	0.0	0.00
136.	86.	68.	67.7	490.	6.8	0.0	0.00
137.	85.	65.	69.3	465.	7.3	0.0	0.46
138.	82.	65.	65.5	499	6.3	0.0	0.72
139	81	61	65.9	462	2.8	0.0	0 03
140	80	66	67 3	372	57	0.0	0.00
140.	67	50.	40 5	260	0.7	0.0	0.00
141.	77	JU.	49.J 45.5	309.	0.2	0.0	0.10
142.	//.	45.	45.5	/18.	7.1	0.0	0.00
143.	82.	49.	50.2	699.	5.8	0.0	0.02
144.	69.	49.	42.4	656.	6.3	0.0	0.00
145.	72.	48.	46.6	660.	4.4	0.0	0.00
146.	82.	47.	53.7	676.	3.4	0.0	0.00
147.	87.	58.	63.1	532.	5.3	0.0	1.47
148.	84.	61.	63.3	712.	4.7	0.0	0.00
149.	73.	65.	66.2	116.	7.8	0.0	1.19
150.	75.	62.	64.9	256.	7.4	0.0	1.47
151.	68.	57.	58.3	187.	4.9	0.0	0.00
152.	81.	50.	58.5	700.	4.5	0.0	0.00
153.	78.	68.	57.3	710.	6.9	0.0	0.00
154.	88.	67.	67.9	481.	9.3	0.0	0.00
155.	88.	77.	71.6	549.	11.3	0.0	0.00
156.	87.	73.	70.6	491.	9.8	0.0	0.00
157.	87.	69.	67.9	643.	5.4	0.0	0.00
158.	89.	67.	68.6	639.	6.5	0.0	0.00
159.	88.	68.	70.4	579.	4.0	0.0	0.08
160.	88.	69.	70.0	630.	3.1	0.0	0.00
161	89	68	70 2	629	5 2	0 0	0 00
162	90	69	68 9	718	63	0 0	0 00
163	90.	71	68 5	712	11 4	0.0	0.00
164	90.	75	72 1	559	14 0	0.0	0.00
165	20.	67	71 0	278	10.2	0.0	0.00
166	88	62	61 5	Z70. 710	3 0	0.0	0.09
167	00.	62.	60 7	710.	5.0	0.0	0.01
160	90. 00	65	61 7	600	0.5	0.0	0.00
160.	09.	63.	62.0	700.	1.5	0.0	0.00
170	90.	607.	65.9	720.	0.2	0.0	0.00
171	90.	69.	00.0	691.	6.5	0.0	0.00
$\pm / \pm \cdot$	85.	69.	69.0	480.	5.6	0.0	0.02
172.	80.	62.	64.8	419.	5.5	0.0	0.11
174	84.	57.	58.8	661. 700	3.2	0.0	0.00
1/4.	88.	60.	61.8	700.	4.2	0.0	0.00
175.	90.	63.	63.9	704.	5.0	0.0	0.00
1/6.	89.	64.	63.9	704.	5.2	0.0	0.00
177.	88.	63.	64.7	663.	4.9	0.0	0.00
178.	91.	65.	67.1	652.	4.6	0.0	0.00
179.	76.	68.	67.2	193.	8.0	0.0	1.50
180.	84.	67.	68.3	472.	9.7	0.0	0.46
181.	85.	70.	70.2	517.	4.5	0.0	0.00
182.	88.	68.	71.4	647.	3.7	0.0	0.00
183.	89.	69.	70.0	676.	4.7	0.0	0.00
184.	90.	68.	70.4	616.	4.9	0.0	0.00
185.	96.	70.	71.2	677.	5.8	0.0	0.00
186.	91.	75.	72.8	474.	6.3	0.0	0.00
187.	97.	73.	74.3	698.	6.3	0.0	0.00
188.	96.	77.	73.6	703.	7.5	0.0	0.00
189.	97.	74.	72.2	701.	6.9	0.0	0.00
190.	98.	74.	72.0	706.	4.6	0.0	0.00

191. 192. 193. 194. 195. 196.	97. 97. 101. 82. 86. 87.	72. 73. 77. 75. 70. 66.	72.7 73.9 73.4 72.3 65.0 67.0	649. 678. 646. 299. 516. 451.	4.0 5.1 5.8 9.7 5.9 7.1	0.0 0.0 0.0 0.0 0.0 0.0	0.00 0.00 0.00 0.00 0.00 0.00
197. 198. 199.	94. 98. 98.	74. 78. 76.	73.6 75.0 74.6	510. 664. 663.	9.9 8.5 8.0	0.0 0.0 0.0	0.18 0.00 0.00
200. 201. 202. 203. 204. 205. 206. 207. 208. 209. 210.	97. 95. 100. 100. 100. 100. 99. 97. 95. 97. 95.	79. 75. 73. 74. 75. 75. 75. 75. 75. 75.	73.7 71.4 71.6 70.5 70.3 73.3 72.7 73.8 73.8 73.8 73.5 74.9	667. 665. 674. 686. 666. 656. 603. 513. 410. 553. 579.	7.2 6.3 5.3 6.4 6.2 4.3 6.2 4.7 6.1 6.5 5.5	0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0	$\begin{array}{c} 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\$
<ol> <li>211.</li> <li>212.</li> <li>213.</li> <li>214.</li> <li>215.</li> <li>216.</li> <li>217.</li> <li>218.</li> </ol>	100. 99. 97. 97. 101. 101. 98.	76. 77. 72. 74. 69. 70. 74. 68.	73.0 71.3 69.8 71.9 68.6 69.0 70.4 66.2	628. 654. 660. 631. 642. 649. 651. 641.	7.1 7.2 5.7 5.8 2.7 2.6 3.9 3.9	$\begin{array}{c} 0.0\\ 0.0\\ 0.0\\ 0.0\\ 0.0\\ 0.0\\ 0.0\\ 0.0$	0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.0
<ul> <li>219.</li> <li>220.</li> <li>221.</li> <li>222.</li> <li>223.</li> <li>224.</li> <li>225.</li> </ul>	97. 98. 104. 98. 88. 94. 96.	70. 73. 79. 74. 73. 73. 70.	70.3 70.0 68.7 71.6 74.1 73.4 66.7	609. 555. 595. 335. 495. 597.	3.0 4.5 6.8 4.2 2.1 3.4 4.5	0.0 0.0 0.0 0.0 0.0 0.0	0.00 0.00 0.28 0.00 0.00 0.00
<ol> <li>226.</li> <li>227.</li> <li>228.</li> <li>229.</li> <li>230.</li> <li>231.</li> <li>232.</li> <li>233.</li> </ol>	95. 95. 91. 93. 93. 92. 98. 96.	<ol> <li>66.</li> <li>70.</li> <li>68.</li> <li>67.</li> <li>68.</li> <li>66.</li> <li>67.</li> <li>73.</li> </ol>	63.0 68.0 68.1 69.1 69.7 59.7 65.6 67.6	637. 419. 585. 590. 581. 627. 557. 570.	9.3 9.3 3.1 4.4 5.3 5.3 6.5 9.2	0.0 0.0 0.0 0.0 0.0 0.0 0.0	0.00 0.73 0.00 0.00 0.00 0.00 0.00 0.00
234. 235. 236. 237. 238. 239. 240.	96. 90. 92. 97. 92. 94. 93.	78. 74. 73. 73. 69. 71. 68.	68.3 69.5 68.0 70.4 70.3 70.8 70.0	585. 218. 388. 395. 533. 606. 591.	5.1 6.3 8.7 7.2 4.0 2.6 3.8	0.0 0.0 0.0 0.0 0.0 0.0 0.0	0.00 0.32 0.07 0.49 0.00 0.00 0.00
241. 242. 243. 244. 245. 246.	90. 86. 90. 92. 94.	68. 73. 66. 66. 67. 68.	68.3 69.5 65.9 67.5 68.6 68.9	555. 492. 492. 571. 581. 495.	5.4 7.0 4.9 4.0 3.4 2.8	0.0 0.0 0.0 0.0 0.0	0.00 0.00 0.00 0.00 0.00

247.	91.	68.	69.3	382.	3.0	0.0	0.00
248.	88.	72.	71.4	243.	3.7	0.0	0.03
249.	88.	72.	70.7	402.	10.2	0.0	0.01
250.	91.	66.	70.3	380.	14.1	0.0	0.41
251.	77.	62.	64.4	213.	3.9	0.0	0.86
252.	79.	55.	56.2	529.	5.2	0.0	0.01
253.	82.	49.	53.8	588.	2.0	0.0	0.00
254.	84.	52	56.8	569	3.1	0.0	0.00
255	86	55	58 7	544	2 5	0.0	0 00
256	88	55	60.4	552	2.5	0.0	0.00
250.	88	60 60	61 1	102	2.1 1 1	0.0	0.00
258	76	66	65 /	187	4.I 6 6	0.0	0.00
250.	70. 77	60.	60.4	144	1.0	0.0	0.20
259.	//. 0/	70	60.4	144. 000	4.0	0.0	
200.	04. 77	70. CC	09.9	232.	0.0	0.0	1 04
201.	11.	66.	01.0	202.	7.0	0.0	1.04
262.	81.	63.	61.1	4/9.	4.0	0.0	0.00
263.	//.	58.	62.9	210.	4.8	0.0	0.00
264.	80.	62.	65.6	315.	3.9	0.0	0.00
265.	85.	60.	66.2	405.	2.8	0.0	0.00
266.	82.	60.	61.3	437.	9.1	0.0	0.01
267.	69.	50.	47.9	418.	9.9	0.0	0.00
268.	71.	42.	45.3	527.	2.6	0.0	0.00
269.	77.	40.	48.6	515.	2.0	0.0	0.00
270.	78.	48.	51.4	490.	3.6	0.0	0.00
271.	80.	50.	53.8	484.	2.7	0.0	0.00
272.	77.	48.	51.0	498.	4.0	0.0	0.00
273.	78.	44.	49.5	500.	1.9	0.0	0.00
274.	80.	44.	49.9	493.	1.7	0.0	0.00
275.	81.	47.	51.4	486.	6.0	0.0	0.01
276.	82.	57.	55.6	436.	7.9	0.0	0.00
277.	83.	63.	63.4	378.	9.3	0.0	0.00
278.	73.	38.	48.8	167.	11.2	0.0	0.09
279.	68.	33.	38.0	483	2.0	0.0	0.00
280.	69	36	40 5	466	56	0.0	0 00
281	76	47	48 9	445	11 7	0.0	0 00
282	73	66	40.J	126	12 2	0.0	0.00
283	75	60.	63 /	27	7 1	0.0	211
203.	62	57	50 7	Z7. 57	7.4	0.0	2.14
204.	02.	57.	50.7	J/. 212	3.5	0.0	0.24
205.	73. 60	JU. 47	JO,/ 51 5	262	1.0	0.0	0.07
200.	09. 71	47.	11.0	203.	4.Z 2 E	0.0	0.00
207.	72.	43.	44.5	407.	3.5	0.0	0.00
200.	15.	57. 21	44.5	Z/1. 440	9.5	0.0	0.00
209.	00.	51.	34.8	449.	2.5	0.0	0.00
290.	70.	36.	37.5	436.	6.8	0.0	0.00
291.	/5.	53.	47.2	405.	10.4	0.0	0.00
292.	68.	46.	52.5	184.	2.4	0.0	0.00
293.	82.	47.	57.0	370.	5.6	0.0	0.00
294.	80.	60.	59.4	384.	8.4	0.0	0.00
295.	80.	58.	62.8	245.	9.3	0.0	0.00
296.	85.	58.	61.2	336.	7.6	0.0	0.00
297.	72.	42.	44.9	398.	8.9	0.0	0.00
298.	67.	34.	29.8	408.	4.2	0.0	0.00
299.	67.	31.	30.8	398.	3.2	0.0	0.00
300.	65.	30.	30.8	400.	3.1	0.0	0.00
301.	70.	39.	34.6	380.	9.4	0.0	0.00
302.	75.	48.	40.5	333.	8.5	0.0	0.00

303.	73.	43.	41.2	378.	6.3	0.0	0.00
304.	71.	53.	49.8	220.	12.9	0.0	0.00
305.	78.	62.	60.5	148.	11.3	0.0	0.00
306.	69.	61.	62.8	86.	5.1	0.0	0.00
307.	64.	59.	61.4	46.	4.4	0.0	2.78
308	74	53	593	214	1 5	0 0	0 00
300. 309	75	19	55.0	223 773	1 2	0.0	0.00
310	יטי. רר	47	53.0 53.5	200	2.0	0.0	0.00
31U. 311	70	4/.	10.5	294.	5.9	0.0	0.00
311. 210	78.	49.	48.3	337.	5.9	0.0	0.00
312.	6Z.	49.	46.9	224.	8.4	0.0	0.03
313.	58.	40.	40.2	236.	4.8	0.0	0.00
314.	/1.	45.	45.2	332.	2.9	0.0	0.00
315.	71.	40.	46.6	315.	3.0	0.0	0.00
316.	68.	45.	47.4	218.	7.0	0.0	0.00
317.	72.	48.	53.2	225.	8.6	0.0	0.00
318.	72.	55.	54.8	226.	8.2	0.0	0.00
319.	72.	46.	51.0	275.	2.9	0.0	0.00
320.	69.	45.	50.9	139.	1.9	0.0	0.00
321.	67.	56.	53.2	104.	3.7	0.0	0.00
322.	74.	55.	57.1	222.	8.2	0.0	0.41
323.	61.	32.	34.0	254.	11.9	0.0	0.35
324.	56.	26.	26.2	308.	2.6	0.0	0.00
325.	59.	28.	29.5	252.	5.3	0.0	0.00
326.	67.	41	42 4	283	11 2	0 0	0 00
327	73	53	55 3	122	13 2	0.0	0.00
01.		00.	00.0	122.	10.2	0.0	0.00
328.	59.	48.	41.7	60.	11.3	0.0	0.00
329.	70.	33.	37.5	294	8.2	0.0	0.00
330.	63.	38.	37.1	81.	11.3	0.0	0.00
331.	39.	29	21.2	162	8.4	0.0	0.00
332	30	26	25.8	55	13 1	0.0	0.00
333	34	28	28.5	51	67	0.0	0 00
334	57	30	32 6	201	79	0.0	0.00
335	62	28	37 1	100	3 1	0.0	0.01
336	71	20.	11 2	199.	J.1 7 6	0.0	0.00
227	62	55.	44.J 54 0	200.	7.0	0.0	0.00
220	75	55. 61	54.0	/⊥. 1⊑⊑	9.0 12 C	0.0	0.00
220.	7J. 75	01. E1		100.	12.0	0.0	0.00
240	15.	DI.	30.0	95.	12.2	0.0	0.00
54U.	66.	38.	39.0	140.	3.0	0.0	0.00
341. 240	67.	33.	43.0	206.	4.1	0.0	0.00
342.	53.	28.	27.4	257.	10.0	0.0	0.00
343.	55.	22.	23.4	288.	1.4	0.0	0.00
344.	56.	20.	25.1	285.	5.2	0.0	0.00
345.	51.	28.	35.3	141.	8.9	0.0	0.26
346.	52.	46.	48.7	31.	4.0	0.0	0.13
347.	49.	44.	45.0	54.	4.6	0.0	0.25
348.	48.	37.	40.7	111.	6.6	0.0	0.00
349.	49.	40.	42.2	52.	11.4	0.0	0.00
350.	53.	47.	49.3	16.	6.1	0.0	1.49
351.	57.	35.	37.8	276.	5.3	0.0	0.33
352.	66.	31.	35.5	277.	7.9	0.0	0.00
353.	50.	28.	23.5	284.	7.0	0.0	0.00
354.	61.	27.	28.2	275.	4.9	0.0	0.00
355.	63.	32.	33.2	268.	10.6	0.0	0.00
356.	60.	28.	36.6	268.	11.5	0.0	0.00
357.	43.	21.	14.7	279.	6.1	0.0	0.00
358.	42.	19.	16.1	289.	5.4	0.0	0.00

359.	39.	16.	19.1	261.	2.4	0.0	0.00
360.	39.	21.	14.7	284.	6.5	0.0	0.00
361.	51.	23.	26.0	233.	2.8	0.0	0.00
362.	58.	22.	26.7	230.	4.0	0.0	0.00
363.	37.	20.	14.2	275.	8.6	0.0	0.00
364.	27.	16.	15.1	76.	3.3	0.0	0.00
365.	38.	13.	16.4	266.	1.6	0.0	0.00

NFPET = 1:

PET is partitioned into PT and PE according to the relationship developed by Ritchie (1972)

DAY	PET	PTRANS	PEVAPO	DAY	PET	PTRANS	PEVAPO
1	0.0344	0.0000	0.0344	2	0.0446	0.0000	0.0446
3	0.0652	0.0000	0.0652	4	0.0920	0.0000	0.0920
5	0.1353	0.0000	0.1353	6	0.1549	0.0000	0.1549
7	0.1085	0.0000	0.1085	8	0.1473	0.0000	0.1473
9	0.1392	0.0000	0.1392	10	0.0714	0.0000	0.0714
11	0.0000	0.0000	0.0000	12	0.0000	0.0000	0.0000
13	0.0037	0.0000	0.0037	14	0.1895	0.0000	0.1895
15	0.1157	0.0000	0.1157	16	0.0000	0.0000	0.0000
17	0.0122	0.0000	0.0122	18	0.1497	0.0000	0.1497
19	0.2042	0.0000	0.2042	20	0.1231	0.0000	0.1231
21	0.1310	0.0000	0.1310	22	0.1736	0.0000	0.1736
23	0.0905	0.0000	0.0905	24	0.3505	0.0000	0.3505
25	0.3079	0.0000	0.3079	26	0.3789	0.0000	0.3789
27	0.1306	0.0000	0.1306	28	0.0000	0.0000	0.0000
29	0.0307	0.0000	0.0307	30	0.3811	0.0000	0.3811
31	0.3426	0.0000	0.3426	32	0.3257	0.0000	0.3257
33	0.8340	0.0000	0.8340	34	1.1020	0.0000	1.1620
22	1 1146	0.0000	1.3081	20	1.2/92	0.0000	1.2/92
30	1 0053	0.0000	1.0053	30	0.6810	0.0000	0.6010
11	1.0055	0.0000	1.0055	40	0.8859	0.0000	0.0911
43	0.0200	0.0000	0.0200	42	0.6556	0.0000	0.6556
45	0.7157	0.0000	0.0000	46	0.0000	0.0000	0.0000
47	0.6604	0.0000	0.6604	48	0.7685	0.0000	0.7685
49	0.8301	0.0000	0.8301	50	1.2925	0.0000	1.2925
51	0.6273	0.0000	0.6273	52	0.9353	0.0000	0.9353
53	0.8044	0.0000	0.8044	54	0.5666	0.0000	0.5666
55	0.7538	0.0000	0.7538	56	0.6519	0.0000	0.6519
57	0.7344	0.0000	0.7344	58	0.6304	0.0000	0.6304
59	0.4525	0.0000	0.4525	60	0.0000	0.0000	0.0000
61	0.0021	0.0000	0.0021	62	0.2246	0.0000	0.2246
63	0.4896	0.0000	0.4896	64	0.3410	0.0000	0.3410
65	0.2563	0.0000	0.2563	66	0.3918	0.0000	0.3918
67	0.2355	0.0000	0.2355	68	0.4158	0.0000	0.4158
69	0.6366	0.0000	0.6366	70	0.3189	0.0000	0.3189
71	0.3127	0.0000	0.3127	72	0.5490	0.0000	0.5490
73	0.2615	0.0000	0.2615	74	0.1037	0.0000	0.1037
75	0.3805	0.0000	0.3805	76	0.3753	0.0000	0.3753
77	0.1698	0.0000	0.1698	78	0.2764	0.0000	0.2764
79	0.4015	0.0000	0.4015	80	0.5375	0.0000	0.5375
81	0.7257	0.6894	0.0363	82	0.4540	0.4313	0.0227
83	0.8822	0.8381	0.0441	84	0.3375	0.3207	0.0169
85	0.4707	0.4471	0.0235	86	0.2076	0.1972	0.0104

87	0.0000	0.0000	0.0000	88	0.0020	0.0019	0.0001
89	0.3704	0.3519	0.0185	90	0.4033	0.3831	0.0202
91	0.3358	0.3190	0.0168	92	0.2821	0.2680	0.0141
93	0.6193	0.5884	0.0310	94	0.5844	0.5552	0.0292
95	0.3606	0.3425	0.0180	96	0.8724	0.8288	0.0436
97	1.4021	1,3320	0.0701	98	0.8415	0.7995	0.0421
99	0.9910	0,9415	0.0496	100	0.4049	0.3847	0.0202
101	1.4297	1.3582	0.0715	102	0.4977	0.4728	0.0249
103	0.5631	0.5349	0.0282	104	0.1902	0.1807	0.0095
105	0.6423	0.6102	0.0321	106	0.6165	0.5856	0.0308
107	0.7052	0.6699	0.0353	108	0.5406	0.5136	0.0270
109	0.5963	0.5665	0.0298	110	0.5313	0.5047	0.0266
111	0.3974	0.3775	0.0199	112	0.5262	0.4999	0.0263
113	0.7438	0.7066	0.0372	114	0.5061	0.4808	0.0253
115	0.6527	0.6201	0.0326	116	0.7484	0.7110	0.0374
117	0.7901	0.7506	0.0395	118	0.6231	0.5919	0.0312
119	0.7811	0.7421	0.0391	120	0.7122	0.6766	0.0356
121	0.9063	0.8610	0.0453	122	0.8571	0.8142	0.0429
123	0.6854	0.6511	0.0343	124	0.5626	0.5345	0.0281
125	0.4837	0.4595	0.0242	126	0.4056	0.3853	0.0203
127	0.6455	0.6132	0.0323	128	0.5944	0.5646	0.0297
129	0.7397	0.7027	0.0370	130	0.4770	0.4531	0.0238
131	0.2837	0.2695	0.0142	132	0.6780	0.6441	0.0339
133	0.5976	0.5678	0.0299	134	0.8391	0.7971	0.0420
135	1.0227	0.9716	0.0511	136	0.6221	0.5910	0.0311
137	0.5109	0.4854	0.0255	138	0.5629	0.5348	0.0281
139	0.3874	0.3680	0.0194	140	0.3751	0.3563	0.0188
141	0.4134	0.3928	0.0207	142	0.8201	0.7791	0.0410
143	0.7913	0.7517	0.0396	144	0.7289	0.6925	0.0364
145	0.6285	0.5971	0.0314	146	0.6186	0.5877	0.0309
147	0.5850	0.5558	0.0293	148	0.7391	0.7022	0.0370
149	0.0709	0.0674	0.0035	150	0.2189	0.2080	0.0109
151	0.1140	0.1083	0.0057	152	0.6301	0.5986	0.0315
153	0.9309	0.8844	0.0465	154	0.7048	0.6696	0.0352
155	0.9301	0.8836	0.0465	156	0.7459	0.7086	0.0373
157	0.7551	0.7173	0.0378	158	0.7739	0.7352	0.0387
159	0.6097	0.5792	0.0305	160	0.6518	0.6192	0.0326
161	0.7081	0.6727	0.0354	162	0.8855	0.8412	0.0443
163	1.1189	1.0630	0.0559	164	1.0209	0.9699	0.0510
165	0.4273	0.4059	0.0214	166	0.7487	0.7113	0.0374
167	0.9632	0.9151	0.0482	168	0.8810	0.8370	0.0441
169	1.0437	0.9916	0.0522	170	0.9056	0.8604	0.0453
171	0.5513	0.5237	0.0276	172	0.4140	0.3933	0.0207
173	0.6546	0.6219	0.0327	174	0.7641	0.7259	0.0382
175	0.8270	0.7857	0.0414	176	0.8353	0.7935	0.0418
177	0.7473	0.7099	0.0374	178	0.7491	0.7116	0.0375
179	0.2159	0.2051	0.0108	180	0.6179	0.5870	0.0309
181	0.5507	0.5232	0.0275	182	0.6576	0.6247	0.0329
183	0.7570	0.7192	0.0379	184	0.6957	0.6609	0.0348
185	0.8795	0.8355	0.0440	186	0.6555	0.6227	0.0328
187	0.9202	0.8742	0.0460	188	1.0349	0.9831	0.0517
189	1.0046	0.9543	0.0502	190	0.9197	0.8737	0.0460
191	0.7917	0.7521	0.0396	192	0.8609	0.8178	0.0430
193	0.9664	0.9181	0.0483	194	0.4335	0.4118	0.0217
195	0.6941	0.6594	0.0347	196	0.5936	0.5639	0.0297
197	0.8362	0.7944	0.0418	198	1.0559	1.0031	0.0528
T 8 8	1.0075	0.9571	0.0504	200	1.0264	0.9751	0.0513

201	0.9393	0.8923	0.0470	202	0.9369	0.8900	0.0468
203	1.0363	0.9845	0.0518	204	0.9943	0.9445	0.0497
205	0 8685	0 8251	0 0434	206	0 9519	0 9043	0 0476
203	0.6932	0.6585	0.0347	200	0.6550	0 6223	0.0328
207	0.0000	0.0000	0.0347	210	0.0550	0.0225	0.0320
209	0.0400	0.0037	0.0423	210	1 0501	1 0000	0.0577
211	0.9938	0.9441	0.0497	212	1.0591	1.0062	0.0530
213	0.9442	0.8970	0.0472	214	0.8850	0.840/	0.0442
215	0.7401	0.7031	0.0370	216	0.7783	0.7394	0.0389
217	0.8736	0.8299	0.0437	218	0.8200	0.7790	0.0410
219	0.7123	0.6767	0.0356	220	0.7677	0.7293	0.0384
221	1.1037	1.0485	0.0552	222	0.6909	0.6564	0.0345
223	0.3156	0.2998	0.0158	224	0.5781	0.5492	0.0289
225	0.7996	0.7596	0.0400	226	0.8013	0.7612	0.0401
227	0.8153	0.7745	0.0408	228	0.6433	0.6112	0.0322
229	0.6946	0.6599	0.0347	230	0.7213	0.6853	0.0361
231	0.8749	0.8311	0.0437	232	0.8671	0.8237	0.0434
233	1.0454	0.9931	0.0523	234	0.8848	0.8405	0.0442
235	0 4236	0 4024	0 0212	236	0 7541	0 7164	0 0377
237	0 7128	0 6771	0.0356	238	0 6166	0 5858	0 0308
239	0.7120	0.6387	0.0336	240	0.6731	0.5050	0.0337
235	0.6941	0.6100	0.0342	240	0.0751	0.6327	0.0333
241	0.0041	0.0499	0.0342	242	0.0000	0.0327	0.0333
243	0.6043	0.5741	0.0302	244	0.0399	0.0079	0.0320
245	0.6446	0.6124	0.0322	246	0.5561	0.5283	0.0278
247	0.4198	0.3988	0.0210	248	0.2844	0.2701	0.0142
249	0.6649	0.6317	0.0332	250	0.7054	0.6/01	0.0353
251	0.1615	0.1535	0.0081	252	0.5604	0.5324	0.0280
253	0.5079	0.4825	0.0254	254	0.5406	0.5136	0.0270
255	0.5187	0.4927	0.0259	256	0.5131	0.4874	0.0257
257	0.4341	0.4124	0.0217	258	0.2004	0.1904	0.0100
259	0.1046	0.0994	0.0052	260	0.3088	0.2934	0.0154
261	0.2311	0.2195	0.0116	262	0.5095	0.4840	0.0255
263	0.1579	0.1500	0.0079	264	0.2702	0.2567	0.0135
265	0.3548	0.3370	0.0177	266	0.6048	0.5746	0.0302
267	0.5710	0.5424	0.0285	268	0.4115	0.3910	0.0206
269	0.3871	0.3678	0.0194	270	0.4552	0.4324	0.0228
271	0.4293	0.4079	0.0215	272	0.4714	0.4478	0.0236
273	0.3963	0.3765	0.0198	274	0.3929	0.3732	0.0196
275	0.5537	0.5260	0.0277	276	0.6461	0.6138	0.0323
277	0.5661	0.5378	0.0283	278	0.2359	0.2241	0.0118
279	0.3330	0.3163	0.0166	280	0.4341	0.4124	0.0217
281	0.6975	0.6626	0.0349	282	0.3427	0.3255	0.0171
283	0.0145	0.0138	0.0007	284	0.0000	0.0000	0.0000
285	0.3002	0.2852	0.0150	286	0.1890	0.1795	0.0094
287	0 3567	0 3388	0 0178	288	0 3780	0 3591	0 0189
289	0.3196	0.3037	0.0160	290	0.5014	0.4763	0.0251
205	0.7384	0.7015	0.0100	290	0.0683	0.4703	0.0231
203	0.7504	0.7015	0.0303	292	0.0005	0.0040	0.0034
295	0.3303	0.3300	0.0159	294	0.3400	0.0192	0.0275
295	0.5134	0.2997	0.0158	290	0.4702	0.4545	0.0239
291	0.5144	0.4007	0.0257	290	0.4111	0.3903	0.0200
299	0.3300	0.3199	0.0100	300	0.3100	0.3010	0.0100
202	0.0412	0.0091	0.0321	302	16591	0.00/1	0.0320
303	0.4886	0.4641	0.0244	304	0.5330	0.5063	0.0266
305	0.3829	0.3638	0.0191	306	0.0000	0.0000	0.0000
307	0.0000	0.0000	0.0000	308	0.0963	0.0914	0.0048
309	0.1217	0.1156	0.0061	310	0.2510	0.2384	0.0125
311	0.4512	0.4286	0.0226	312	0.2730	0.2594	0.0137
313	0.1789	0.1700	0.0089	314	0.2793	0.2654	0.0140

315	0.2241	0.2129	0.0112	316	0.2495	0.2370	0.0125
317	0.2563	0.2435	0.0128	318	0.3105	0.2949	0.0155
319	0.1929	0.1832	0.0096	320	0.0373	0.0355	0.0019
321	0.0752	0.0714	0.0038	322	0.2803	0.2663	0.0140
323	0.4231	0.4020	0.0212	324	0.1968	0.1870	0.0098
325	0.2415	0.2294	0.0121	326	0.4572	0.4343	0.0229
327	0.3076	0.2922	0.0154	328	0.2844	0.2702	0.0142
329	0.4053	0.3851	0.0203	330	0.3241	0.3079	0.0162
331	0.2078	0.1974	0.0104	332	0.0000	0.0000	0.0000
333	0.0000	0.0000	0.0000	334	0.2953	0.2806	0.0148
335	0.0960	0.0912	0.0048	336	0.2655	0.0000	0.2655
337	0.0183	0.0000	0.0183	338	0.3385	0.0000	0.3385
339	0.3181	0.0000	0.3181	340	0.1046	0.0000	0.1046
341	0.1241	0.0000	0.1241	342	0.3445	0.0000	0.3445
343	0.1436	0.0000	0.1436	344	0.2246	0.0000	0.2246
345	0.0740	0.0000	0.0740	346	0.0000	0.0000	0.0000
347	0.0000	0.0000	0.0000	348	0.0000	0.0000	0.0000
349	0.0000	0.0000	0.0000	350	0.0000	0.0000	0.0000
351	0.2005	0.0000	0.2005	352	0.3478	0.0000	0.3478
353	0.3040	0.0000	0.3040	354	0.2643	0.0000	0.2643
355	0.4404	0.0000	0.4404	356	0.2832	0.0000	0.2832
357	0.2536	0.0000	0.2536	358	0.2110	0.0000	0.2110
359	0.0921	0.0000	0.0921	360	0.2384	0.0000	0.2384
361	0.1201	0.0000	0.1201	362	0.1714	0.0000	0.1714
363	0.2621	0.0000	0.2621	364	0.0000	0.0000	0.0000
365	0.0829	0.0000	0.0829				

Totals: PET = 148.7921 PTRANS = 138.6167 PEVAPO = 44.5650

IRAIN	= 1	
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## Rainfall/Irrigation Details

Day	Time (hr)	Amount (cm)	Application Type	Efficiency	Changes In Rate/Head
				1 000	
1	0.000	0.0762	$\perp$	1.000	Ζ.
0	1.000	0.0000	1	1 000	0
2	0.000	0.0254	T	1.000	2
-	1.000	0.0000			
3	0.000	0.1016	1	1.000	2
	1.000	0.0000			
4	0.000	0.7874	1	1.000	2
	1.000	0.0000			
5	0.000	1.0000	1	1.000	3
	1.000	0.0668			
	2.000	0.0000			
6	0.000	0.5080	1	1.000	2
	1.000	0.0000			
10	0.000	0.2032	1	1.000	2
	1.000	0.0000			
11	0.000	0.4064	1	1.000	2
	1.000	0.0000			
12	0.000	0.0254	1	1.000	2
	1.000	0.0000			

13	0.000	0.0508	1	1.000	2
17	0.000	0.1016	1	1.000	2
18	$1.000 \\ 0.000$	0.0000 0.0254	1	1.000	2
27	1.000	0.0000	1	1 000	2
21	1.000	0.0160	T	1.000	3
	2.000	0.0000			
28	0.000	5.0000	1	1.000	3
	5.000	0.2578			
20	6.000	0.0000	1	1 000	2
29	1 000	1.0000	Т	1.000	2
	2,000	0.0000			
43	0.000	0.0254	1	1.000	2
	1.000	0.0000			
47	0.000	0.3810	1	1.000	2
	1.000	0.0000			
48	0.000	0.1270	1	1.000	2
19	1.000	0.0000	1	1 000	2
	1.000	0.0000	T	1.000	6
52	0.000	0.0254	1	1.000	2
	1.000	0.0000			
54	0.000	5.0000	1	1.000	3
	5.000	0.3848			
6.0	6.000	0.0000		1 000	0
60	0.000	0.3810	1	1.000	2
70	0 000	1 0000	1	1 000	З
70	1.000	0.4986	Т	1.000	5
	2.000	0.0000			
71	0.000	0.1524	1	1.000	2
	1.000	0.0000			
74	0.000	0.2286	1	1.000	2
86	1.000	0.0000	1	1 000	2
00	1 000	0.2280	T	1.000	2
87	0.000	0.1270	1	1.000	2
	1.000	0.0000			
88	0.000	0.0508	1	1.000	2
	1.000	0.0000			
00	0 000	0 0254	1	1 000	2
90	1.000	0.0204	T	1.000	2
92	0.000	0.0254	1	1.000	2
	1.000	0.0000			
101	0.000	1.0000	1	1.000	3
	1.000	0.2446			
105	2.000	0.0000	4	1 000	2
105	1 000	1.0000	T	1.000	3
	2.000	0.0000			
112	0.000	2.0000	1	1.000	3
	2.000	0.9210	-		-
	3.000	0.0000			

113	0.000 1.000	0.2540 0.0000	1	1.000	2
124	0.000	0.2540	1	1.000	2
125	0.000	0.6858	1	1.000	2
130	0.000	0.1778	1	1.000	2
131	0.000	1.0000	1	1.000	3
	1.000 2.000	0.3208 0.0000			
137	0.000 1.000	1.0000 0.1684	1	1.000	3
138	2.000 0.000	0.0000 1.0000	1	1.000	3
	1.000	0.8288			
139	0.000	0.0762	1	1.000	2
140	0.000	1.0000	1	1.000	3
7 4 7	2.000	0.0000	1	1 000	0
141	1.000	0.2540	Ţ	1.000	Z
143	0.000 1.000	0.0508 0.0000	1	1.000	2
147	0.000 3.000	3.0000 0.7338	1	1.000	3
149	4.000	0.0000	1	1.000	3
	3.000	0.0226	_		-
150	0.000	3.0000	1	1.000	3
	4.000	0.0000			
159	0.000 1.000	0.2032 0.0000	1	1.000	2
165	0.000 2.000	2.0000 0.2606	1	1.000	3
166	3.000	0.0000	1	1 000	2
100	1.000	0.0000	1	1.000	2
171	0.000 1.000	0.0508 0.0000	1	1.000	2
172	0.000 1.000	0.2794 0.0000	1	1.000	2
179	0.000	3.0000	1	1.000	3
100	4.000	0.0000	1	1 000	2
180	1.000	0.1684	Ţ	1.000	3
197	2.000 0.000	0.0000 0.4572	1	1.000	2
209	1.000	0.0000 0.1524	1	1.000	2
222	1.000	0.0000	-	1 000	-
<i>LLL</i>	0.000	0./112	Ŧ	T.000	4

	1.000	0.0000			
227	0.000	1.0000	1	1.000	3
	1.000	0.8542			
	2.000	0 0000			
235	0.000	0.8128	1	1 000	2
200	1 000	0.0120	T	1.000	2
000	1.000	0.0000	-	1 000	0
236	0.000	0.1778	1	1.000	2
	1.000	0.0000			
237	0.000	1.0000	1	1.000	3
	1.000	0.2446			
	2.000	0.0000			
248	0.000	0.0762	1	1 000	2
210	1 000	0 0000	-	1.000	2
210	1,000	0.0000	1	1 000	2
249	1 000	0.0254	T	1.000	2
0 - 0	1.000	0.0000	_		
250	0.000	1.0000	1	1.000	3
	1.000	0.0414			
	2.000	0.0000			
251	0.000	2.0000	1	1.000	3
	2.000	0.1844			
	3.000	0 0000			
252	0 000	0.0254	1	1 000	2
292	1 000	0.0254	Ŧ	1.000	2
050	1.000	0.0000		1 000	0
258	0.000	0.6604	1	1.000	2
	1.000	0.0000			
259	0.000	0.4572	1	1.000	· 2
	1.000	0.0000			
260	0.000	0.0254	1	1.000	2
	1.000	0 0000	_		_
261	0 000	2 0000	1	1 000	З
201	2 000	2.0000	1	1.000	5
	2.000	0.6416			
	3.000	0.0000			
266	0.000	0.0254	1	1.000	2
	1.000	0.0000			
275	0.000	0.0254	1	1.000	2
	1.000	0.0000			
278	0.000	0.2286	1	1.000	2
	1 000	0 0000	-		_
282	0 000	1 0000	1	1 000	З
202	1 000	1.0000	1	1.000	5
	1.000	0.4986			
	2.000	0.0000			
283	0.000	5.0000	1	1.000	3
	5.000	0.4356			
	6.000	0.0000			
284	0.000	0.6096	1	1.000	2
	1.000	0.0000	_		_
285	0.000	1 0000	1	1 000	2
205	1 000	1.0000	T	1.000	5
	1.000	0.44/8			
	2.000	0.0000			
307	0.000	7.0000	1	1.000	3
	7.000	0.0612			
	8.000	0.0000			
312	0.000	0.0762	1	1.000	2
	1.000	0.0000	-	2.000	2
300	0.000	1 0000	1	1 000	2
J22	1 000	1.0000	1	T.000	3
	1.000	0.0414			
	2.000	0.0000			

323	0.000	0.8890	1	1.000	2
	1.000	0.0000			
334	0.000	0.7874	1	1.000	2
	1.000	0.0000			
345	0.000	0.6604	1	1.000	2
	1.000	0.0000			
346	0.000	0.3302	1	1.000	2
	1.000	0.0000			
347	0.000	0.6350	1	1.000	2
	1.000	0.0000			
350	0.000	3.0000	1	1.000	3
	3.000	0.7846			
	4.000	0.0000			
351	0.000	0.8382	1	1.000	2
	1.000	0.0000			

NWATER (number of days of rain/irrigation) = 87

Total Water Applied = 89.5604 cm

## UNSAT-H Version 2.03 INITIAL CONDITIONS

Input Filename:	S:\Nevzat\WinUnsatH\project.inp
Results Filename:	S:\Nevzat\WinUnsatH\project.res
Date of Run:	15-JUL-**
Time of Run:	13:15:16
Title:	
Muskogee AFC Alte:	rnative Final Cover

_										
			Initial	Conditi	ons			Initial	Conditi	ons
	NODE	DEPTH (cm)	HEAD (cm)	THETA (vol.)	TEMP (K)	NODE	DEPTH (cm)	HEAD (cm)	THETA (vol.)	TEMP (K)
	1	0.000E+00	5.035E+05	0.0875	0.00	2	5.000E+00	8.787E+02	0.2319	0.00
	3	9.000E+00	8.557E+02	0.2337	0.00	4	1.200E+01	8.410E+02	0.2348	0.00
	5	1.500E+01	8.288E+02	0.2358	0.00	6	1.800E+01	8.157E+02	0.2726	0.00
	7	2.100E+01	8.025E+02	0.2734	0.00	8	2.500E+01	7.927E+02	0.2741	0.00
	9	3.000E+01	7.914E+02	0.2741	0.00	10	3.500E+01	8.016E+02	0.2735	0.00
	11	4.000E+01	8.244E+02	0.2720	0.00	12	5.000E+01	9.183E+02	0.2665	0.00
	13	6.000E+01	1.097E+03	0.2577	0.00	14	7.000E+01	1.307E+03	0.2496	0.00
	15	8.000E+01	1.406E+03	0.2463	0.00	16	8.500E+01	1.414E+03	0.2461	0.00
	17	8.800E+01	1.411E+03	0.2462	0.00	18	9.150E+01	1.403E+03	0.2464	0.00
	19	9.200E+01	1.401E+03	0.2465	0.00	20	9.600E+01	1.390E+03	0.2468	0.00
	21	1.000E+02	1.378E+03	0.2472	0.00	22	1.100E+02	1.356E+03	0.2479	0.00
	23	1.150E+02	1.350E+03	0.2481	0.00	24	1.180E+02	1.348E+03	0.2482	0.00
	25	1.220E+02	1.347E+03	0.2482	0.00					

Initial Water Storage = 30.6291 cm

NOTE: There are no temperature data when plants are modelled.

DAILY SUMMARY: I	Day = 1,	, Simulat	ed Time =	24.0000 1	hr		
Node Number	=	2	12	25			
Depth (cm)	= 5	.00000	50.00000	122.000	000		
Water (cm3/cm3)	= 0	.21792	0.26621	0.248	818		
Head (cm)	= 1.095	98E+03 9.1	22806E+02	1.34811E	+03		
Water Flow (cm)	=-5.7890	DOE-02 1.	66282E-02	3.72437E	-04		
Plant Sink (cm)	= 0.0000	DOE+00 0.	00000E+00	0.0000E	+00		
PRESTOR INFIL 30.6291+ 0.0762-	RUNOFF + 0.0000 ·	EVAPO - 0.0340-	TRANS 0.0000-	DRAIN 0.0004 =	NEWSTOR 30.6709	Versus	STORAGE 30.6716
Mass Balance = -6 Evaporation: H	6.5613E-04 Potential	4 cm; Tin = 0.137	me step at 5 cm, Actu	tempts = 0.0	14960 and 0340 cm	successe	es =14960
Transpiration: H RHMEAN = 92.6 %;	Potential TMEAN =	= 0.000 266.8 K;	0 cm, Actu HDRY =	al = 0.( 1.0584E+(	0000 cm 05 cm; DA	AYUBC =	0

_____

DAILY SUMMARY: Day = 2, Simulated Time = 24.0000 hr ____ Node Number=21225Depth (cm)=5.0000050.00000122.00000Water (cm3/cm3)=0.210610.265970.24814 Head (cm) = 1.23786E+03 9.27237E+02 1.34951E+03 Water Flow (cm) =-7.62075E-02 1.54229E-02 3.71259E-04Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 30.6716+ 0.0254+ 0.0000 - 0.0442- 0.0000- 0.0004 = 30.6524 Versus 30.6526 Mass Balance = -1.6975E-04 cm; Time step attempts = 5466 and successes = 5466 Evaporation: Potential = 0.1786 cm, Actual = 0.0442 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 87.7 %; TMEAN = 262.0 K; HDRY = 1.8049E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 3, Simulated Time = 24.0000 hr _____ Node Number Depth (cm) = 2 12 2.5 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.21581 0.26572 0.24809 = 1.13462E+03 9.31719E+02 1.35090E+03 Head (cm) Water Flow (cm) =-3.43221E-02 1.43605E-02 3.70077E-04 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 30.6526+ 0.1016+ 0.0000 - 0.0646- 0.0000- 0.0004 = 30.6893 Versus 30.6892 Mass Balance = 2.0981E-05 cm; Time step attempts = 2176 and successes = 2176 Evaporation: Potential = 0.2608 cm, Actual = 0.0646 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 89.7 %; TMEAN = 268.2 K; HDRY = 1.4889E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 4, Simulated Time = 24.0000 hr _____ Node Number = 2 12 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.25822 0.26547 0.24805 = 5.99578E+02 9.36441E+02 1.35225E+03 Head (cm) Water Flow (cm) = 3.81934E-01 1.34043E-02 3.68923E-04Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 30.6892+ 0.7874+ 0.0000 - 0.0911- 0.0000- 0.0004 = 31.3852 Versus 31.3851 Mass Balance = 1.1635E-04 cm; Time step attempts = 2261 and successes = 2261 Evaporation: Potential = 0.3681 cm, Actual = 0.0911 cmTranspiration: Potential = 0.0000 cm, Actual = 0.0000 cmRHMEAN = 79.4 %; TMEAN = 272.6 K; HDRY = 3.1556E+05 cm; DAYUBC = 0

DAILY SUMMARY: Day = 5, Simulated Time = 24.0000 hr Node Number = Depth (cm) = 2 12 25 5.00000 50.00000 122.00000 Depend (cm)=5.0000050.00000Water (cm3/cm3)=0.292270.265220.24801 Head (cm) = 3.75956E+02 9.41072E+02 1.35351E+03 Water Flow (cm) =  $6.96660E-01 \ 1.25274E-02 \ 3.67843E-04$ Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 31.3851+ 1.0668+ 0.0000 - 0.1326- 0.0000- 0.0004 = 32.3188 Versus 32.3188 Mass Balance = 7.6294E-05 cm; Time step attempts = 3307 and successes = 3307 Evaporation: Potential = 0.5414 cm, Actual = 0.1326 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 62.8 %; TMEAN = 277.0 K; HDRY = 6.3781E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 6, Simulated Time = 24.0000 hr _____ 2 = 12 Node Number 25 Node Number=21223Depth (cm)=5.0000050.00000122.00000Water (cm3/cm3)=0.296950.265310.24797= 3.52613E+02 9.39477E+02 1.35477E+03 Head (cm) Water Flow (cm) = 3.22762E-01 1.19654E-02 3.66771E-04Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 32.3188+ 0.5080+ 0.0000 - 0.1534- 0.0000- 0.0004 = 32.6730 Versus 32.6729 Mass Balance = 1.0300E-04 cm; Time step attempts = 2096 and successes = 2096 Evaporation: Potential = 0.6197 cm, Actual = 0.1534 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cmRHMEAN = 64.8 %; TMEAN = 278.2 K; HDRY = 5.9459E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 7, Simulated Time = 24.0000 hr _____ Node Number = 2 12 25 Depth (cm) = Depth (cm) = 5.00000 50.00000 12 Water (cm3/cm3) = 0.28493 0.26655 122.00000 0.24792 Head (cm)  $= 4.15527E+02 \ 9.16598E+02 \ 1.35603E+03$ Water Flow (cm) =-2.39829E-02 1.29068E-02 3.65703E-04 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 32.6729+ 0.0000+ 0.0000 - 0.1085- 0.0000- 0.0004 = 32.5640 Versus 32.5640 Mass Balance = 0.0000E+00 cm; Time step attempts = 96 and successes = 96

Evaporation: Potential = 0.4341 cm, Actual = 0.1085 cmTranspiration: Potential = 0.0000 cm, Actual = 0.0000 cmRHMEAN = 77.6 %; TMEAN = 276.5 K; HDRY = 3.4736E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 8, Simulated Time = 24.0000 hr _____ Node Number Depth (cm) Node Number=21225Depth (cm)=5.0000050.00000122.00000Water (cm3/cm3)=0.273150.268870.24789 = 2 12 25 Head (cm) = 4.87893E+02 8.75740E+02 1.35720E+03Water Flow (cm) =-6.19010E-02 1.62747E-02 3.64696E-04 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 32.5640+ 0.0000+ 0.0000 - 0.1473- 0.0000- 0.0004 = 32.4163 Versus 32.4163 Mass Balance = -7.6294E-06 cm; Time step attempts = 96 and successes = 96 Evaporation: Potential = 0.5893 cm, Actual = 0.1473 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 69.0 %; TMEAN = 275.9 K; HDRY = 5.0887E+05 cm; DAYUBC = Ω DAILY SUMMARY: Day = 9, Simulated Time = 24.0000 hr ______ Node Number = Depth (cm) = 2 12 25 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.26396 0.27132 0.24785= 5.53547E+02 8.35180E+02 1.35829E+03Head (cm) Water Flow (cm) =-7.29764E-02 2.07280E-02 3.63762E-04 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 32.4163+ 0.0000+ 0.0000 - 0.1392- 0.0000- 0.0004 = 32.2767 Versus 32.2767 Mass Balance = 0.0000E+00 cm; Time step attempts = 96 and successes = 96 Evaporation: Potential = 0.5570 cm, Actual = 0.1392 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 77.5 %; TMEAN = 274.0 K; HDRY = 3.4943E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 10, Simulated Time = 24.0000 hr _____ Node Number = 2 12 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.271600.27328 0.24782 Head (cm) = 4.98342E+02 8.04507E+02 1.35922E+03Water Flow (cm) =  $7.36384E-02 \ 2.44425E-02 \ 3.62949E-04$ Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00

PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 32.2767+ 0.2032+ 0.0000 - 0.0707- 0.0000- 0.0004 = 32.4089 Versus 32.4087 Mass Balance = 2.0981E-04 cm; Time step attempts = 2096 and successes = 2096 Evaporation: Potential = 0.2856 cm, Actual = 0.0707 cmTranspiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 78.1 %; TMEAN = 275.7 K; HDRY = 3.3921E+05 cm; DAYUBC = Ο DAILY SUMMARY: Day = 11, Simulated Time = 24.0000 hr _____ Node Number = 2 12 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.28921 0.27469 0.24779 Head (cm) = 3.92045E+027.83243E+021.36006E+03Water Flow (cm) =  $2.80545E-01 \ 2.66420E-02 \ 3.62220E-04$ Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 32.4087+ 0.4064+ 0.0000 - 0.0000- 0.0000- 0.0004 = 32.8147 Versus 32.8146 Mass Balance = 1.2970E-04 cm; Time step attempts = 2096 and successes = 2096 Evaporation: Potential = 0.0000 cm, Actual = 0.0000 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 94.9 %; TMEAN = 275.7 K; HDRY = 7.2186E+04 cm; DAYUBC = 0 DAILY SUMMARY: Day = 12, Simulated Time = 24.0000 hr ______ Node Number Depth (cm) = 2 12 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.28558 0.27599 0.24777 Head (cm) = 4.11916E+027.64223E+021.36078E+03Water Flow (cm) = 5.11988E-02 2.79439E-02 3.61581E-04Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 32.8146+ 0.0254+ 0.0000 - 0.0000- 0.0000- 0.0004 = 32.8396 Versus 32.8395 Mass Balance = 9.1553E-05 cm; Time step attempts = 2096 and successes = 2096 Evaporation: Potential = 0.0000 cm, Actual = 0.0000 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = **** %; TMEAN = 274.8 K; HDRY = -6.1934E+04 cm; DAYUBC = 0 DAILY SUMMARY: Day = 13, Simulated Time = 24.0000 hr _____ Node Number = 2 12 Depth (cm) = 5.00000 50.00000 122.00000

Water (cm3/cm3) = 0.28440 0.277440.24775 Head (cm) = 4.18568E+027.43712E+021.36138E+03Water Flow (cm)  $= 5.54572E-02 \ 2.95232E-02 \ 3.61040E-04$ Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 32.8395+ 0.0508+ 0.0000 - 0.0037- 0.0000- 0.0004 = 32.8863 Versus 32.8862 Mass Balance = 9.1553E-05 cm; Time step attempts = 2096 and successes = 2096 Evaporation: Potential = 0.0149 cm, Actual = 0.0037 cmTranspiration: Potential = 0.0000 cm, Actual = 0.0000 cmRHMEAN = 89.9 %; TMEAN = 279.0 K; HDRY = 1.4591E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 14, Simulated Time = 24.0000 hr _____ Node Number = 2 12 25 Depth (cm) = Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.26896 0.27889 0.24773 Head (cm) = 5.16719E+02 7.23977E+02 1.36187E+03Water Flow (cm) =-7.30080E-02 3.13621E-02 3.60580E-04 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 32.8862+ 0.0000+ 0.0000 - 0.1895- 0.0000- 0.0004 = 32.6963 Versus 32.6963 Mass Balance = -3.8147E-06 cm; Time step attempts = 96 and successes = 96 Evaporation: Potential = 0.7582 cm, Actual = 0.1895 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cmRHMEAN = 75.1 %; TMEAN = 277.0 K; HDRY = 3.9301E+05 cm; DAYUBC = DAILY SUMMARY: Day = 15, Simulated Time = 24.0000 hr _____ Node Number = 2 12 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.26273 0.28011 0.24772 = 5.63092E+02 7.07720E+02 1.36221E+03 Head (cm) Water Flow (cm) =-7.38029E-02 3.28909E-02 3.60237E-04 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 32.6963+0.0000+0.0000-0.1157-0.0000-0.0004 = 32.5802 Versus 32.5802Mass Balance = 0.0000E+00 cm; Time step attempts = 96 and successes = 96 Evaporation: Potential = 0.4628 cm, Actual = 0.1157 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cmRHMEAN = 69.3 %; TMEAN = 275.9 K; HDRY = 5.0351E+05 cm; DAYUBC = 0 

DAILY SUMMARY: Day = 16, Simulated Time = 24.0000 hr

Node Number = 2 12 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.26489 0.28090 0.24772 Head (cm) = 5.46490E+02 6.97465E+02 1.36240E+03Water Flow (cm) =-2.15382E-02 3.34125E-02 3.60021E-04 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 32.5802+ 0.0000+ 0.0000 - 0.0000- 0.0000- 0.0004 = 32.5799 Versus 32.5799 Mass Balance = 0.0000E+00 cm; Time step attempts = 96 and successes = 96 Evaporation: Potential = 0.0000 cm, Actual = 0.0000 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 89.1 %; TMEAN = 273.2 K; HDRY = 1.5860E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 17, Simulated Time = 24.0000 hr _____ Node Number = 2 12 25 Depth (cm) Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.26947 0.28127 0.24772 Head (cm)  $= 5.13099E+02 \ 6.92756E+02 \ 1.36243E+03$ Water Flow (cm) = 5.71209E-02 3.26715E-02 3.59938E-04Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 EVAPO TRANS PRESTOR INFIL RUNOFF DRAIN NEWSTOR STORAGE 32.5799 + 0.1016 + 0.0000 - 0.0121 - 0.0000 - 0.0004 = 32.6690 Versus 32.6690Mass Balance = 1.1444E-05 cm; Time step attempts = 2096 and successes = 2096 Evaporation: Potential = 0.0489 cm, Actual = 0.0121 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 88.7 %; TMEAN = 273.2 K; HDRY = 1.6392E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 18, Simulated Time = 24.0000 hr _____ Node Number = 2 12 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.25894 0.28141 0.24772 Head (cm) = 5.93604E+02 6.90968E+02 1.36230E+03Water Flow (cm) =-3.99672E-02 3.11922E-02 3.59975E-04 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 32.6690+ 0.0254+ 0.0000 - 0.1482- 0.0000- 0.0004 = 32.5459 Versus 32.5458 Mass Balance = 8.7738E-05 cm; Time step attempts = 2096 and successes = 2096 Evaporation: Potential = 0.5987 cm, Actual = 0.1482 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 66.6 %; TMEAN = 275.9 K; HDRY = 5.5728E+05 cm; DAYUBC = 0
DAILY SUMMARY: Day = 19, Simulated Time = 24.0000 hr _____ Node Number 2 12 25 Depth (cm) = 5.00000 50.00000 122.00000 = 0.24571 0.28148 0.24773 Water (cm3/cm3) = 7.16300E+02 6.90091E+02 1.36198E+03 Head (cm) Water Flow (cm) =-9.62323E-02 2.96652E-02 3.60157E-04 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 32.5458+ 0.0000+ 0.0000 - 0.2042- 0.0000- 0.0004 = 32.3412 Versus 32.3412 Mass Balance = 0.0000E+00 cm; Time step attempts = 260 and successes = 260 Evaporation: Potential = 0.8170 cm, Actual = 0.2042 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cmRHMEAN = 66.9 %; TMEAN = 270.4 K; HDRY = 5.5057E+05 cm; DAYUBC = DAILY SUMMARY: Day = 20, Simulated Time = 24.0000 hr = 2 Node Number 12 25 Depth (cm) Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.24054 0.28145 0.24775 = 7.72416E+02 6.90495E+02 1.36147E+03 Head (cm) Water Flow (cm) =-9.29740E-02 2.82134E-02 3.60493E-04 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 32.3412+ 0.0000+ 0.0000 - 0.1231- 0.0000- 0.0004 = 32.2177 Versus 32.2177 Mass Balance = -3.8147E-06 cm; Time step attempts = 144 and successes = 144Evaporation: Potential = 0.4926 cm, Actual = 0.1231 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cmRHMEAN = 64.3 %; TMEAN = 268.7 K; HDRY = 6.0445E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 21, Simulated Time = 24.0000 hr _____ Node Number 2 12 25 Depth (cm) = 5.00000 50.00000 122.00000 = 0.23445 0.28125 0.24777Water (cm3/cm3) Head (cm) = 8.45918E+02 6.93000E+02 1.36075E+03Water Flow (cm) =-8.19779E-02 2.65956E-02 3.60993E-04Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 32.2177+ 0.0000+ 0.0000 - 0.1310- 0.0000- 0.0004 = 32.0863 Versus 32.0863 Mass Balance = 0.0000E+00 cm; Time step attempts = 176 and successes = 176 Evaporation: Potential = 0.5238 cm, Actual = 0.1310 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm

RHMEAN = 61.7 %; TMEAN = 275.7 K; HDRY = 6.6173E+05 cm; DAYUBC =

DAILY SUMMARY: Day = 22, Simulated Time = 24.0000 hr _____ Node Number =: 2 12 25 Depth (cm) Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.22591 0.28089 0.24780 Head (cm)  $= 9.64513E+02 \ 6.97622E+02 \ 1.35982E+03$ Water Flow (cm) =-8.75746E-02 2.47189E-02 3.61666E-04 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 32.0863+ 0.0000+ 0.0000 - 0.1736- 0.0000- 0.0004 = 31.9123 Versus 31.9123 Mass Balance = 0.0000E+00 cm; Time step attempts = 430 and successes = 430 Evaporation: Potential = 0.6946 cm, Actual = 0.1736 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 65.2 %; TMEAN = 276.5 K; HDRY = 5.8689E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 23, Simulated Time = 24.0000 hr ______ ____ Node Number 2 12 25 Note Handel21220Depth (cm)=5.0000050.00000122.00000Water (cm3/cm3)=0.222460.280400.24784= 1.01848E+03 7.03905E+02 1.35864E+03Head (cm) Water Flow (cm) =-8.52225E-02 2.26953E-02 3.62528E-04 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 31.9123+ 0.0000+ 0.0000 - 0.0905- 0.0000- 0.0004 = 31.8215 Versus 31.8215 Mass Balance = -1.9073E-06 cm; Time step attempts = 160 and successes = 160Evaporation: Potential = 0.3621 cm, Actual = 0.0905 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 73.3 %; TMEAN = 279.8 K; HDRY = 4.2514E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 24, Simulated Time = 24.0000 hr _____ Node Number Depth (cm) = 2 12 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.22045 0.27984 0.24786 = 1.05186E+03 7.11278E+02 1.35788E+03Head (cm) Water Flow (cm) =-7.35878E-02 2.06741E-02 3.63455E-04 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 31.8215+ 0.0000+ 0.0000 - 0.3505- 0.0000- 0.0004 = 31.4706 Versus 31.4697

Mass Balance = 9.0981E-04 cm; Time step attempts =17108 and successes =17108 Evaporation: Potential = 1.4021 cm, Actual = 0.3505 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 63.6 %; TMEAN = 278.2 K; HDRY = 6.2118E+05 cm; DAYUBC =  $\cap$ DAILY SUMMARY: Day = 25, Simulated Time = 24.0000 hr _____ Node Number = 2 12 25 Depth (cm) = 5.00000 50.0000 122.00000Water (cm3/cm3) = 0.22488 0.27927 0.24790Head (cm) = 9.80169E+02 7.18811E+02 1.35686E+03Water Flow (cm) =-4.07441E-02 1.88426E-02 3.63913E-04 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 31.4697+ 0.0000+ 0.0000 - 0.0595- 0.0000- 0.0004 = 31.4098 Versus 31.4109 Mass Balance = -1.0815E-03 cm; Time step attempts =34318 and successes =34318 Evaporation: Potential = 1.2314 cm, Actual = 0.0595 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cmRHMEAN = 60.3 %; TMEAN = 274.8 K; HDRY = 6.9394E+05 cm; DAYUBC = 3446 DAILY SUMMARY: Day = 26, Simulated Time = 24.0000 hr _____ Node Number = 2 12 25 Depth (cm) = Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.22664 0.27860 0.24796 Head (cm) = 9.53513E+02 7.27885E+02 1.35502E+03Water Flow (cm) =-2.30898E-02 1.70160E-02 3.65225E-04 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 31.4109+ 0.0000+ 0.0000 - 0.0127- 0.0000- 0.0004 = 31.3977 Versus 31.3976 Mass Balance = 1.4305E-04 cm; Time step attempts = 1835 and successes = 1835 Evaporation: Potential = 1.5158 cm, Actual = 0.0127 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cmRHMEAN = 68.6 %; TMEAN = 279.0 K; HDRY = 5.1683E+05 cm; DAYUBC = 1809 DAILY SUMMARY: Day = 27, Simulated Time = 24.0000 hr _____ 2 Node Number = 12 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.25798 0.27792 0.24803 Head (cm)  $= 6.01581E+02 \ 7.37126E+02 \ 1.35291E+03$ Water Flow (cm) = 3.23733E-01 1.53136E-02 3.66838E-04Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00

PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 31.3976+ 1.0142+ 0.0018 - 0.1279- 0.0000- 0.0004 = 32.2835 Versus 32.2837 Mass Balance = -1.9836E-04 cm; Time step attempts = 5133 and successes = 5133 Evaporation: Potential = 0.5222 cm, Actual = 0.1279 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 76.1 %; TMEAN = 272.9 K; HDRY = 3.7449E+05 cm; DAYUBC = 69 DAILY SUMMARY: Day = 28, Simulated Time = 24.0000 hr _____ Node Number = 2 12 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.35335 0.28035 0.24811 2 Head (cm) = 1.45671E+02 7.04654E+02 1.35042E+03Water Flow (cm)  $= 2.63291E+00 \ 1.53595E-02 \ 3.68772E-04$ Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 INFIL RUNOFF PRESTOR EVAPO TRANS DRAIN NEWSTOR STORAGE 32.2837+ 3.3019+ 1.9559 - 0.0000- 0.0000- 0.0004 = 35.5852 Versus 35.5852 Mass Balance = 1.5259E-05 cm; Time step attempts = 2937 and successes = 2937 Evaporation: Potential = 0.0000 cm, Actual = 0.0000 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 92.8 %; TMEAN = 277.3 K; HDRY = 1.0183E+05 cm; DAYUBC = 616 DAILY SUMMARY: Day = 29, Simulated Time = 24.0000 hr _____ Node Number = 2 12 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.35668 0.31339 0.24820 Head (cm)  $= 1.36130E+02 \ 3.92387E+02 \ 1.34761E+03$ Water Flow (cm)  $= 9.89212E-01 \ 8.54832E-02 \ 3.70994E-04$ Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 35.5852+ 1.0420+ 0.4820 - 0.0300- 0.0000- 0.0004 = 36.5968 Versus 36.5967 Mass Balance = 1.1826E-04 cm; Time step attempts = 2175 and successes = 2175 Evaporation: Potential = 0.1226 cm, Actual = 0.0300 cmTranspiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 84.6 %; TMEAN = 277.9 K; HDRY = 2.2939E+05 cm; DAYUBC = 917 DAILY SUMMARY: Day = 30, Simulated Time = 24.0000 hr ______ Node Number = 2 12 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.32898 0.32691 0.24831

= 2.22110E+02 3.09099E+02 1.34434E+03Head (cm) Water Flow (cm) =-1.85008E-01 2.50203E-01 3.73616E-04 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 36.5967+ 0.0000+ 0.0000 - 0.3811- 0.0000- 0.0004 = 36.2152 Versus 36.2152 Mass Balance = -3.8147E-06 cm; Time step attempts = 96 and successes = 96 Evaporation: Potential = 1.5245 cm, Actual = 0.3811 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 58.8 %; TMEAN = 280.7 K; HDRY = 7.2878E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 31, Simulated Time = 24.0000 hr Node Number = 2 12 25 Depth (cm) = 5.00000 50.00000 122.00000  $\text{Water (cm3/cm3)} = 0.31028 \quad 0.32580 \quad 0.24843$ = 2.92815E+02 3.15302E+02 1.34070E+03 Head (cm) Water Flow (cm) =-2.10424E-01 2.26131E-01 3.76583E-04 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 36.2152+ 0.0000+ 0.0000 - 0.3426- 0.0000- 0.0004 = 35.8722 Versus 35.8722 Mass Balance = -7.6294E-06 cm; Time step attempts = 96 and successes = 96 Evaporation: Potential = 1.3704 cm, Actual = 0.3426 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 52.9 %; TMEAN = 276.8 K; HDRY = 8.7361E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 32, Simulated Time = 24.0000 hr _______ Node Number=21225Depth (cm)=5.0000050.00000122.00000Water (cm3/cm3)=0.294330.323000.24856 = 3.65506E+02 3.31402E+02 1.33669E+03 Head (cm) Water Flow (cm) =-2.11795E-01 1.62776E-01 3.79901E-04 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 35.8722+ 0.0000+ 0.0000 - 0.3257- 0.0000- 0.0004 = 35.5461 Versus 35.5461 Mass Balance = -1.9073E-05 cm; Time step attempts = 96 and successes = 96 Evaporation: Potential = 1.3029 cm, Actual = 0.3257 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cmRHMEAN = 59.6 %; TMEAN = 276.8 K; HDRY = 7.0964E+05 cm; DAYUBC = 0 

DAILY SUMMARY: Day = 33, Simulated Time = 24.0000 hr

_____ Node Number = 2 12 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.28300 0.31990 0.24868 Head (cm)  $= 4.26635E+02 \ 3.50086E+02 \ 1.33316E+03$ Water Flow (cm) =-2.22377E-01 1.20357E-01 3.83331E-04 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 35.5461+ 0.0000+ 0.0000 - 0.7842- 0.0000- 0.0004 = 34.7616 Versus 34.7609 Mass Balance = 6.4850E-04 cm; Time step attempts = 30887 and successes = 30887 Evaporation: Potential = 3.3361 cm, Actual = 0.7842 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 58.2 %; TMEAN = 272.3 K; HDRY = 7.4279E+05 cm; DAYUBC = 4924 DAILY SUMMARY: Day = 34, Simulated Time = 24.0000 hr _____ Node Number Depth (cm) = 2 12 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.28580 0.31682 0.24884 Head (cm)  $= 4.10657E+02 \ 3.69557E+02 \ 1.32843E+03$ Water Flow (cm) =-5.02221E-02 9.08838E-02 3.86873E-04 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 34.7609+0.0000+0.0000-0.0369-0.0000-0.0004 = 34.7236 Versus 34.7236Mass Balance = 3.8147E-06 cm; Time step attempts = 703 and successes = 703 Evaporation: Potential = 4.6481 cm, Actual = 0.0369 cmTranspiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 61.0 %; TMEAN = 279.5 K; HDRY = 6.7727E+05 cm; DAYUBC = 676 DAILY SUMMARY: Day = 35, Simulated Time = 24.0000 hr _____ Node Number = 2 12 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.28536 0.31421 0.24901 Head (cm) = 4.13155E+02 3.86848E+02 1.32342E+03Water Flow (cm) =-3.38977E-02 7.09950E-02 3.91176E-04 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 34.7236+ 0.0000+ 0.0000 - 0.0367- 0.0000- 0.0004 = 34.6865 Versus 34.6863 Mass Balance = 1.4496E-04 cm; Time step attempts = 5094 and successes = 5094 Evaporation: Potential = 6.2724 cm, Actual = 0.0367 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 57.2 %; TMEAN = 280.7 K; HDRY = 7.6572E+05 cm; DAYUBC = 4441 

DAILY SUMMARY: Day = 36, Simulated Time = 24.0000 hr _____ Node Number 2 12 25 Depth (cm) = 5.00000 50.00000 122.00000 = 0.28411 0.31214 0.24920 Water (cm3/cm3) Head (cm) = 4.20212E+02 4.01048E+02 1.31771E+03 Water Flow (cm) =-2.95736E-02 5.89322E-02 3.96050E-04 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 34.6863+ 0.0000+ 0.0000 - 0.0353- 0.0000- 0.0004 = 34.6507 Versus 34.6509 Mass Balance = -2.1362E-04 cm; Time step attempts = 5094 and successes = 5094 Evaporation: Potential = 5.1168 cm, Actual = 0.0353 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cmRHMEAN = 57.1 %; TMEAN = 279.5 K; HDRY = 7.6758E+05 cm; DAYUBC = 5082 DAILY SUMMARY: Day = 37, Simulated Time = 24.0000 hr _____ Node Number = 2 12 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.28247 0.31045 0.24944 = 4.29722E+02 4.13005E+02 1.31078E+03 Head (cm) Water Flow (cm) =-2.85335E-02 5.12539E-02 4.01964E-04 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 34.6509+ 0.0000+ 0.0000 - 0.0353- 0.0000- 0.0004 = 34.6152 Versus 34.6153 Mass Balance = -8.0109E-05 cm; Time step attempts = 1051 and successes = 1051 Evaporation: Potential = 4.4585 cm, Actual = 0.0353 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cmRHMEAN = 59.9 %; TMEAN = 282.6 K; HDRY = 7.0145E+05 cm; DAYUBC = 1036 DAILY SUMMARY: Day = 38, Simulated Time = 24.0000 hr _____ Node Number = 2 12 25 Depth (cm) = 5.00000 50.00000 122.00000 = 0.27776 0.30902 0.24973Water (cm3/cm3) Head (cm) =  $4.58137E+02 \ 4.23434E+02 \ 1.30237E+03$ Water Flow (cm) =-4.88195E-02 4.58019E-02 4.09193E-04 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 34.6153+ 0.0000+ 0.0000 - 0.0525- 0.0000- 0.0004 = 34.5624 Versus 34.5624 Mass Balance = -5.3406E-05 cm; Time step attempts = 1952 and successes = 1952 Evaporation: Potential = 2.7264 cm, Actual = 0.0525 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm

RHMEAN = 89.7 %; TMEAN = 284.0 K; HDRY = 1.4970E+05 cm; DAYUBC = 1908

DAILY SUMMARY: Day = 39, Simulated Time = 24.0000 hr Node Number = 2 12 25 Depth (cm) Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.27609 0.30781 0.25007 Head (cm) = 4.68670E+02 4.32470E+02 1.29249E+03Water Flow (cm) =-4.33497E-02 4.16955E-02 4.17774E-04 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 34.5624+ 0.0000+ 0.0000 - 0.0605- 0.0000- 0.0004 = 34.5016 Versus 34.5041 Mass Balance = -2.5253E-03 cm; Time step attempts =17589 and successes =17589 Evaporation: Potential = 4.0214 cm, Actual = 0.0605 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 80.4 %; TMEAN = 292.6 K; HDRY = 2.9946E+05 cm; DAYUBC = 4843 DAILY SUMMARY: Day = 40, Simulated Time = 24.0000 hr _________ 2 Node Number 12 = 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.27588 0.30668 0.25049 Head (cm) = 4.70039E+02 4.41043E+02 1.28067E+03Water Flow (cm) =-2.67972E-02 3.81364E-02 4.28416E-04 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 34.5041+ 0.0000+ 0.0000 - 0.0387- 0.0000- 0.0004 = 34.4650 Versus 34.4692 Mass Balance = -4.2381E-03 cm; Time step attempts =25086 and successes =25086 Evaporation: Potential = 2.7644 cm, Actual = 0.0387 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 46.8 %; TMEAN = 281.5 K; HDRY = 1.0421E+06 cm; DAYUBC = 4854 DAILY SUMMARY: Day = 41, Simulated Time = 24.0000 hr ______ Node Number Depth (cm) 2 = 12 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.27414 0.30553 0.25106 Head (cm) = 4.81348E+02 4.49977E+02 1.26460E+03Water Flow (cm) =-2.71802E-02 3.46814E-02 4.43490E-04 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 34.4692+ 0.0000+ 0.0000 - 0.0300- 0.0000- 0.0004 = 34.4387 Versus 34.4391

Mass Balance = -3.9673E-04 cm; Time step attempts = 3504 and successes = 3504Evaporation: Potential = 2.5036 cm, Actual = 0.0300 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 67.7 %; TMEAN = 271.5 K; HDRY = 5.3416E+05 cm; DAYUBC = 3479 DAILY SUMMARY: Day = 42, Simulated Time = 24.0000 hr ______ Node Number Depth (cm) 2 = 12 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.27247 0.30448 0.25177 = 4.92428E+02 4.58347E+02 1.24518E+03 Head (cm) Water Flow (cm) =-2.86412E-02 3.17952E-02 4.63038E-04 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 34.4391+ 0.0000+ 0.0000 - 0.0355- 0.0000- 0.0005 = 34.4032 Versus 34.4033 Mass Balance = -8.3923E-05 cm; Time step attempts = 1162 and successes = 1162Evaporation: Potential = 3.5437 cm, Actual = 0.0355 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 69.7 %; TMEAN = 275.4 K; HDRY = 4.9511E+05 cm; DAYUBC = 1139 DAILY SUMMARY: Day = 43, Simulated Time = 24.0000 hr Node Number = Depth (cm) = 2 12 25 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.24607 0.30350 0.25259= 7.12569E+02 4.66193E+02 1.22318E+03 Head (cm) Water Flow (cm) =-1.47007E-01 2.93471E-02 4.87112E-04 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 34.4033+ 0.0254+ 0.0000 - 0.0000- 0.0000- 0.0005 = 34.4282 Versus 34.4280 Mass Balance = 2.4796E-04 cm; Time step attempts =25369 and successes =25369 Evaporation: Potential = 0.0000 cm, Actual = 0.0000 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 95.7 %; TMEAN = 277.9 K; HDRY = 6.0702E+04 cm; DAYUBC = 0 DAILY SUMMARY: Day = 44, Simulated Time = 24.0000 hr ______ 2 12 Node Number = 25 Depth (cm) Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.24264 0.30253 0.25352 Head (cm) = 7.49055E+02 4.74204E+02 1.19871E+03Water Flow (cm) =-1.67009E-01 2.70626E-02 5.16549E-04 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00

PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 34.4280+ 0.0000+ 0.0000 - 0.4126- 0.0000- 0.0005 = 34.0148 Versus 34.0149 Mass Balance = -1.1063E-04 cm; Time step attempts =11446 and successes =11446 Evaporation: Potential = 2.6223 cm, Actual = 0.4126 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 98.2 %; TMEAN = 282.3 K; HDRY = 2.4333E+04 cm; DAYUBC = 3046 DAILY SUMMARY: Day = 45, Simulated Time = 24.0000 hr ______ 2 Node Number -----12 25 Note Namber21223Depth (cm)=5.0000050.00000122.00000Water (cm3/cm3)=0.230020.301360.25459Head (cm) = 9.04880E+02 4.84004E+02 1.17170E+03Water Flow (cm) =-1.29843E-01 2.42410E-02 5.51668E-04 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 34.0149+ 0.0000+ 0.0000 - 0.0000- 0.0000- 0.0006 = 34.0143 Versus 34.0143 Mass Balance = 3.0518E-05 cm; Time step attempts = 96 and successes = 96 Evaporation: Potential = 2.8630 cm, Actual = 0.0000 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = **** %; TMEAN = 282.0 K; HDRY = -1.2593E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 46, Simulated Time = 24.0000 hr -----Node Number Depth (cm) ...... 2 12 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.23250 0.30005 0.25573 Head (cm) = 8.71263E+02 4.95161E+02 1.14342E+03Water Flow (cm) =-1.13949E-01 2.08303E-02 5.92949E-04 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 34.0143+ 0.0000+ 0.0000 - 0.0000- 0.0000- 0.0006 = 34.0137 Versus 34.0137 Mass Balance = 0.0000E+00 cm; Time step attempts = 364 and successes = 364 Evaporation: Potential = 0.0000 cm, Actual = 0.0000 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 94.0 %; TMEAN = 272.0 K; HDRY = 8.5107E+04 cm; DAYUBC = 0 DAILY SUMMARY: Day = 47, Simulated Time = 24.0000 hr _____ Node Number=21225Depth (cm)=5.0000050.00000122.00000Water (cm3/cm3)=0.248060.298790.25687 Head (cm) = 6.92423E+02 5.06278E+02 1.11621E+03

Water Flow (cm)  $= 6.96618E-02 \ 1.75411E-02 \ 6.38837E-04$ Plant Sink (cm) =  $0.00000E+00 \ 0.00000E+00 \ 0.00000E+00$ PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 34.0137+ 0.3810+ 0.0000 - 0.5834- 0.0000- 0.0006 = 33.8107 Versus 33.8128 Mass Balance = -2.0943E-03 cm; Time step attempts =23941 and successes =23941 Evaporation: Potential = 2.6417 cm, Actual = 0.5834 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 84.5 %; TMEAN = 271.8 K; HDRY = 2.3075E+05 cm; DAYUBC = 3202 DAILY SUMMARY: Day = 48, Simulated Time = 24.0000 hr Node Number=21225Depth (cm)=5.0000050.00000122.00000Water (cm3/cm3)=0.243430.297590.25810 Head (cm) = 7.40374E+02 5.17019E+02 1.08785E+03Water Flow (cm) =-7.45915E-02 1.47473E-02 6.87786E-04 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 33.8128+ 0.1270+ 0.0000 - 0.2332- 0.0000- 0.0007 = 33.7059 Versus 33.7092 Mass Balance = -3.2730E-03 cm; Time step attempts =28317 and successes =28317 Evaporation: Potential = 3.0741 cm, Actual = 0.2332 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 61.6 %; TMEAN = 271.2 K; HDRY = 6.6480E+05 cm; DAYUBC = 3215 DAILY SUMMARY: Day = 49, Simulated Time = 24.0000 hr _____ Node Number Depth (cm) ----2 12 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.24418 0.29657 0.25924 Head (cm) = 7.32410E+02 5.26401E+02 1.06238E+03Water Flow (cm) =-3.79638E-02 1.26095E-02 7.40264E-04 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 33.7092+ 0.0508+ 0.0000 - 0.0824- 0.0000- 0.0007 = 33.6768 Versus 33.6836 Mass Balance = -6.8321E-03 cm; Time step attempts =45096 and successes =45096 Evaporation: Potential = 3.3203 cm, Actual = 0.0824 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cmRHMEAN = 71.5 %; TMEAN = 274.8 K; HDRY = 4.5964E+05 cm; DAYUBC = 3949 

DAILY SUMMARY: Day = 50, Simulated Time = 24.0000 hr

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Node Number=21225Depth (cm)=5.0000050.00000122.00000Water (cm3/cm3)=0.245010.295510.26050 Head (cm) = 7.23571E+025.36322E+021.03521E+03Water Flow (cm) =-2.51376E-02 1.10662E-02 8.00146E-04Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 33.6836+ 0.0000+ 0.0000 - 0.0225- 0.0000- 0.0008 = 33.6603 Versus 33.6606 Mass Balance = -3.2806E-04 cm; Time step attempts = 5094 and successes = 5094 Evaporation: Potential = 5.1699 cm, Actual = 0.0225 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 68.1 %; TMEAN = 282.9 K; HDRY = 5.2566E+05 cm; DAYUBC = 4084 DAILY SUMMARY: Day = 51, Simulated Time = 24.0000 hr ______ Node Number = 2 12 25 Depth (cm) Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.24345 0.29451 0.26176 Head (cm) = 7.40195E+025.45858E+021.00902E+03Water Flow (cm) =-2.91749E-029.80799E-038.65267E-04Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 33.6606+ 0.0000+ 0.0000 - 0.0178- 0.0000- 0.0009 = 33.6420 Versus 33.6419 Mass Balance = 8.3923E-05 cm; Time step attempts = 1986 and successes = 1986 Evaporation: Potential = 2.5091 cm, Actual = 0.0178 cmTranspiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 91.4 %; TMEAN = 281.5 K; HDRY = 1.2308E+05 cm; DAYUBC = 1919 DAILY SUMMARY: Day = 52, Simulated Time = 24.0000 hr 2 Node Number == 12 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.24361 0.29378 0.26287 Head (cm) = 7.38478E+02 5.52951E+02 9.86606E+02Water Flow (cm) =-3.29326E-02 8.92421E-03 9.29505E-04 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 33.6419+ 0.0254+ 0.0000 - 0.0743- 0.0000- 0.0009 = 33.5921 Versus 33.6007 Mass Balance = -8.6517E-03 cm; Time step attempts =52225 and successes =52225 Evaporation: Potential = 3.7411 cm, Actual = 0.0743 cmTranspiration: Potential = 0.0000 cm, Actual = 0.0000 cmRHMEAN = 73.1 %; TMEAN = 275.9 K; HDRY = 4.2865E+05 cm; DAYUBC = 4532

DAILY SUMMARY: Day = 53, Simulated Time = 24.0000 hr _____ Node Number = Depth (cm) = 2 12 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.24397 0.29294 0.26402 Head (cm) = 7.34626E+02 5.61282E+02 9.63944E+02Water Flow (cm) =-2.24334E-02 8.04998E-03 9.95736E-04Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 33.6007+ 0.0000+ 0.0000 - 0.0229- 0.0000- 0.0010 = 33.5768 Versus 33.5778 Mass Balance = -9.4986E-04 cm; Time step attempts = 7593 and successes = 7593 Evaporation: Potential = 3.2177 cm, Actual = 0.0229 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 67.3 %; TMEAN = 275.7 K; HDRY = 5.4240E+05 cm; DAYUBC = 4579 DAILY SUMMARY: Day = 54, Simulated Time = 24.0000 hr _____ -----Node Number 2 12 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.33482 0.29533 0.26516 Head (cm) = 2.02457E+02 5.38001E+02 9.42303E+02 Water Flow (cm) = 2.17507E+00 1.00445E-02 1.06739E-03Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 33.5778+ 3.7293+ 1.6555 - 0.5326- 0.0000- 0.0011 = 36.7735 Versus 36.7740 Mass Balance = -5.4550E-04 cm; Time step attempts = 5919 and successes = 5919 Evaporation: Potential = 2.2663 cm, Actual = 0.5326 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cmRHMEAN = 96.0 %; TMEAN = 276.5 K; HDRY = 5.6282E+04 cm; DAYUBC = 805 DAILY SUMMARY: Day = 55, Simulated Time = 24.0000 hr _____ Node Number 2 12 = 25 Node Number = Depth (cm) = 5.00000 50.00000 122.00000 Dependence(cm)=5.0000050.00000122.00000Water (cm3/cm3)=0.285790.306550.26626Head (cm) = 4.10735E+02 4.42065E+02 9.21918E+02Water Flow (cm) =-3.90727E-01 5.28101E-02 1.14150E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 STORAGE PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR 36.7740+ 0.0000+ 0.0000 - 0.7538- 0.0000- 0.0011 = 36.0190 Versus 36.0191 Mass Balance = -1.9073E-05 cm; Time step attempts = 442 and successes = 442 Evaporation: Potential = 3.0153 cm, Actual = 0.7538 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm

RHMEAN = 77.6 %; TMEAN = 285.1 K; HDRY = 3.4778E+05 cm; DAYUBC = 0

DAILY SUMMARY: Day = 56, Simulated Time = 24.0000 hr _____ Node Number Depth (cm) 2 ----12 25 Depth (cm)=21225Water (cm3/cm3)=0.258630.310100.26731 Head (cm) = 5.96125E+02 4.15594E+02 9.03025E+02Water Flow (cm) =-3.12020E-01 8.49101E-02 1.21652E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 36.0191+ 0.0000+ 0.0000 - 0.6519- 0.0000- 0.0012 = 35.3660 Versus 35.3660 Mass Balance = -1.9073E-05 cm; Time step attempts = 2892 and successes = 2892 Evaporation: Potential = 2.6075 cm, Actual = 0.6519 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 60.8 %; TMEAN = 279.8 K; HDRY = 6.8096E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 57, Simulated Time = 24.0000 hr _____ Node Number = 2 12 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.26204 0.30957 0.26826 Head (cm) = 5.68478E+02 4.19441E+02 8.86367E+02Water Flow (cm) =-2.22251E-01 7.44138E-02 1.29011E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 35.3660+ 0.0000+ 0.0000 - 0.4615- 0.0000- 0.0013 = 34.9032 Versus 34.9020 Mass Balance = 1.2856E-03 cm; Time step attempts =17279 and successes =17279 Evaporation: Potential = 2.9374 cm, Actual = 0.4615 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 61.5 %; TMEAN = 283.4 K; HDRY = 6.6580E+05 cm; DAYUBC = 2611 DAILY SUMMARY: Day = 58, Simulated Time = 24.0000 hr _ _ _ _ _ _ _ _ _ _ _ _ _ _ _ _ _ _ _ Node Number Depth (cm) 12 = 2 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.26420 0.30792 0.26920 Head (cm) = 5.51750E+02 4.31662E+02 8.70243E+02Water Flow (cm) =-6.20121E-02 5.65573E-02 1.36304E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 34.9020+ 0.0000+ 0.0000 - 0.0337- 0.0000- 0.0014 = 34.8669 Versus 34.8669

Mass Balance = 6.4850E-05 cm; Time step attempts = 2022 and successes = 2022 Evaporation: Potential = 2.5217 cm, Actual = 0.0337 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cmRHMEAN = 90.6 %; TMEAN = 279.0 K; HDRY = 1.3578E+05 cm; DAYUBC = 1969 DAILY SUMMARY: Day = 59, Simulated Time = 24.0000 hr ______ Node Number 2 12 = 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.26061 0.30620 0.27009 = 5.79905E+02 4.44761E+02 8.55305E+02 Head (cm) Water Flow (cm) =-7.31020E-02 4.27616E-02 1.43717E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 DRAIN NEWSTOR PRESTOR INFIL RUNOFF EVAPO TRANS STORAGE 34.8669+ 0.0000+ 0.0000 - 0.0741- 0.0000- 0.0014 = 34.7914 Versus 34.7914 Mass Balance = 1.5259E-05 cm; Time step attempts = 442 and successes = 442 Evaporation: Potential = 1.8100 cm, Actual = 0.0741 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cmRHMEAN = 95.4 %; TMEAN = 271.5 K; HDRY = 6.4075E+04 cm; DAYUBC = 410 DAILY SUMMARY: Day = 60, Simulated Time = 24.0000 hr -----------Node Number = 2 12 25 Depth (cm) = Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.26397 0.30467 0.27093 Head (cm) = 5.53536E+02 4.56831E+02 8.41502E+02Water Flow (cm) =-4.13321E-02 3.36206E-02 1.50973E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 34.7914+ 0.3810+ 0.0000 - 0.0000- 0.0000- 0.0015 = 35.1708 Versus 35.1704 Mass Balance = 4.5395E-04 cm; Time step attempts = 5093 and successes = 5093 Evaporation: Potential = 0.0000 cm, Actual = 0.0000 cmTranspiration: Potential = 0.0000 cm, Actual = 0.0000 cmRHMEAN = 92.7 %; TMEAN = 275.7 K; HDRY = 1.0327E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 61, Simulated Time = 24.0000 hr _____ 2 Node Number = 12 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.26556 0.30330 0.27178 Head (cm) = 5.41487E+02 4.67849E+02 8.27957E+02Water Flow (cm) =-1.35847E-02 2.74961E-02 1.58484E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00

PRESTOR INFIL RUNOFF DRAIN NEWSTOR EVAPO TRANS STORAGE 35.1704+ 0.0000+ 0.0000 - 0.0021- 0.0000- 0.0016 = 35.1667 Versus 35.1667 Mass Balance = 0.0000E+00 cm; Time step attempts = 96 and successes = 96 Evaporation: Potential = 0.0083 cm, Actual = 0.0021 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 95.3 %; TMEAN = 276.5 K; HDRY = 6.6547E+04 cm; DAYUBC = DAILY SUMMARY: Day = 62, Simulated Time = 24.0000 hr -----Node Number=21225Depth (cm)=5.0000050.00000122.00000Water (cm3/cm3)=0.249970.302120.27262 2 Head (cm)  $= 6.73713E+02 \ 4.77624E+02 \ 8.14715E+02$ Water Flow (cm) =-9.29955E-02 2.32031E-02 1.66241E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 35.1667+ 0.0000+ 0.0000 - 0.2246- 0.0000- 0.0017 = 34.9405 Versus 34.9405 Mass Balance = -7.6294E-06 cm; Time step attempts = 248 and successes = 248 Evaporation: Potential = 0.8982 cm, Actual = 0.2246 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 75.0 %; TMEAN = 279.8 K; HDRY = 3.9421E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 63, Simulated Time = 24.0000 hr _____ Node Number = 2 12 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.24295 0.30108 0.27344 122.00000 Head (cm) = 7.45645E+02 4.86314E+02 8.02005E+02Water Flow (cm) =-1.25224E-01 2.01833E-02 1.74271E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 34.9405+ 0.0000+ 0.0000 - 0.4896- 0.0000- 0.0017 = 34.4491 Versus 34.4487 Mass Balance = 4.1199E-04 cm; Time step attempts =13640 and successes =13640 Evaporation: Potential = 1.9585 cm, Actual = 0.4896 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 57.4 %; TMEAN = 281.5 K; HDRY = 7.6038E+05 cm; DAYUBC = DAILY SUMMARY: Day = 64, Simulated Time = 24.0000 hr ______ Node Number = 2 12 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.24725 0.30014 0.27422

Head (cm) = 7.00548E+02 4.94453E+02 7.90221E+02Water Flow (cm) =-6.51570E-02 1.77403E-02 1.82139E-03Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 34.4487+ 0.0000+ 0.0000 - 0.0958- 0.0000- 0.0018 = 34.3511 Versus 34.3523 Mass Balance = -1.2283E-03 cm; Time step attempts =25870 and successes =25870 Evaporation: Potential = 1.3641 cm, Actual = 0.0958 cmTranspiration: Potential = 0.0000 cm, Actual = 0.0000 cmRHMEAN = 66.2 %; TMEAN = 278.7 K; HDRY = 5.6541E+05 cm; DAYUBC = 4035 DAILY SUMMARY: Day = 65, Simulated Time = 24.0000 hr _____ Node Number = Depth (cm) = 2 12 25 5.00000 50.00000 122.00000 = 0.24920 0.29910 0.27503Water (cm3/cm3) = 6.81156E+02 5.03542E+02 7.78130E+02 Head (cm) Water Flow (cm) =-2.92819E-02 1.52608E-02 1.90339E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 34.3523+ 0.0000+ 0.0000 - 0.0221- 0.0000- 0.0019 = 34.3283 Versus 34.3287 Mass Balance = -3.8147E-04 cm; Time step attempts =12591 and successes =12591 Evaporation: Potential = 1.0253 cm, Actual = 0.0221 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 60.6 %; TMEAN = 279.0 K; HDRY = 6.8577E+05 cm; DAYUBC = 3366 DAILY SUMMARY: Day = 66, Simulated Time = 24.0000 hr _____ 2 Node Number Depth (cm) 12 = 25 Depth (cm) = 5.00000 50.00000 122.00000Water (cm3/cm3) = 0.25002 0.29806 0.27586Head (cm) = 6.73294E+02 5.12766E+02 7.66076E+02Water Flow (cm) =-2.36376E-02 1.28215E-02 1.99304E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 34.3287+ 0.0000+ 0.0000 - 0.0210- 0.0000- 0.0020 = 34.3057 Versus 34.3062 Mass Balance = -4.3488E-04 cm; Time step attempts = 7593 and successes = 7593 Evaporation: Potential = 1.5673 cm, Actual = 0.0210 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 55.9 %; TMEAN = 281.5 K; HDRY = 7.9718E+05 cm; DAYUBC = 3947 

DAILY SUMMARY: Day = 67, Simulated Time = 24.0000 hr

Node Number=21225Depth (cm)=5.0000050.00000122.00000Water (cm3/cm3)=0.250090.297080.27670 Head (cm) = 6.72535E+02 5.21673E+02 7.54167E+02Water Flow (cm) =-2.26214E-02 1.08303E-02 2.08795E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 34.3062+ 0.0000+ 0.0000 - 0.0199- 0.0000- 0.0021 = 34.2842 Versus 34.2842 Mass Balance = -7.6294E-05 cm; Time step attempts = 1566 and successes = 1566 Evaporation: Potential = 0.9421 cm, Actual = 0.0199 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 65.1 %; TMEAN = 279.5 K; HDRY = 5.8767E+05 cm; DAYUBC = 1547 DAILY SUMMARY: Day = 68, Simulated Time = 24.0000 hr ______ Node Number -----2 12 25 Depth (cm) Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.25032 0.29623 0.27747 Head (cm) = 6.70347E+02 5.29524E+02 7.43260E+02Water Flow (cm) =-2.05276E-02 9.33949E-03 2.18036E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 34.2842+ 0.0000+ 0.0000 - 0.0225- 0.0000- 0.0022 = 34.2595 Versus 34.2608 Mass Balance = -1.2589E-03 cm; Time step attempts =12591 and successes =12591 Evaporation: Potential = 1.6632 cm, Actual = 0.0225 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 52.1 %; TMEAN = 280.7 K; HDRY = 8.9332E+05 cm; DAYUBC = 3011 DAILY SUMMARY: Day = 69, Simulated Time = 24.0000 hr ______ Node Number=21225Depth (cm)=5.0000050.00000122.00000Water (cm3/cm3)=0.250280.295440.27825 Head (cm) = 6.70749E+025.36983E+027.32629E+02Water Flow (cm) =-1.91573E-02 8.16493E-03 2.27600E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 INFIL RUNOFF TRANS DRAIN NEWSTOR PRESTOR EVAPO STORAGE 34.2608+ 0.0000+ 0.0000 - 0.0193- 0.0000- 0.0023 = 34.2392 Versus 34.2397 Mass Balance = -4.8828E-04 cm; Time step attempts = 4975 and successes = 4975Evaporation: Potential = 2.5464 cm, Actual = 0.0193 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 52.1 %; TMEAN = 282.9 K; HDRY = 8.9255E+05 cm; DAYUBC = 4963 

DAILY SUMMARY: Day = 70, Simulated Time = 24.0000 hr Node Number = Depth (cm) = 2 12 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.28299 0.29476 0.27898 Head (cm) = 4.26654E+02 5.43437E+02 7.22660E+02Water Flow (cm) = 5.54391E-01 7.28810E-03 2.37121E-03Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 34.2397+ 1.4986+ 0.0000 - 0.3125- 0.0000- 0.0024 = 35.4234 Versus 35.4239 Mass Balance = -5.5313E-04 cm; Time step attempts = 5340 and successes = 5340 Evaporation: Potential = 1.2756 cm, Actual = 0.3125 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 60.8 %; TMEAN = 287.0 K; HDRY = 6.8103E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 71, Simulated Time = 24.0000 hr _____ Node Number 2 = 12 25 Depth (cm) = 5.00000 50.00000 122.00000Water (cm3/cm3) = 0.26856 0.29481 0.27969Head (cm) = 5.19573E+02 5.43018E+02 7.13200E+02 Water Flow (cm) =-4.88067E-02 7.99287E-03 2.46682E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 35.4239+ 0.1524+ 0.0000 - 0.3095- 0.0000- 0.0025 = 35.2643 Versus 35.2644 Mass Balance = -9.9182E-05 cm; Time step attempts = 2192 and successes = 2192 Evaporation: Potential = 1.2506 cm, Actual = 0.3095 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 77.7 %; TMEAN = 282.6 K; HDRY = 3.4503E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 72, Simulated Time = 24.0000 hr Node Number = Depth (cm) = 2 12 25 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.24841 0.29545 0.28036Head (cm) = 6.88992E+02 5.36914E+02 7.04439E+02 Water Flow (cm) =-1.64741E-01 1.16003E-02 2.56003E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 35.2644+ 0.0000+ 0.0000 - 0.5490- 0.0000- 0.0026 = 34.7128 Versus 34.7129 Mass Balance = -9.9182E-05 cm; Time step attempts = 4174 and successes = 4174Evaporation: Potential = 2.1961 cm, Actual = 0.5490 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm

RHMEAN = 48.9 %; TMEAN = 286.2 K; HDRY = 9.8105E+05 cm; DAYUBC =

DAILY SUMMARY: Day = 73, Simulated Time = 24.0000 hr Node Number = 2 12 25 Depth (cm) Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.23906 0.29585 0.28099 Head (cm) = 7.89519E+025.33163E+026.96347E+02Water Flow (cm) =-1.67186E-01 1.46425E-02 2.65033E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 EVAPO TRANS PRESTOR INFIL RUNOFF DRAIN NEWSTOR STORAGE 34.7129+ 0.0000+ 0.0000 - 0.2615- 0.0000- 0.0027 = 34.4488 Versus 34.4487 Mass Balance = 7.6294E-05 cm; Time step attempts = 9173 and successes = 9173Evaporation: Potential = 1.0460 cm, Actual = 0.2615 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 77.5 %; TMEAN = 284.8 K; HDRY = 3.4858E+05 cm; DAYUBC = Ο DAILY SUMMARY: Day = 74, Simulated Time = 24.0000 hr ______ Node Number = 2 12 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.23510 0.29575 0.28156 Head (cm) = 8.37585E+02 5.34034E+02 6.89113E+02Water Flow (cm) =-1.10113E-01 1.50489E-02 2.73514E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 34.4487+ 0.2286+ 0.0000 - 0.1027- 0.0000- 0.0027 = 34.5719 Versus 34.5717 Mass Balance = 2.4033E-04 cm; Time step attempts = 5110 and successes = 5110 Evaporation: Potential = 0.4149 cm, Actual = 0.1027 cmTranspiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 83.2 %; TMEAN = 281.5 K; HDRY = 2.5259E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 75, Simulated Time = 24.0000 hr _____ Node Number Depth (cm) = 2 12 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.22878 0.29530 0.28210 Head (cm) = 9.22418E+02 5.38361E+02 6.82314E+02Water Flow (cm) =-9.94823E-02 1.35130E-02 2.81777E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 34.5717+ 0.0000+ 0.0000 - 0.3805- 0.0000- 0.0028 = 34.1884 Versus 34.1884

Mass Balance = -3.8147E-05 cm; Time step attempts = 5220 and successes = 5220 Evaporation: Potential = 1.5218 cm, Actual = 0.3805 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 61.3 %; TMEAN = 279.0 K; HDRY = 6.6982E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 76, Simulated Time = 24.0000 hr Node Number Depth (cm) = 2 12 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.23297 0.29470 0.28254 = 8.65073E+02 5.44081E+02 6.76766E+02 Head (cm) Water Flow (cm) =-7.32209E-02 1.13568E-02 2.89151E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 34.1884+ 0.0000+ 0.0000 - 0.1492- 0.0000- 0.0029 = 34.0363 Versus 34.0374 Mass Balance = -1.0910E-03 cm; Time step attempts =25979 and successes =25979 Evaporation: Potential = 1.5011 cm, Actual = 0.1492 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 60.3 %; TMEAN = 278.7 K; HDRY = 6.9271E+05 cm; DAYUBC = 3564 DAILY SUMMARY: Day = 77, Simulated Time = 24.0000 hr _____ Node Number = Depth (cm) = 2 12 25 5.00000 50.00000 122.00000 = 0.23613 0.29393 0.28301 Water (cm3/cm3) = 8.24713E+02 5.51476E+02 6.71038E+02 Head (cm) Water Flow (cm) =-3.13723E-02 9.11316E-03 2.96234E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 34.0374+ 0.0000+ 0.0000 - 0.0158- 0.0000- 0.0030 = 34.0186 Versus 34.0187 Mass Balance = -1.0681E-04 cm; Time step attempts = 2365 and successes = 2365 Evaporation: Potential = 0.6794 cm, Actual = 0.0158 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 65.0 %; TMEAN = 280.1 K; HDRY = 5.9016E+05 cm; DAYUBC = 2345 DAILY SUMMARY: Day = 78, Simulated Time = 24.0000 hr _____ Node Number Depth (cm) 2 = 12 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.23754 0.29315 0.28343 = 8.07575E+02 5.59207E+02 6.65837E+02Head (cm) Water Flow (cm) =-2.38644E-02 7.01284E-03 3.03188E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00

PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 34.0187+ 0.0000+ 0.0000 - 0.0179- 0.0000- 0.0030 = 33.9978 Versus 33.9980 Mass Balance = -2.5177E-04 cm; Time step attempts = 5094 and successes = 5094Evaporation: Potential = 1.1055 cm, Actual = 0.0179 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 63.4 %; TMEAN = 281.5 K; HDRY = 6.2473E+05 cm; DAYUBC = 2811 ______ DAILY SUMMARY: Day = 79, Simulated Time = 24.0000 hr _____ Node Number = 2 12 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.23838 0.29239 0.28383 Head (cm) = 7.97485E+02 5.66787E+02 6.61095E+02Water Flow (cm) =-2.08535E-02 5.26412E-03 3.09720E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 33.9980+ 0.0000+ 0.0000 - 0.0181- 0.0000- 0.0031 = 33.9768 Versus 33.9772 Mass Balance = -4.3106E-04 cm; Time step attempts = 7593 and successes = 7593 Evaporation: Potential = 1.6059 cm, Actual = 0.0181 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 58.8 %; TMEAN = 280.7 K; HDRY = 7.2878E+05 cm; DAYUBC = 3959 DAILY SUMMARY: Day = 80, Simulated Time = 24.0000 hr _____ = Node Number 2 12 Node Number = Depth (cm) = 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.23900 0.29167 0.28420 Head (cm) = 7.90266E+02 5.74044E+02 6.56668E+02Water Flow (cm) =-1.87482E-02 3.85641E-03 3.15965E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 33.9772+ 0.0000+ 0.0000 - 0.0179- 0.0000- 0.0032 = 33.9562 Versus 33.9565 Mass Balance = -3.5095E-04 cm; Time step attempts = 7593 and successes = 7593 Evaporation: Potential = 2.1500 cm, Actual = 0.0179 cmTranspiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 49.6 %; TMEAN = 284.3 K; HDRY = 9.6041E+05 cm; DAYUBC = 3463 DAILY SUMMARY: Day = 81, Simulated Time = 24.0000 hr Node Number = 2 12 25 Head (cm) = 7.88593E+02 5.77389E+02 6.55185E+02

Water Flow (cm) _=-1.78333E-02 2.84707E-03 3.20040E-03 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 33.9565+ 0.0000+ 0.0000 - 0.0145- 0.0000- 0.0032 = 33.9388 Versus 33.9487 Mass Balance = -9.8343E-03 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.1451 cm, Actual = 0.0145 cm Transpiration: Potential = 2.7576 cm, Actual = 0.0000 cm RHMEAN = 48.5 %; TMEAN = 290.4 K; HDRY = 9.9280E+05 cm; DAYUBC = 4573 DAILY SUMMARY: Day = 82, Simulated Time = 24.0000 hr _____ Node Number = Depth (cm) = 2 12 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.20925 0.29032 0.28444 Head (cm) = 1.26691E+03 5.88105E+02 6.53811E+02Water Flow (cm) =-5.05469E-02 8.14929E-04 3.22126E-03 Plant Sink (cm) = 3.78819E-02 5.95197E-04 0.00000E+00 EVAPO TRANS PRESTOR INFIL RUNOFF DRAIN NEWSTOR STORAGE 33.9487+ 0.0000+ 0.0000 - 0.0122- 0.4313- 0.0032 = 33.5020 Versus 33.5095 Mass Balance = -7.4425E-03 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.0908 cm, Actual = 0.0122 cm Transpiration: Potential = 1.7250 cm, Actual = 0.4313 cm RHMEAN = 61.3 %; TMEAN = 288.2 K; HDRY = 6.6988E+05 cm; DAYUBC = 3488 DAILY SUMMARY: Day = 83, Simulated Time = 24.0000 hr _____ 2 Node Number Depth (cm) -----12 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.14967 0.28844 0.28473 Head (cm) = 4.76185E+03 6.08162E+02 6.50315E+02Water Flow (cm) =-4.71805E-02-3.10767E-03 3.25490E-03 Plant Sink (cm) = 6.82038E-02 1.15671E-03 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 33.5095+ 0.0000+ 0.0000 - 0.0127- 0.8137- 0.0033 = 32.6798 Versus 32.6813 Mass Balance = -1.4801E-03 cm; Time step attempts =31452 and successes =31452 Evaporation: Potential = 0.1764 cm, Actual = 0.0127 cmTranspiration: Potential = 3.3524 cm, Actual = 0.8137 cm RHMEAN = 48.5 %; TMEAN = 281.8 K; HDRY = 9.9144E+05 cm; DAYUBC = 3849 DAILY SUMMARY: Day = 84, Simulated Time = 24.0000 hr _____ Node Number = 2 12 25

Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.13934 0.28720 0.28494 Head (cm)  $= 6.66801E+03 \ 6.21900E+02 \ 6.47883E+02$ Water Flow (cm) =-2.56512E-02-6.29377E-03 3.29997E-03 Plant Sink (cm) = 1.46710E-02 4.42561E-04 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 32.6813+ 0.0000+ 0.0000 - 0.0051- 0.2598- 0.0033 = 32.4131 Versus 32.4159 Mass Balance = -2.7390E-03 cm; Time step attempts =53643 and successes =53643 Evaporation: Potential = 0.0675 cm, Actual = 0.0051 cm Transpiration: Potential = 1.2826 cm, Actual = 0.2598 cm RHMEAN = 51.4 %; TMEAN = 276.2 K; HDRY = 9.1100E+05 cm; DAYUBC = 3469 DAILY SUMMARY: Day = 85, Simulated Time = 24.0000 hr _____ Node Number = Depth (cm) = 2 12 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.12920 0.28581 0.28513 Head (cm)  $= 9.86990E+03 \ 6.37724E+02 \ 6.45574E+02$ Water Flow (cm) =-1.30617E-02-8.38423E-03 3.33621E-03 Plant Sink (cm) = 1.15885E-02 6.17129E-04 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 32.4159+ 0.0000+ 0.0000 - 0.0086- 0.3072- 0.0033 = 32.0967 Versus 32.1006 Mass Balance = -3.8986E-03 cm; Time step attempts =59865 and successes =59865 Evaporation: Potential = 0.0941 cm, Actual = 0.0086 cm Transpiration: Potential = 1.7886 cm, Actual = 0.3072 cm RHMEAN = 41.1 %; TMEAN = 277.6 K; HDRY = 1.2170E+06 cm; DAYUBC = 5400 DAILY SUMMARY: Day = 86, Simulated Time = 24.0000 hr _____ 2 Node Number = 12 25 Node Number=21225Depth (cm)=5.0000050.00000122.00000Water (cm3/cm3)=0.131120.284640.28541 Head (cm)  $= 9.10990E+03 \ 6.51432E+02 \ 6.42359E+02$ Water Flow (cm) =-7.12838E-03-9.92594E-03 3.37435E-03 Plant Sink (cm) = 3.76314E-03 2.69462E-04 0.00000E+00PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 32.1006+ 0.2286+ 0.0000 - 0.0103- 0.1213- 0.0034 = 32.1942 Versus 32.1951 Mass Balance = -8.5068E-04 cm; Time step attempts = 8109 and successes = 8109 Evaporation: Potential = 0.0415 cm, Actual = 0.0103 cm Transpiration: Potential = 0.7888 cm, Actual = 0.1213 cm RHMEAN = 49.6 %; TMEAN = 280.4 K; HDRY = 9.5991E+05 cm; DAYUBC = 0

DAILY SUMMARY: Day = 87, Simulated Time = 24.0000 hr 25 Node Number == 2 12 Depth (cm) = 5.00000 50.00000 122.00000 = 0.14707 0.28376 Water (cm3/cm3) 0.28567 Head (cm) = 5.15703E+03 6.61870E+02 6.39341E+02 =-4.89844E-03-1.02170E-02 3.42206E-03 Water Flow (cm) Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 32.1951+ 0.1270+ 0.0000 - 0.0000- 0.0000- 0.0034 = 32.3187 Versus 32.3186 Mass Balance = 5.3406E-05 cm; Time step attempts = 3435 and successes = 3435 Evaporation: Potential = 0.0000 cm, Actual = 0.0000 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 90.6 %; TMEAN = 279.0 K; HDRY = 1.3578E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 88, Simulated Time = 24.0000 hr = Node Number 2 12 25 Depth (cm) Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.16022 0.28296 0.28590 Head (cm) = 3.53764E+03 6.71646E+02 6.36651E+02Water Flow (cm) = 6.96324E-05-1.01918E-02 3.46588E-03Plant Sink (cm) = 1.27044E-04 2.65052E-06 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 32.3186+ 0.0508+ 0.0000 - 0.0001- 0.0016- 0.0035 = 32.3642 Versus 32.3648 Mass Balance = -6.0272E-04 cm; Time step attempts = 5532 and successes = 5532 Evaporation: Potential = 0.0004 cm, Actual = 0.0001 cm Transpiration: Potential = 0.0078 cm, Actual = 0.0016 cm RHMEAN = 88.1 %; TMEAN = 280.9 K; HDRY = 1.7350E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 89, Simulated Time = 24.0000 hr Node Number 2 12 25 = Depth (cm) Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.14467 0.28170 0.28611 Head (cm) = 5.56789E+03 6.87257E+02 6.34204E+02Water Flow (cm) =-3.62873E-04-1.10629E-02 3.50682E-03 Plant Sink (cm) = 2.09666E-024.85624E-040.00000E+00PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 32.3648+ 0.0000+ 0.0000 - 0.0185- 0.2860- 0.0035 = 32.0568 Versus 32.0568 Mass Balance = -7.6294E-06 cm; Time step attempts = 852 and successes = 852 Evaporation: Potential = 0.0741 cm, Actual = 0.0185 cm Transpiration: Potential = 1.4074 cm, Actual = 0.2860 cm RHMEAN = 71.8 %; TMEAN = 284.3 K; HDRY = 4.5453E+05 cm; DAYUBC = 0

DAILY SUMMARY: Day = 90, Simulated Time = 24.0000 hr ______ Node Number = 2 12 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.13536 0.28045 0.28629 Head (cm) = 7.70977E+03 7.03283E+02 6.32149E+02Water Flow (cm) =-1.86185E-03-1.24616E-02 3.54217E-03 Plant Sink (cm) = 1.40160E-02 5.23463E-04 0.00000E+00PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 32.0568+ 0.0254+ 0.0000 - 0.0200- 0.2483- 0.0035 = 31.8104 Versus 31.8107 Mass Balance = -2.1553E-04 cm; Time step attempts = 3046 and successes = 3046 Evaporation: Potential = 0.0807 cm, Actual = 0.0200 cmTranspiration: Potential = 1.5324 cm, Actual = 0.2483 cm RHMEAN = 66.5 %; TMEAN = 283.2 K; HDRY = 5.6000E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 91, Simulated Time = 24.0000 hr _____ 2 == Node Number 12 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.13028 0.27931 0.28645 = 9.42956E+03 7.18303E+02 6.30399E+02 Head (cm) Water Flow (cm) =-1.44255E-03-1.35138E-02 3.57311E-03 Plant Sink (cm) = 7.51284E-03 4.40236E-04 0.00000E+00PRESTOR INFIL RUNOFF DRAIN NEWSTOR EVAPO TRANS STORAGE 31.8107+ 0.0000+ 0.0000 - 0.0168- 0.1735- 0.0036 = 31.6167 Versus 31.6168 Mass Balance = -9.5367E-06 cm; Time step attempts = 789 and successes = 789 Evaporation: Potential = 0.0672 cm, Actual = 0.0168 cm Transpiration: Potential = 1.2759 cm, Actual = 0.1735 cm RHMEAN = 59.8 %; TMEAN = 283.7 K; HDRY = 7.0524E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 92, Simulated Time = 24.0000 hr _____ Node Number = Depth (cm) = 2 12 25 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.12814 0.27828 0.28657= 1.03267E+04 7.32224E+02 6.29022E+02Head (cm) Water Flow (cm) =-1.04788E-03-1.41485E-02 3.59814E-03 Plant Sink (cm) = 4.51487E-03 3.66210E-04 0.00000E+00 INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR PRESTOR STORAGE 31.6168+ 0.0254+ 0.0000 - 0.0140- 0.1277- 0.0036 = 31.4969 Versus 31.4971 Mass Balance = -1.8311E-04 cm; Time step attempts = 2539 and successes = 2539 Evaporation: Potential = 0.0564 cm, Actual = 0.0140 cm

Transpiration: Potential = 1.0721 cm, Actual = 0.1277 cm RHMEAN = 75.0 %; TMEAN = 292.0 K; HDRY = 3.9374E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 93, Simulated Time = 24.0000 hr Node Number Depth (cm) = 2 12 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.12348 0.27684 0.28667 Head (cm) = 1.27729E+04 7.52175E+02 6.27930E+02Water Flow (cm) =-6.77568E-04-1.52543E-02 3.61865E-03 Plant Sink (cm) = 5.84221E-03 8.12040E-04 0.00000E+00PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 31.4971+ 0.0000+ 0.0000 - 0.0310- 0.2380- 0.0036 = 31.2245 Versus 31.2245 Mass Balance = 7.6294E-06 cm; Time step attempts = 1056 and successes = 1056 Evaporation: Potential = 0.1239 cm, Actual = 0.0310 cm Transpiration: Potential = 2.3535 cm, Actual = 0.2380 cm RHMEAN = 68.0 %; TMEAN = 297.9 K; HDRY = 5.2825E+05 cm; DAYUBC = Ω DAILY SUMMARY: Day = 94, Simulated Time = 24.0000 hr _____ Node Number=21225Depth (cm)=5.0000050.00000122.00000Water (cm3/cm3)=0.121600.275480.28673 = 1.40102E+04 7.71561E+02 6.27162E+02Head (cm) Water Flow (cm) =-3.92964E-04-1.65782E-02 3.63396E-03 Plant Sink (cm) = 2.28747E-03 7.66225E-04 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 31.2245+ 0.0000+ 0.0000 - 0.0292- 0.1811- 0.0036 = 31.0106 Versus 31.0105 Mass Balance = 3.2425E-05 cm; Time step attempts = 1817 and successes = 1817 Evaporation: Potential = 0.1169 cm, Actual = 0.0292 cm Transpiration: Potential = 2.2207 cm, Actual = 0.1811 cm RHMEAN = 73.3 %; TMEAN = 296.5 K; HDRY = 4.2502E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 95, Simulated Time = 24.0000 hr ____ Node Number Depth (cm) = 12 2 25 Node Number=21225Depth (cm)=5.0000050.00000122.00000Water (cm3/cm3)=0.121020.274460.28677 Head (cm) = 1.44263E+04 7.86705E+02 6.26701E+02Water Flow (cm) =-2.64750E-04-1.71649E-02 3.64407E-03 Plant Sink (cm) =  $6.74017E-04 \ 4.72762E-04 \ 0.00000E+00$ PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 31.0105+ 0.0000+ 0.0000 - 0.0180- 0.0987- 0.0036 = 30.8901 Versus 30.8901

Mass Balance = 1.7166E-05 cm; Time step attempts = 1836 and successes = 1836 Evaporation: Potential = 0.0721 cm, Actual = 0.0180 cm Transpiration: Potential = 1.3702 cm, Actual = 0.0987 cm RHMEAN = 81.9 %; TMEAN = 296.2 K; HDRY = 2.7302E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 96, Simulated Time = 24.0000 hr -----Node Number == 2 12 25 Depth (cm) = 5.00000 50.00000 122.00000Water (cm3/cm3) = 0.12036 0.27281 0.28678= 1.49241E+04 8.11764E+02 6.26582E+02 Head (cm) Water Flow (cm) =-1.68003E-04-1.81889E-02 3.64893E-03 Plant Sink (cm) = 5.89090E-04 1.14387E-03 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 30.8901+ 0.0000+ 0.0000 - 0.0436- 0.2090- 0.0036 = 30.6339 Versus 30.6347 Mass Balance = -8.2016E-04 cm; Time step attempts =13477 and successes =13477 Evaporation: Potential = 0.1745 cm, Actual = 0.0436 cm Transpiration: Potential = 3.3152 cm, Actual = 0.2090 cm RHMEAN = 62.7 %; TMEAN = 298.4 K; HDRY = 6.3905E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 97, Simulated Time = 24.0000 hr _____ Node Number == 2 12 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.11945 0.27055 0.28679Head (cm)  $= 1.56508E+04 \ 8.47764E+02 \ 6.26521E+02$ Water Flow (cm) =-8.71637E-05-2.07606E-02 3.64976E-03 Plant Sink (cm) = 2.99600E-05 1.83827E-03 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 30.6347+ 0.0000+ 0.0000 - 0.0473- 0.2712- 0.0036 = 30.3125 Versus 30.3169 Mass Balance = -4.4537E-03 cm; Time step attempts =56089 and successes =56089 Evaporation: Potential = 0.2804 cm, Actual = 0.0473 cm Transpiration: Potential = 5.3278 cm, Actual = 0.2712 cm RHMEAN = 37.9 %; TMEAN = 297.3 K; HDRY = 1.3282E+06 cm; DAYUBC = 3962 DAILY SUMMARY: Day = 98, Simulated Time = 24.0000 hr _____ 2 Node Number -----12 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.11825 0.26912 0.28678 = 1.66907E+04 8.71593E+02 6.26635E+02 Head (cm) Water Flow (cm) =-1.52874E-04-2.22680E-02 3.65004E-03 Plant Sink (cm) = 0.00000E+00 1.10337E-03 0.00000E+00

INFIL RUNOFF DRAIN NEWSTOR EVAPO TRANS PRESTOR STORAGE 30.3169+ 0.0000+ 0.0000 - 0.0000- 0.1396- 0.0037 = 30.1737 Versus 30.1737 Mass Balance = 0.0000E+00 cm; Time step attempts = 103 and successes = 103 Evaporation: Potential = 0.1683 cm, Actual = 0.0000 cm Transpiration: Potential = 3.1978 cm, Actual = 0.1396 cm RHMEAN = 67.1 %; TMEAN = 297.6 K; HDRY = 5.4756E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 99, Simulated Time = 24.0000 hr _____ Node Number Depth (cm) 2 12 = 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.11740 0.26753 0.28674 Head (cm) = 1.74938E+04 8.99035E+02 6.27036E+02Water Flow (cm) =-2.19715E-04-2.27330E-02 3.64570E-03 Plant Sink (cm) = 0.00000E+00 1.29935E-03 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 30.1737+ 0.0000+ 0.0000 - 0.0034- 0.1467- 0.0036 = 30.0199 Versus 30.0201 Mass Balance = -1.2016E-04 cm; Time step attempts = 1475 and successes = 1475Evaporation: Potential = 0.1982 cm, Actual = 0.0034 cm Transpiration: Potential = 3.7658 cm, Actual = 0.1467 cm RHMEAN = 67.0 %; TMEAN = 298.2 K; HDRY = 5.4966E+05 cm; DAYUBC = 1460 DAILY SUMMARY: Day = 100, Simulated Time = 24.0000 hr Node Number = Depth (cm) = 2 12 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.11669 0.26673 0.28669 Head (cm) = 1.82036E+04 9.13317E+02 6.27681E+02Water Flow (cm) =-2.73584E-04-2.25440E-02 3.63685E-03 Plant Sink (cm) = 0.00000E+00 5.30893E-04 0.00000E+00 INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR PRESTOR STORAGE 30.0201+ 0.0000+ 0.0000 - 0.0006- 0.0559- 0.0036 = 29.9599 Versus 29.9599 Mass Balance = -4.3869E-05 cm; Time step attempts = 948 and successes = 948Evaporation: Potential = 0.0810 cm, Actual = 0.0006 cm Transpiration: Potential = 1.5386 cm, Actual = 0.0559 cm RHMEAN = 74.4 %; TMEAN = 297.0 K; HDRY = 4.0514E+05 cm; DAYUBC = 854 DAILY SUMMARY: Day = 101, Simulated Time = 24.0000 hr _____ Node Number = 2 12 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.15149 0.26472 0.28662

Head (cm) =  $4.50990E+03 \ 9.50622E+02 \ 6.28422E+02$ Water Flow (cm) =  $7.43659E-02-2.31041E-02 \ 3.62638E-03$ Plant Sink (cm) = 1.11254E-01 1.83707E-03 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 29.9599+ 1.0718+ 0.1728 - 0.0701- 0.7311- 0.0036 = 30.2270 Versus 30.2283 Mass Balance = -1.3180E-03 cm; Time step attempts =13250 and successes =13250 Evaporation: Potential = 0.2859 cm, Actual = 0.0701 cm Transpiration: Potential = 5.4329 cm, Actual = 0.7311 cm RHMEAN = 48.7 %; TMEAN = 291.2 K; HDRY = 9.8604E+05 cm; DAYUBC = 2880 DAILY SUMMARY: Day = 102, Simulated Time = 24.0000 hr _____ Node Number = Depth (cm) = 2 12 25 5.00000 50.00000 122.00000 = 0.13725 0.26392 0.28652Water (cm3/cm3) = 7.18777E+03 9.65933E+02 6.29529E+02 Head (cm) Water Flow (cm) = 5.33129E-03-2.34257E-02 3.61030E-03 Plant Sink (cm) = 2.12081E-02 6.52571E-04 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 30.2283+ 0.0000+ 0.0000 - 0.0249- 0.1658- 0.0036 = 30.0340 Versus 30.0340 Mass Balance = 0.0000E+00 cm; Time step attempts = 1117 and successes = 1117 Evaporation: Potential = 0.0995 cm, Actual = 0.0249 cmTranspiration: Potential = 1.8913 cm, Actual = 0.1658 cm RHMEAN = 61.5 %; TMEAN = 285.9 K; HDRY = 6.6719E+05 cm; DAYUBC = DAILY SUMMARY: Day = 103, Simulated Time = 24.0000 hr _____ Node Number Depth (cm) == 2 12 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.12799 0.26303 0.28641 = 1.03955E+04 9.83255E+02 6.30835E+02 Head (cm) Water Flow (cm) = 1.55021E-03-2.28076E-02 3.59059E-03Plant Sink (cm) = 1.19452E-02 7.38300E-04 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 30.0340+ 0.0000+ 0.0000 - 0.0282- 0.1273- 0.0036 = 29.8749 Versus 29.8749 Mass Balance = -1.1444E-05 cm; Time step attempts = 1338 and successes = 1338 Evaporation: Potential = 0.1126 cm, Actual = 0.0282 cm Transpiration: Potential = 2.1398 cm, Actual = 0.1273 cm RHMEAN = 64.8 %; TMEAN = 288.4 K; HDRY = 5.9468E+05 cm; DAYUBC = 0 

DAILY SUMMARY: Day = 104, Simulated Time = 24.0000 hr

Node Number = 2 12 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.12687 0.26263 0.28628 Head (cm)  $= 1.09190E+04 \ 9.91310E+02 \ 6.32339E+02$ Water Flow (cm) = 6.52686E-04-2.20263E-02 3.56784E-03Plant Sink (cm) = 2.46247E-03 2.49386E-04 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 29.8749+ 0.0000+ 0.0000 - 0.0095- 0.0348- 0.0036 = 29.8270 Versus 29.8270 Mass Balance = 3.8147E-06 cm; Time step attempts = 332 and successes = 332 Evaporation: Potential = 0.0380 cm, Actual = 0.0095 cm Transpiration: Potential = 0.7228 cm, Actual = 0.0348 cm RHMEAN = 87.1 %; TMEAN = 289.8 K; HDRY = 1.8918E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 105, Simulated Time = 24.0000 hr _____ Node Number = 2 12 25 Depth (cm) Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.22102 0.26168 0.28615 Head (cm) = 1.04223E+03 1.01058E+03 6.33760E+02Water Flow (cm) = 5.22665E - 01 - 2.15838E - 02 3.54594E - 03Plant Sink (cm) = 5.25277E-02 8.25308E-04 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 29.8270+ 1.4166+ 0.5392 - 0.0315- 0.4352- 0.0035 = 30.7734 Versus 30.7748 Mass Balance = -1.3638E-03 cm; Time step attempts =18060 and successes =18060 Evaporation: Potential = 0.1285 cm, Actual = 0.0315 cm Transpiration: Potential = 2.4407 cm, Actual = 0.4352 cm RHMEAN = 72.9 %; TMEAN = 290.1 K; HDRY = 4.3325E+05 cm; DAYUBC = 4962 DAILY SUMMARY: Day = 106, Simulated Time = 24.0000 hr ______ 2 Node Number = 12 25 Node A(ab S1)=5.0000050.00000122.00000Depth (cm)=0.172790.260780.28599 Head (cm) = 2.59768E+03 1.02938E+03 6.35611E+02Water Flow (cm) = 4.49909E-02-2.16302E-02 3.51830E-03 Plant Sink (cm) = 5.14428E-02 8.08262E-04 0.00000E+00 INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR PRESTOR STORAGE 30.7748+ 0.0000+ 0.0000 - 0.0308- 0.4662- 0.0035 = 30.2742 Versus 30.2742 Mass Balance = -5.7220E-06 cm; Time step attempts = 1294 and successes = 1294 Evaporation: Potential = 0.1233 cm, Actual = 0.0308 cm Transpiration: Potential = 2.3425 cm, Actual = 0.4662 cm RHMEAN = 60.2 %; TMEAN = 285.4 K; HDRY = 6.9554E+05 cm; DAYUBC = 0

DAILY SUMMARY: Day = 107, Simulated Time = 24.0000 hr _____ Node Number 100 2 12 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.13836 0.25980 0.28582Head (cm) = 6.90527E+03 1.05025E+03 6.37631E+02 Water Flow (cm) = 2.98411E-03-2.16707E-02 3.48781E-03Plant Sink (cm) = 3.99763E-02 9.24577E-04 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 30.2742+ 0.0000+ 0.0000 - 0.0353- 0.3879- 0.0035 = 29.8476 Versus 29.8476 Mass Balance = 1.9073E-06 cm; Time step attempts = 1812 and successes = 1812 Evaporation: Potential = 0.1410 cm, Actual = 0.0353 cm Transpiration: Potential = 2.6796 cm, Actual = 0.3879 cm RHMEAN = 53.5 %; TMEAN = 282.0 K; HDRY = 8.5655E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 108, Simulated Time = 24.0000 hr Node Number = 2 12 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.12871 0.25907 0.28563 = 1.00779E+04 1.06628E+03 6.39799E+02 Head (cm) Water Flow (cm) =-7.34026E-04-2.15603E-02 3.45520E-03 Plant Sink (cm) = 1.23573E-02 7.08826E-04 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 29.8476+ 0.0000+ 0.0000 - 0.0270- 0.1659- 0.0035 = 29.6513 Versus 29.6513 Mass Balance = 0.0000E+00 cm; Time step attempts = 1254 and successes = 1254 Evaporation: Potential = 0.1081 cm, Actual = 0.0270 cm Transpiration: Potential = 2.0543 cm, Actual = 0.1659 cm RHMEAN = 58.9 %; TMEAN = 282.9 K; HDRY = 7.2567E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 109, Simulated Time = 24.0000 hr _ _ _ _ _ _ _ _ _ _ _ _ _ _ _ Node Number = Depth (cm) = 2 12 25 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.12377 0.25828 0.28543= 1.26016E+04 1.08385E+03 6.42099E+02Head (cm) Water Flow (cm) =-4.39154E-04-2.13546E-02 3.42084E-03 Plant Sink (cm) = 6.06026E-03 7.81890E-04 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 29.6513+ 0.0000+ 0.0000 - 0.0298- 0.1218- 0.0034 = 29.4962 Versus 29.4962 Mass Balance = -7.6294E-06 cm; Time step attempts = 1047 and successes = 1047 Evaporation: Potential = 0.1193 cm, Actual = 0.0298 cm Transpiration: Potential = 2.2661 cm, Actual = 0.1218 cm RHMEAN = 69.7 %; TMEAN = 290.1 K; HDRY = 4.9529E+05 cm; DAYUBC = 0

DAILY SUMMARY: Day = 110, Simulated Time = 24.0000 hr ------Node Number 1777 2 12 25 Depth (cm) Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.12183 0.25760 0.28522 = 1.38495E+04 1.09942E+03 6.44524E+02Head (cm) Water Flow (cm) =-2.59677E-04-2.11505E-02 3.38498E-03 Plant Sink (cm) = 2.32940E-03 6.96552E-04 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 29.4962+ 0.0000+ 0.0000 - 0.0266- 0.0804- 0.0034 = 29.3859 Versus 29.3859 Mass Balance = 5.7220E-06 cm; Time step attempts = 1465 and successes = 1465 Evaporation: Potential = 0.1063 cm, Actual = 0.0266 cm Transpiration: Potential = 2.0188 cm, Actual = 0.0804 cm RHMEAN = 76.5 %; TMEAN = 295.9 K; HDRY = 3.6803E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 111, Simulated Time = 24.0000 hr ______ 2 12 Node Number = 25 Depth (cm) = Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.12108 0.25709 0.28501 = 1.43814E+04 1.11109E+03 6.47068E+02Head (cm) Water Flow (cm) =-1.71216E-04-2.07841E-02 3.34782E-03 Plant Sink (cm) = 8.39708E-04 5.21036E-04 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 29.3859+ 0.0000+ 0.0000 - 0.0199- 0.0505- 0.0033 = 29.3122 Versus 29.3122 Mass Balance = -5.7220E-06 cm; Time step attempts = 1921 and successes = 1921 Evaporation: Potential = 0.0795 cm, Actual = 0.0199 cm Transpiration: Potential = 1.5101 cm, Actual = 0.0505 cm RHMEAN = 78.1 %; TMEAN = 294.8 K; HDRY = 3.3948E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 112, Simulated Time = 24.0000 hr Node Number == 2 12 25 Depth (cm) Depth (cm) = 5.00000 50.00000 1. Water (cm3/cm3) = 0.24457 0.25646 122.00000 0.28481 Head (cm) = 7.28287E+02 1.12594E+03 6.49339E+02Water Flow (cm) = 9.82346E-01-2.05127E-02 3.31440E-03Plant Sink (cm) = 4.25978E-02 6.69290E-04 0.00000E+00PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 29.3122+ 2.0788+ 0.8422 - 0.0255- 0.3915- 0.0033 = 30.9707 Versus 30.9727 Mass Balance = -2.0466E-03 cm; Time step attempts =22358 and successes =22358

## 40 of 130

Evaporation: Potential = 0.1052 cm, Actual = 0.0255 cm Transpiration: Potential = 1.9997 cm, Actual = 0.3915 cm RHMEAN = 73.8 %; TMEAN = 294.8 K; HDRY = 4.1700E+05 cm; DAYUBC = 6479 DAILY SUMMARY: Day = 113, Simulated Time = 24.0000 hr __________ 2 Node Number = 12 25 Depth (cm) Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.20925 0.25556 0.28459 Head (cm) = 1.26697E+03 1.14764E+03 6.51983E+02Water Flow (cm) = 1.76254E-01-2.05570E-02 3.27714E-03Plant Sink (cm) = 6.14510E-02 9.65508E-04 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 30.9727+ 0.2540+ 0.0000 - 0.0368- 0.6109- 0.0033 = 30.5757 Versus 30.5762 Mass Balance = -4.8065E-04 cm; Time step attempts = 7114 and successes = 7114 Evaporation: Potential = 0.1488 cm, Actual = 0.0368 cm Transpiration: Potential = 2.8265 cm, Actual = 0.6109 cm RHMEAN = 55.9 %; TMEAN = 287.6 K; HDRY = 7.9775E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 114, Simulated Time = 24.0000 hr _____ = 2 Node Number 12 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.17450 0.25498 0.28435Head (cm) = 2.49847E+03 1.16176E+03 6.54801E+02Water Flow (cm) = 9.91059E-03-2.04831E-02 3.23741E-03Plant Sink (cm) = 4.22340E-02 6.63573E-04 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 30.5762+ 0.0000+ 0.0000 - 0.0253- 0.4265- 0.0032 = 30.1212 Versus 30.1212 Mass Balance = 1.9073E-06 cm; Time step attempts = 1184 and successes = 1184 Evaporation: Potential = 0.1012 cm, Actual = 0.0253 cm Transpiration: Potential = 1.9232 cm, Actual = 0.4265 cm RHMEAN = 62.6 %; TMEAN = 285.9 K; HDRY = 6.4209E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 115, Simulated Time = 24.0000 hr Node Number = 2 12 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.14100 0.25423 0.28411 = 6.29250E+03 1.18052E+03 6.57703E+02 Head (cm) Water Flow (cm) =-3.74315E-03-2.03329E-02 3.19706E-03 Plant Sink (cm) =  $3.97551E-02 \ 8.55787E-04 \ 0.00000E+00$ PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE

41 of 130

30.1212+ 0.0000+ 0.0000 - 0.0326- 0.4558- 0.0032 = 29.6295 Versus 29.6295 Mass Balance = 1.9073E-06 cm; Time step attempts = 1614 and successes = 1614 Evaporation: Potential = 0.1305 cm, Actual = 0.0326 cm Transpiration: Potential = 2.4803 cm, Actual = 0.4558 cm RHMEAN = 57.6 %; TMEAN = 290.1 K; HDRY = 7.5695E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 116, Simulated Time = 24.0000 hr -----Node Number 2 12 = 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.12698 0.25339 0.28386 Head (cm) = 1.08681E+04 1.20206E+03 6.60667E+02 Water Flow (cm) =-2.12485E-03-2.03644E-02 3.15636E-03 Plant Sink (cm) = 1.67960E-02 9.81225E-04 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 29.6295+ 0.0000+ 0.0000 - 0.0374- 0.2844- 0.0032 = 29.3046 Versus 29.3046 Mass Balance = -7.6294E-06 cm; Time step attempts = 1868 and successes = 1868 Evaporation: Potential = 0.1497 cm, Actual = 0.0374 cm Transpiration: Potential = 2.8438 cm, Actual = 0.2844 cm RHMEAN = 57.4 %; TMEAN = 292.6 K; HDRY = 7.6090E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 117, Simulated Time = 24.0000 hr _____ Node Number -----2 12 25 Depth (cm) Depth (cm) = 5.00000 50.00000 12 Water (cm3/cm3) = 0.12225 0.25254 122.00000 0.28361 Head (cm)  $= 1.35609E+04 \ 1.22458E+03 \ 6.63699E+02$ Water Flow (cm) =-7.50490E-04-2.04392E-02 3.11545E-03 Plant Sink (cm) = 5.58764E-03 1.03593E-03 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 29.3046+ 0.0000+ 0.0000 - 0.0395- 0.1737- 0.0031 = 29.0883 Versus 29.0883 Mass Balance = 1.3351E-05 cm; Time step attempts = 1702 and successes = 1702 Evaporation: Potential = 0.1580 cm, Actual = 0.0395 cm Transpiration: Potential = 3.0024 cm, Actual = 0.1737 cm RHMEAN = 55.6 %; TMEAN = 293.2 K; HDRY = 8.0550E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 118, Simulated Time = 24.0000 hr _____ 2 Node Number = 12 25 Node Handel-Depth (cm)=5.0000050.00000Water (cm3/cm3)=0.120990.251920.251920.28336 Head (cm) = 1.44491E+04 1.24117E+03 6.66791E+02

Water Flow (cm) =-3.31559E-04-2.03476E-02 3.07437E-03 Plant Sink (cm) = 1.44518E-03 8.16980E-04 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 29.0883+ 0.0000+ 0.0000 - 0.0312- 0.0944- 0.0031 = 28.9596 Versus 28.9596 Mass Balance = 3.2425E-05 cm; Time step attempts = 2767 and successes = 2767 Evaporation: Potential = 0.1246 cm, Actual = 0.0312 cm Transpiration: Potential = 2.3678 cm, Actual = 0.0944 cm RHMEAN = 65.0 %; TMEAN = 292.3 K; HDRY = 5.9033E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 119, Simulated Time = 24.0000 hr _____ Node Number = 2 12 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.12038 0.25112 0.28311 Head (cm) = 1.49043E+04 1.26308E+03 6.69751E+02Water Flow (cm) =-1.72078E-04-2.02519E-02 3.03434E-03Plant Sink (cm) =  $5.40482E-04 \ 1.02415E-03 \ 0.00000E+00$ PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 28.9596+ 0.0000+ 0.0000 - 0.0391- 0.0915- 0.0030 = 28.8260 Versus 28.8267 Mass Balance = -6.8283E-04 cm; Time step attempts =10491 and successes =10491 Evaporation: Potential = 0.1562 cm, Actual = 0.0391 cm Transpiration: Potential = 2.9682 cm, Actual = 0.0915 cm RHMEAN = 61.1 %; TMEAN = 292.6 K; HDRY = 6.7459E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 120, Simulated Time = 24.0000 hr 2 Node Number test 12 25 Depth (cm) Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.12012 0.25050 0.28299 Head (cm)  $= 1.51139E+04 \ 1.28028E+03 \ 6.71245E+02$ Water Flow (cm) =-9.23643E-05-2.02822E-02 3.00252E-03 Plant Sink (cm) = 4.03405E-05 9.33849E-04 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 28.8267+ 0.0000+ 0.0000 - 0.0356- 0.0686- 0.0030 = 28.7195 Versus 28.7301 Mass Balance = -1.0601E-02 cm; Time step attempts =70464 and successes =70464 Evaporation: Potential = 0.1424 cm, Actual = 0.0356 cmTranspiration: Potential = 2.7065 cm, Actual = 0.0686 cm RHMEAN = 60.9 %; TMEAN = 292.6 K; HDRY = 6.7937E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 121, Simulated Time = 24.0000 hr _____ Node Number = 2 12 25
Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.11933 0.24968 0.28281 = 1.57532E+04 1.30392E+03 6.73429E+02Head (cm) Water Flow (cm) =-9.11965E-05-2.04983E-02 2.98578E-03 Plant Sink (cm) = 0.00000E+00 1.18829E-03 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 28.7301+ 0.0000+ 0.0000 - 0.0112- 0.0762- 0.0030 = 28.6397 Versus 28.6466 Mass Balance = -6.8874E-03 cm; Time step attempts =50313 and successes =50313 Evaporation: Potential = 0.1813 cm, Actual = 0.0112 cm Transpiration: Potential = 3.4439 cm, Actual = 0.0762 cm RHMEAN = 66.0 %; TMEAN = 295.7 K; HDRY = 5.7002E+05 cm; DAYUBC = 4998 DAILY SUMMARY: Day = 122, Simulated Time = 24.0000 hr _____ Node Number = Depth (cm) = 2 12 25 5.00000 50.00000 122.00000 Water  $(cm3/cm3) = 0.11854 \quad 0.24889$ 0.28255 Head (cm) = 1.64355E+04 1.32710E+03 6.76708E+02 Water Flow (cm) =-1.38969E-04-2.05215E-02 2.94725E-03 Plant Sink (cm) = 0.00000E+00 1.12379E-03 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 28.6466+ 0.0000+ 0.0000 - 0.0020- 0.0645- 0.0029 = 28.5772 Versus 28.5772 Mass Balance = -4.7684E-05 cm; Time step attempts = 705 and successes = 705 Evaporation: Potential = 0.1714 cm, Actual = 0.0020 cm Transpiration: Potential = 3.2570 cm, Actual = 0.0645 cm RHMEAN = 70.9 %; TMEAN = 296.2 K; HDRY = 4.7140E+05 cm; DAYUBC = 648 DAILY SUMMARY: Day = 123, Simulated Time = 24.0000 hr ______ Node Number=21225Depth (cm)=5.0000050.00000122.00000Water (cm3/cm3)=0.117800.248330.28228 Head (cm)  $= 1.71072E+04 \ 1.34362E+03 \ 6.80001E+02$ Water Flow (cm) =-1.90803E-04-2.03330E-02 2.90637E-03 Plant Sink (cm) = 0.00000E+00 8.98678E-04 0.00000E+00 INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR PRESTOR STORAGE 28.5772+ 0.0000+ 0.0000 - 0.0032- 0.0480- 0.0029 = 28.5232 Versus 28.5233 Mass Balance = -1.2207E-04 cm; Time step attempts = 1313 and successes = 1313 Evaporation: Potential = 0.1371 cm, Actual = 0.0032 cm Transpiration: Potential = 2.6046 cm, Actual = 0.0480 cm RHMEAN = 71.6 %; TMEAN = 296.2 K; HDRY = 4.5775E+05 cm; DAYUBC = 1297 

DAILY SUMMARY: Day = 124, Simulated Time = 24.0000 hr -----Node Number 2 12 25 Depth (cm) == 5.00000 50.00000 122.00000 = 0.12234 Water (cm3/cm3) 0.24793 0.28203 Head (cm) = 1.35007E+04 1.35578E+03 6.83209E+02Water Flow (cm) = 1.90887E - 05 - 1.99899E - 02 2.86722E - 03Plant Sink (cm) = 5.06501E-04 7.30287E-04 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 28.5233+ 0.2540+ 0.0000 - 0.0278- 0.0396- 0.0029 = 28.7070 Versus 28.7074 Mass Balance = -3.6430E-04 cm; Time step attempts = 8879 and successes = 8879 Evaporation: Potential = 0.1125 cm, Actual = 0.0278 cm Transpiration: Potential = 2.1379 cm, Actual = 0.0396 cm RHMEAN = 73.4 %; TMEAN = 295.4 K; HDRY = 4.2426E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 125, Simulated Time = 24.0000 hr _____ Node Number 2 12 25 = Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.18582 0.24761 0.28177 = 1.96085E+03 1.36554E+03 6.86419E+02 Head (cm) = 5.23324E-02-1.95869E-02 2.82881E-03 Water Flow (cm) Plant Sink (cm) = 3.98050E-02 6.27843E-04 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 28.7074+ 0.6304+ 0.0554 - 0.0239- 0.2218- 0.0028 = 29.0892 Versus 29.0897 Mass Balance = -4.9973E-04 cm; Time step attempts =10612 and successes =10612 Evaporation: Potential = 0.0967 cm, Actual = 0.0239 cmTranspiration: Potential = 1.8380 cm, Actual = 0.2218 cm RHMEAN = 79.8 %; TMEAN = 294.3 K; HDRY = 3.0910E+05 cm; DAYUBC = 1185 DAILY SUMMARY: Day = 126, Simulated Time = 24.0000 hr _____ Node Number = 2 12 25 Depth (cm) Depth (cm) = 5.00000 Water (cm3/cm3) = 0.15675 50.00000 122.00000 0.24738 0.28150 Head (cm) = 3.88356E+03 1.37275E+03 6.89817E+02 Water Flow (cm) = 2.69853E-02-1.91358E-02 2.78875E-03 Plant Sink (cm) = 3.21181E-025.31770E-040.00000E+00PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 29.0897+ 0.0000+ 0.0000 - 0.0203- 0.1919- 0.0028 = 28.8747 Versus 28.8747 Mass Balance = -5.7220E-06 cm; Time step attempts = 1119 and successes = 1119 Evaporation: Potential = 0.0811 cm, Actual = 0.0203 cm Transpiration: Potential = 1.5412 cm, Actual = 0.1919 cm RHMEAN = 81.0 %; TMEAN = 293.4 K; HDRY = 2.8831E+05 cm; DAYUBC = 0

DAILY SUMMARY: Day = 127, Simulated Time = 24.0000 hr _____ Node Number ..... 2 12 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.13341 0.24683 0.28123 Head (cm) = 8.31150E+03 1.38993E+03 6.93243E+02 Water Flow (cm) = 5.23179E-03-1.88665E-02 2.74915E-03Plant Sink (cm) = 2.62699E-02 8.46292E-04 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 28.8747+ 0.0000+ 0.0000 - 0.0323- 0.1838- 0.0027 = 28.6559 Versus 28.6559 Mass Balance = -7.6294E-06 cm; Time step attempts = 1647 and successes = 1647Evaporation: Potential = 0.1291 cm, Actual = 0.0323 cmTranspiration: Potential = 2.4527 cm, Actual = 0.1838 cmRHMEAN = 68.6 %; TMEAN = 290.9 K; HDRY = 5.1573E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 128, Simulated Time = 24.0000 hr 2 Node Number = 12 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.12561 0.24637 0.28096 Head (cm)  $= 1.15631E+04 \ 1.40482E+03 \ 6.96694E+02$ Water Flow (cm) = 6.94094E-04-1.87191E-02 2.70997E-03 Plant Sink (cm) = 9.28154E-03 7.79302E-04 0.00000E+00 PRESTOR INFIL RUNOFF DRAIN NEWSTOR EVAPO TRANS STORAGE 28.6559+ 0.0000+ 0.0000 - 0.0297- 0.0906- 0.0027 = 28.5329 Versus 28.5328 Mass Balance = 1.3351E-05 cm; Time step attempts = 1152 and successes = 1152 Evaporation: Potential = 0.1189 cm, Actual = 0.0297 cm Transpiration: Potential = 2.2586 cm, Actual = 0.0906 cm RHMEAN = 68.3 %; TMEAN = 290.4 K; HDRY = 5.2286E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 129, Simulated Time = 24.0000 hr Node Number = Depth (cm) = == 2 12 25 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.12190 0.24573 0.28069Head (cm) = 1.38008E+04 1.42544E+03 7.00171E+02Water Flow (cm) = 1.13803E-04-1.86225E-02 2.67122E-03 Plant Sink (cm) = 4.21908E-03 9.69867E-04 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 28.5328+ 0.0000+ 0.0000 - 0.0370- 0.0709- 0.0027 = 28.4223 Versus 28.4223 Mass Balance = 0.0000E+00 cm; Time step attempts = 1724 and successes = 1724 Evaporation: Potential = 0.1479 cm, Actual = 0.0370 cm

Transpiration: Potential = 2.8109 cm, Actual = 0.0709 cm RHMEAN = 71.2 %; TMEAN = 294.0 K; HDRY = 4.6601E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 130, Simulated Time = 24.0000 hr _____ Node Number Depth (cm) = 2 12 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.12590 0.24544 0.28043 Head (cm)  $= 1.14101E+04 \ 1.43488E+03 \ 7.03560E+02$ Water Flow (cm) = 3.44485E-04-1.84478E-02 2.63410E-03Plant Sink (cm) = 3.71578E-03 6.19131E-04 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 28.4223+ 0.1778+ 0.0000 - 0.0236- 0.0470- 0.0026 = 28.5268 Versus 28.5274 Mass Balance = -5.5122E-04 cm; Time step attempts = 6126 and successes = 6126 Evaporation: Potential = 0.0954 cm, Actual = 0.0236 cm Transpiration: Potential = 1.8125 cm, Actual = 0.0470 cm RHMEAN = 72.5 %; TMEAN = 295.7 K; HDRY = 4.4154E+05 cm; DAYUBC = DAILY SUMMARY: Day = 131, Simulated Time = 24.0000 hr ______ Node Number = 2 12 25 Node Number=21225Depth (cm)=5.0000050.00000122.00000Water (cm3/cm3)=0.221290.245370.28018 Head (cm) = 1.03773E+03 1.43711E+03 7.06798E+02 Water Flow (cm) = 3.33875E-01-1.80486E-02 2.59906E-03 Plant Sink (cm) = 2.31993E-02 3.64505E-04 0.00000E+00PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 28.5274+ 1.0148+ 0.3060 - 0.0139- 0.1677- 0.0026 = 29.3579 Versus 29.3591 Mass Balance = -1.1196E-03 cm; Time step attempts =15606 and successes =15606 Evaporation: Potential = 0.0567 cm, Actual = 0.0139 cmTranspiration: Potential = 1.0780 cm, Actual = 0.1677 cm RHMEAN = 80.4 %; TMEAN = 294.5 K; HDRY = 2.9934E+05 cm; DAYUBC = 2936 DAILY SUMMARY: Day = 132, Simulated Time = 24.0000 hr _____ Node Number Depth (cm) 12 = 2 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.16690 0.24480 0.27991 = 2.98653E+03 1.45632E+03 7.10327E+02Head (cm) Water Flow (cm) = 5.40748E - 02 - 1.77995E - 02 2.56199E - 03Plant Sink (cm) = 5.65796E-02 8.88971E-04 0.00000E+00PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 29.3591+ 0.0000+ 0.0000 - 0.0339- 0.4412- 0.0026 = 28.8814 Versus 28.8814

Mass Balance = 1.9073E-05 cm; Time step attempts = 1792 and successes = 1792 Evaporation: Potential = 0.1356 cm, Actual = 0.0339 cm Transpiration: Potential = 2.5764 cm, Actual = 0.4412 cm RHMEAN = 72.2 %; TMEAN = 294.3 K; HDRY = 4.4650E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 133, Simulated Time = 24.0000 hr _____ Node Number = 2 12 25 Depth (cm) = 5.00000 50.00000 122.00000Water (cm3/cm3) = 0.13948 0.24435 0.27964Head (cm) = 6.63476E+03 1.47158E+03 7.13882E+02Water Flow (cm) = 3.20528E-03-1.77265E-02 2.52516E-03Plant Sink (cm) = 3.29486E-02 7.83589E-04 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 28.8814+ 0.0000+ 0.0000 - 0.0299- 0.2699- 0.0025 = 28.5791 Versus 28.5791 Mass Balance = -7.6294E-06 cm; Time step attempts = 1402 and successes = 1402Evaporation: Potential = 0.1195 cm, Actual = 0.0299 cm Transpiration: Potential = 2.2710 cm, Actual = 0.2699 cm RHMEAN = 78.8 %; TMEAN = 294.3 K; HDRY = 3.2732E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 134, Simulated Time = 24.0000 hr Node Number = 2 12 25 Depth (cm) === 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.12554 0.24362 0.27937Head (cm) = 1.15992E+04 1.49697E+03 7.17448E+02Water Flow (cm) =-3.09099E-04-1.77215E-02 2.48882E-03 Plant Sink (cm) = 1.62866E-02 1.10014E-03 0.00000E+00PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 28.5791+ 0.0000+ 0.0000 - 0.0420- 0.1798- 0.0025 = 28.3549 Versus 28.3549 Mass Balance = -3.8147E-06 cm; Time step attempts = 2070 and successes = 2070 Evaporation: Potential = 0.1678 cm, Actual = 0.0420 cm Transpiration: Potential = 3.1884 cm, Actual = 0.1798 cm RHMEAN = 70.4 %; TMEAN = 297.3 K; HDRY = 4.8077E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 135, Simulated Time = 24.0000 hr Node Number = 2 12 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.12128 0.24270 0.27911 = 1.42381E+04 1.52951E+03 7.21021E+02 Head (cm) Water Flow (cm) =-2.51828E-04-1.78875E-02 2.45308E-03 Plant Sink (cm) = 4.86085E-03 1.34094E-03 0.00000E+00

INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR PRESTOR STORAGE 28.3549+ 0.0000+ 0.0000 - 0.0511- 0.1052- 0.0025 = 28.1961 Versus 28.1961 Mass Balance = 5.7220E-06 cm; Time step attempts = 2541 and successes = 2541 Evaporation: Potential = 0.2045 cm, Actual = 0.0511 cm Transpiration: Potential = 3.8863 cm, Actual = 0.1052 cm RHMEAN = 65.2 %; TMEAN = 297.9 K; HDRY = 5.8719E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 136, Simulated Time = 24.0000 hr _____ Node Number = 2 12 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.12059 0.24234 0.27884 Head (cm) = 1.47499E+04 1.54258E+03 7.24598E+02Water Flow (cm) =-1.23458E-04-1.78410E-02 2.41792E-03 Plant Sink (cm) = 7.16015E-04 8.15605E-04 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 28.1961+ 0.0000+ 0.0000 - 0.0311- 0.0425- 0.0024 = 28.1200 Versus 28.1200 Mass Balance = 1.1444E-05 cm; Time step attempts = 4277 and successes = 4277 Evaporation: Potential = 0.1244 cm, Actual = 0.0311 cm Transpiration: Potential = 2.3638 cm, Actual = 0.0425 cm RHMEAN = 74.4 %; TMEAN = 298.2 K; HDRY = 4.0602E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 137, Simulated Time = 24.0000 hr ______ Node Number Depth (cm) 12 = 2 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.20129 0.24211 0.27859 Head (cm) = 1.45762E+03 1.55099E+03 7.27960E+02Water Flow (cm) = 1.29784E-01-1.75529E-02 2.38552E-03 Plant Sink (cm) = 4.17816E-02 6.56467E-04 0.00000E+00INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR PRESTOR STORAGE 28.1200+ 0.9580+ 0.2104 - 0.0250- 0.2485- 0.0024 = 28.8021 Versus 28.8030 Mass Balance = -9.8610E-04 cm; Time step attempts =12682 and successes =12682 Evaporation: Potential = 0.1022 cm, Actual = 0.0250 cm Transpiration: Potential = 1.9414 cm, Actual = 0.2485 cm RHMEAN = 83.4 %; TMEAN = 297.0 K; HDRY = 2.4906E+05 cm; DAYUBC = 2545 DAILY SUMMARY: Day = 138, Simulated Time = 24.0000 hr _____ Node Number = 2 12 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.24897 0.24180 0.27835

Head (cm) =  $6.83441E+02 \ 1.56242E+03 \ 7.31241E+02$ Water Flow (cm) =  $1.14417E+00-1.72964E-02 \ 2.35419E-03$ Plant Sink (cm) = 4.60345E-02 7.23288E-04 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 28.8030+ 1.6896+ 0.1392 - 0.0276- 0.4221- 0.0024 = 30.0406 Versus 30.0421 Mass Balance = -1.4992E-03 cm; Time step attempts =18261 and successes =18261 Evaporation: Potential = 0.1126 cm, Actual = 0.0276 cm Transpiration: Potential = 2.1390 cm, Actual = 0.4221 cmRHMEAN = 77.3 %; TMEAN = 296.2 K; HDRY = 3.5370E+05 cm; DAYUBC = 1104 DAILY SUMMARY: Day = 139, Simulated Time = 24.0000 hr -----Node Number = Depth (cm) = 2 12 25 5.00000 50.00000 122.00000  $= 0.22275 \qquad 0.24171 \qquad 0.27809$ Water (cm3/cm3) = 1.01391E+03 1.56594E+03 7.34758E+02Head (cm) Water Flow (cm) = 9.62292E-02-1.69811E-02 2.32182E-03 Plant Sink (cm) = 3.20024E-02 5.02818E-04 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 30.0421+ 0.0762+ 0.0000 - 0.0192- 0.3138- 0.0023 = 29.7830 Versus 29.7834 Mass Balance = -4.1771E-04 cm; Time step attempts = 6946 and successes = 6946 Evaporation: Potential = 0.0775 cm, Actual = 0.0192 cm Transpiration: Potential = 1.4720 cm, Actual = 0.3138 cm RHMEAN = 84.8 %; TMEAN = 294.8 K; HDRY = 2.2653E+05 cm; DAYUBC = DAILY SUMMARY: Day = 140, Simulated Time = 24.0000 hr ______ Node Number=21225Depth (cm)=5.0000050.00000122.00000Water (cm3/cm3)=0.264530.241600.27784 = 5.49211E+02 1.56972E+03 7.38261E+02 Head (cm) Water Flow (cm) = 8.23959E-01-1.66164E-02 2.28970E-03Plant Sink (cm) = 3.06736E-02 4.81938E-04 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 29.7834+ 1.2661+ 0.0039 - 0.0184- 0.3084- 0.0023 = 30.7204 Versus 30.7209 Mass Balance = -4.5395E-04 cm; Time step attempts = 8383 and successes = 8383 Evaporation: Potential = 0.0750 cm, Actual = 0.0184 cm Transpiration: Potential = 1.4253 cm, Actual = 0.3084 cm RHMEAN = 83.2 %; TMEAN = 295.9 K; HDRY = 2.5128E+05 cm; DAYUBC = 202 

DAILY SUMMARY: Day = 141, Simulated Time = 24.0000 hr

Node Number=21225Depth (cm)=5.0000050.00000122.00000Water (cm3/cm3)=0.249590.241430.27758 Head (cm) = 6.77408E+02 1.57625E+03 7.41768E+02Water Flow (cm) = 1.84426E-01-1.63199E-02 2.25821E-03 Plant Sink (cm) = 3.41552E-02 5.36641E-04 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 30.7209+ 0.2540+ 0.0000 - 0.0205- 0.3581- 0.0023 = 30.5940 Versus 30.5945 Mass Balance = -4.4250E-04 cm; Time step attempts = 6348 and successes = 6348Evaporation: Potential = 0.0827 cm, Actual = 0.0205 cm Transpiration: Potential = 1.5710 cm, Actual = 0.3581 cm RHMEAN = 73.2 %; TMEAN = 287.9 K; HDRY = 4.2667E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 142, Simulated Time = 24.0000 hr _____ Node Number = 2 12 25 Depth (cm) Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.19828 0.24074 0.27732 Head (cm) = 1.54012E+03 1.60235E+03 7.45390E+02Water Flow (cm) =-1.18597E-02-1.62689E-02 2.22626E-03 Plant Sink (cm) = 6.84391E-02 1.07531E-03 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 30.5945+ 0.0000+ 0.0000 - 0.0410- 0.7215- 0.0022 = 29.8297 Versus 29.8297 Mass Balance = 1.9073E-06 cm; Time step attempts = 1445 and successes = 1445 Evaporation: Potential = 0.1640 cm, Actual = 0.0410 cm Transpiration: Potential = 3.1165 cm, Actual = 0.7215 cm RHMEAN = 58.4 %; TMEAN = 289.3 K; HDRY = 7.3747E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 143, Simulated Time = 24.0000 hr _____ 2 Node Number = 12 25 Note Hansel=5.0000050.00000122.00000Depth (cm)=0.155150.240140.27707Head (cm) = 4.05986E+03 1.62575E+03 7.48892E+02Water Flow (cm) =-1.47964E-02-1.63610E-02 2.19586E-03 Plant Sink (cm) = 6.19232E-02 1.02710E-03 0.00000E+00 INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR PRESTOR STORAGE 29.8297+ 0.0508+ 0.0000 - 0.0392- 0.6765- 0.0022 = 29.1627 Versus 29.1632 Mass Balance = -5.1880E-04 cm; Time step attempts = 6313 and successes = 6313 Evaporation: Potential = 0.1583 cm, Actual = 0.0392 cm Transpiration: Potential = 3.0068 cm, Actual = 0.6765 cm RHMEAN = 59.4 %; TMEAN = 291.8 K; HDRY = 7.1289E+05 cm; DAYUBC = 0 

DAILY SUMMARY: Day = 144, Simulated Time = 24.0000 hr _____ Node Number ____ 2 12 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.13286 0.23965 0.27681Head (cm) = 8.49334E+03 1.64532E+03 7.52507E+02 Water Flow (cm) =-7.64325E-03-1.63441E-02 2.16511E-03 Plant Sink (cm) = 2.89410E-02 9.55695E-04 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 29.1632+ 0.0000+ 0.0000 - 0.0364- 0.4589- 0.0022 = 28.6658 Versus 28.6658 Mass Balance = -1.1444E-05 cm; Time step attempts = 1901 and successes = 1901 Evaporation: Potential = 0.1458 cm, Actual = 0.0364 cm Transpiration: Potential = 2.7698 cm, Actual = 0.4589 cm RHMEAN = 55.8 %; TMEAN = 288.2 K; HDRY = 7.9910E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 145, Simulated Time = 24.0000 hr Node Number = 2 12 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.12553 0.23930 0.27656 Depth (cm) = 1.16030E+04 1.65942E+03 7.56133E+02 Head (cm) Water Flow (cm) =-2.22581E-03-1.62255E-02 2.13485E-03 Plant Sink (cm) = 9.58080E-03 8.24119E-04 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 28.6658+ 0.0000+ 0.0000 - 0.0314- 0.2463- 0.0021 = 28.3859 Versus 28.3859 Mass Balance = -5.7220E-06 cm; Time step attempts = 1236 and successes = 1236 Evaporation: Potential = 0.1257 cm, Actual = 0.0314 cmTranspiration: Potential = 2.3885 cm, Actual = 0.2463 cm RHMEAN = 62.8 %; TMEAN = 288.7 K; HDRY = 6.3737E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 146, Simulated Time = 24.0000 hr ______ Node Number = Depth (cm) = 2 12 25 5.00000 50.00000 122.00000 = 0.12242 0.23896 0.27630Water (cm3/cm3) = 1.34484E+04 1.67305E+03 7.59757E+02 Head (cm) Water Flow (cm) =-9.16299E-04-1.60588E-02 2.10511E-03 Plant Sink (cm) = 3.92533E-03 8.11071E-04 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 28.3859+ 0.0000+ 0.0000 - 0.0309- 0.1603- 0.0021 = 28.1926 Versus 28.1926 Mass Balance = -5.7220E-06 cm; Time step attempts = 1148 and successes = 1148 Evaporation: Potential = 0.1237 cm, Actual = 0.0309 cm Transpiration: Potential = 2.3507 cm, Actual = 0.1603 cm RHMEAN = 69.4 %; TMEAN = 291.2 K; HDRY = 5.0032E+05 cm; DAYUBC = 0

DAILY SUMMARY: Day = 147, Simulated Time = 24.0000 hr _____ Node Number = Node Number = Depth (cm) = 2 12 25 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.26424 0.238680.27608 = 5.51444E+02 1.68447E+03 7.62971E+02Head (cm) Water Flow (cm) = 1.55297E+00-1.59145E-02 2.07893E-03Plant Sink (cm) = 4.68656E-027.36346E-040.00000E+00PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 28.1926+ 2.7651+ 0.9687 - 0.0281- 0.4773- 0.0021 = 30.4503 Versus 30.4520 Mass Balance = -1.7757E-03 cm; Time step attempts =19508 and successes =19508 Evaporation: Potential = 0.1170 cm, Actual = 0.0281 cm Transpiration: Potential = 2.2230 cm, Actual = 0.4773 cm RHMEAN = 73.7 %; TMEAN = 295.7 K; HDRY = 4.1877E+05 cm; DAYUBC = 8760 DAILY SUMMARY: Day = 148, Simulated Time = 24.0000 hr _____ 2 Node Number = 12 25 Depth (cm) = 5.00000 50.0000 122.00000Water (cm3/cm3) = 0.21459 0.23820 0.27583Head (cm) = 1.15767E+03 1.70452E+03 7.66598E+02Water Flow (cm) = 2.46126E-02-1.58005E-02 2.05043E-03Plant Sink (cm)  $= 6.16804E-02 \ 9.69113E-04 \ 0.00000E+00$ PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 30.4520+ 0.0000+ 0.0000 - 0.0370- 0.6471- 0.0021 = 29.7659 Versus 29.7659 Mass Balance = -3.8147E-06 cm; Time step attempts = 983 and successes = 983 Evaporation: Potential = 0.1478 cm, Actual = 0.0370 cm Transpiration: Potential = 2.8087 cm, Actual = 0.6471 cmRHMEAN = 74.2 %; TMEAN = 295.7 K; HDRY = 4.0968E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 149, Simulated Time = 24.0000 hr _ _ _ _ _ _ _ _ _ _ _ _ _ _ _ _ _ _ _ Node Number 2 12 25 = Depth (cm) -----5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.31585 0.238570.27558 Head (cm) = 2.70346E+02 1.68920E+03 7.70120E+02Water Flow (cm) = 1.80444E+00-1.54582E-02 2.02289E-03 Plant Sink (cm) = 5.68178E-03 8.92712E-05 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 29.7659+ 2.5270+ 0.4956 - 0.0034- 0.0607- 0.0020 = 32.2269 Versus 32.2274

Mass Balance = -5.5695E-04 cm; Time step attempts = 7882 and successes = 7882 Evaporation: Potential = 0.0142 cm, Actual = 0.0034 cm Transpiration: Potential = 0.2695 cm, Actual = 0.0607 cm RHMEAN = 91.3 %; TMEAN = 293.7 K; HDRY = 1.2484E+05 cm; DAYUBC = 952 DAILY SUMMARY: Day = 150, Simulated Time = 24.0000 hr ______ Node Number = 2 12 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.34348 0.23869 0.27534 = 1.75014E+02 1.68436E+03 7.73598E+02 Head (cm) Water Flow (cm) = 1.82110E+00-1.49667E-02 1.99622E-03Plant Sink (cm) = 1.75292E-02 2.75526E-04 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 32.2274+ 2.1021+ 1.6317 - 0.0105- 0.1957- 0.0020 = 34.1213 Versus 34.1220 Mass Balance = -6.7902E-04 cm; Time step attempts =10706 and successes =10706 Evaporation: Potential = 0.0438 cm, Actual = 0.0105 cm Transpiration: Potential = 0.8318 cm, Actual = 0.1957 cm RHMEAN = 88.9 %; TMEAN = 293.4 K; HDRY = 1.6116E+05 cm; DAYUBC = 1308 _____ DAILY SUMMARY: Day = 151, Simulated Time = 24.0000 hr Node Number 100 2 12 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.32204 0.23895 0.27510Head (cm) = 2.46822E+02 1.67362E+03 7.77212E+02Water Flow (cm) = 1.01118E-01-1.45491E-02 1.96914E-03Plant Sink (cm) = 9.51504E-03 1.49499E-04 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 34.1220+ 0.0000+ 0.0000 - 0.0057- 0.1080- 0.0020 = 34.0064 Versus 34.0064 Mass Balance = 7.6294E-06 cm; Time step attempts = 1182 and successes = 1182 Evaporation: Potential = 0.0228 cm, Actual = 0.0057 cm Transpiration: Potential = 0.4333 cm, Actual = 0.1080 cm RHMEAN = 86.8 %; TMEAN = 290.1 K; HDRY = 1.9340E+05 cm; DAYUBC = DAILY SUMMARY: Day = 152, Simulated Time = 24.0000 hr _____ Node Number 2 12 200 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.28680 0.23960 0.27485 Head (cm) = 4.05108E+02 1.64710E+03 7.80828E+02 Water Flow (cm) =-2.17974E-02-1.40720E-02 1.94243E-03

Plant Sink (cm) =  $5.25827E-02 \ 8.26172E-04 \ 0.00000E+00$ 

INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR PRESTOR STORAGE 34.0064+ 0.0000+ 0.0000 - 0.0315- 0.5986- 0.0019 = 33.3743 Versus 33.3743 Mass Balance = 3.8147E-06 cm; Time step attempts = 479 and successes = 479Evaporation: Potential = 0.1260 cm, Actual = 0.0315 cmTranspiration: Potential = 2.3944 cm, Actual = 0.5986 cm RHMEAN = 79.2 %; TMEAN = 291.8 K; HDRY = 3.2004E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 153, Simulated Time = 24.0000 hr _____ Node Number = 2 12 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.24095 0.24207 0.27461 Head (cm) = 7.67841E+02 1.55231E+03 7.84441E+02Water Flow (cm) =-7.74820E-02-1.26359E-02 1.91619E-03 Plant Sink (cm) = 7.76867E-02 1.22060E-03 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 33.3743+ 0.0000+ 0.0000 - 0.0465- 0.8844- 0.0019 = 32.4414 Versus 32.4415 Mass Balance = -2.2888E-05 cm; Time step attempts = 176 and successes = 176Evaporation: Potential = 0.1862 cm, Actual = 0.0465 cm Transpiration: Potential = 3.5376 cm, Actual = 0.8844 cm RHMEAN = 59.7 %; TMEAN = 295.9 K; HDRY = 7.0789E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 154, Simulated Time = 24.0000 hr -----Node Number=21225Depth (cm)=5.0000050.00000122.00000Water (cm3/cm3)=0.206510.245540.27437 122.00000 Head (cm) = 1.32861E+03 1.43177E+03 7.88051E+02Water Flow (cm) =-7.08753E-02-9.94239E-03 1.89042E-03 Plant Sink (cm) = 5.88160E-02 9.24109E-04 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 32.4415+ 0.0000+ 0.0000 - 0.0352- 0.6696- 0.0019 = 31.7348 Versus 31.7348 Mass Balance = -3.8147E-06 cm; Time step attempts = 312 and successes = 312Evaporation: Potential = 0.1410 cm, Actual = 0.0352 cmTranspiration: Potential = 2.6783 cm, Actual = 0.6696 cm RHMEAN = 73.7 %; TMEAN = 298.4 K; HDRY = 4.1847E+05 cm; DAYUBC = DAILY SUMMARY: Day = 155, Simulated Time = 24.0000 hr _____ Node Number = 2 12 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.15162 0.24803 0.27412

= 4.49296E+03 1.35276E+03 7.91657E+02 Head (cm) Water Flow (cm) =-4.39184E-02-7.23686E-03 1.86514E-03 Plant Sink (cm) =  $7.17049E-02 \ 1.21949E-03 \ 0.00000E+00$ DRAIN NEWSTOR PRESTOR INFIL RUNOFF EVAPO TRANS STORAGE 31.7348 + 0.0000 + 0.0000 - 0.0465 - 0.8570 - 0.0019 = 30.8294 Versus 30.8294Mass Balance = -3.8147E-06 cm; Time step attempts = 1463 and successes = 1463 Evaporation: Potential = 0.1860 cm, Actual = 0.0465 cm Transpiration: Potential = 3.5344 cm, Actual = 0.8570 cm RHMEAN = 71.2 %; TMEAN = 301.2 K; HDRY = 4.6613E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 156, Simulated Time = 24.0000 hr _____ Node Number = 2 12 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.13191 0.24976 0.27389 Head (cm) = 8.82266E+03 1.30157E+03 7.95259E+02 Water Flow (cm) =-1.59931E-02-5.44958E-03 1.84031E-03 Plant Sink (cm) = 2.72473E-02 9.77967E-04 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 30.8294+ 0.0000+ 0.0000 - 0.0373- 0.5310- 0.0018 = 30.2593 Versus 30.2593 Mass Balance = 3.8147E-06 cm; Time step attempts = 1802 and successes = 1802 Evaporation: Potential = 0.1492 cm, Actual = 0.0373 cm Transpiration: Potential = 2.8344 cm, Actual = 0.5310 cm RHMEAN = 74.4 %; TMEAN = 299.8 K; HDRY = 4.0507E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 157, Simulated Time = 24.0000 hr _____ Node Number Depth (cm) -----2 12 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.12427 0.25062 0.27365 = 1.23014E+04 1.27690E+03 7.98857E+02Head (cm) Water Flow (cm) =-4.05661E-03-4.57418E-03 1.81595E-03 Plant Sink (cm) = 9.86169E-03 9.90038E-04 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 30.2593+ 0.0000+ 0.0000 - 0.0378- 0.3897- 0.0018 = 29.8300 Versus 29.8301 Mass Balance = -3.4332E-05 cm; Time step attempts = 1459 and successes = 1459 Evaporation: Potential = 0.1510 cm, Actual = 0.0378 cm Transpiration: Potential = 2.8693 cm, Actual = 0.3897 cm RHMEAN = 72.6 %; TMEAN = 298.7 K; HDRY = 4.3979E+05 cm; DAYUBC = 0 

DAILY SUMMARY: Day = 158, Simulated Time = 24.0000 hr

Node Number = 2 12 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.12152 0.25084 0.27341 Head (cm) = 1.40673E+04 1.27091E+03 8.02447E+02Water Flow (cm) =-1.31590E-03-4.55011E-03 1.79204E-03 Plant Sink (cm) = 3.37427E-03 1.01465E-03 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 29.8301+ 0.0000+ 0.0000 - 0.0387- 0.2995- 0.0018 = 29.4901 Versus 29.4902 Mass Balance = -4.3869E-05 cm; Time step attempts = 2164 and successes = 2164 Evaporation: Potential = 0.1548 cm, Actual = 0.0387 cm Transpiration: Potential = 2.9407 cm, Actual = 0.2995 cm RHMEAN = 74.2 %; TMEAN = 298.7 K; HDRY = 4.0867E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 159, Simulated Time = 24.0000 hr _____ Node Number = 2 12 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.12557 0.25085 0.27318 Depth (cm) Head (cm)  $= 1.15807E+04 \ 1.27062E+03 \ 8.05943E+02$ Water Flow (cm) =-2.22116E-04-4.93569E-03 1.76911E-03 Plant Sink (cm) = 4.56633E-03 7.91413E-04 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 29.4902+ 0.2032+ 0.0000 - 0.0302- 0.2051- 0.0018 = 29.4563 Versus 29.4565 Mass Balance = -1.4687E-04 cm; Time step attempts = 4537 and successes = 4537Evaporation: Potential = 0.1219 cm, Actual = 0.0302 cmTranspiration: Potential = 2.3169 cm, Actual = 0.2051 cm RHMEAN = 78.7 %; TMEAN = 298.7 K; HDRY = 3.2909E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 160, Simulated Time = 24.0000 hr _______ 2 12 Node Number == 25 = 5.00000 50.00000 122.00000 Depth (cm) 0.25053 0.27295 Water (cm3/cm3) = 0.12385 Head (cm)  $= 1.25492E+04 \ 1.27959E+03 \ 8.09510E+02$ Water Flow (cm) = 1.89349E - 04 - 5.52817E - 03 1.74616E - 03Plant Sink (cm) =  $5.38394E-03 \ 8.54601E-04 \ 0.00000E+00$ PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 29.4565+ 0.0000+ 0.0000 - 0.0326- 0.1977- 0.0017 = 29.2244 Versus 29.2244 Mass Balance = 7.6294E-06 cm; Time step attempts = 964 and successes = 964 Evaporation: Potential = 0.1304 cm, Actual = 0.0326 cmTranspiration: Potential = 2.4768 cm, Actual = 0.1977 cm RHMEAN = 76.5 %; TMEAN = 299.0 K; HDRY = 3.6799E+05 cm; DAYUBC = 0 

DAILY SUMMARY: Day = 161, Simulated Time = 24.0000 hr Node Number -----2 12 25 Depth (cm) ----5.00000 50.00000 122.00000 = 0.12195 0.24995 0.27272 Water (cm3/cm3) Head (cm) = 1.37666E+04 1.29597E+03 8.13063E+02Water Flow (cm) = 1.03735E-04-6.33494E-03 1.72364E-03Plant Sink (cm) = 3.29641E-03 9.28428E-04 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 29.2244+ 0.0000+ 0.0000 - 0.0354- 0.1785- 0.0017 = 29.0088 Versus 29.0088 Mass Balance = 1.9073E-05 cm; Time step attempts = 886 and successes = 886 Evaporation: Potential = 0.1416 cm, Actual = 0.0354 cm Transpiration: Potential = 2.6908 cm, Actual = 0.1785 cm RHMEAN = 76.9 %; TMEAN = 299.0 K; HDRY = 3.5917E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 162, Simulated Time = 24.0000 hr _____ Node Number = 2 12 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.12072 0.24903 0.27250 = 1.46481E+04 1.32290E+03 8.16598E+02 Head (cm) Water Flow (cm) = 1.90131E-05-7.33898E-03 1.70160E-03 Plant Sink (cm) = 1.59190E-03 1.16102E-03 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 29.0088+ 0.0000+ 0.0000 - 0.0443- 0.1840- 0.0017 = 28.7788 Versus 28.7789 Mass Balance = -2.8610E-05 cm; Time step attempts = 2703 and successes = 2703 Evaporation: Potential = 0.1771 cm, Actual = 0.0443 cm Transpiration: Potential = 3.3649 cm, Actual = 0.1840 cm RHMEAN = 71.5 %; TMEAN = 299.5 K; HDRY = 4.5902E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 163, Simulated Time = 24.0000 hr _____ Node Number = 2 12 25 Depth (cm) = 5.00000 50.00000 122.00000 = 0.12026 0.24773 0.27230 Water (cm3/cm3) Head (cm) = 1.50024E+04 1.36194E+03 8.19719E+02Water Flow (cm) =-1.37723E-05-8.58222E-03 1.68108E-03 Plant Sink (cm) = 3.90546E-04 1.46709E-03 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 28.7789+ 0.0000+ 0.0000 - 0.0559- 0.1912- 0.0017 = 28.5300 Versus 28.5319 Mass Balance = -1.8864E-03 cm; Time step attempts =17621 and successes =17621 Evaporation: Potential = 0.2238 cm, Actual = 0.0559 cm Transpiration: Potential = 4.2519 cm, Actual = 0.1912 cm

RHMEAN = 68.5 %; TMEAN = 300.1 K; HDRY = 5.1897E+05 cm; DAYUBC = 0

DAILY SUMMARY: Day = 164, Simulated Time = 24.0000 hr _____ Node Number ==== 2 12 25 Depth (cm) Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.11982 0.24651 0.27214 Head (cm) = 1.53496E+04 1.40020E+03 8.22193E+02Water Flow (cm) =-2.92815E-05-9.81524E-03 1.66461E-03 Plant Sink (cm) = 0.00000E+00 1.33857E-03 0.00000E+00INFIL RUNOFF EVAPO TRANS PRESTOR DRAIN NEWSTOR STORAGE 28.5319+ 0.0000+ 0.0000 - 0.0356- 0.1480- 0.0017 = 28.3467 Versus 28.3501 Mass Balance = -3.3817E-03 cm; Time step attempts =53737 and successes =53737 Evaporation: Potential = 0.2042 cm, Actual = 0.0356 cm Transpiration: Potential = 3.8795 cm, Actual = 0.1480 cm RHMEAN = 72.3 %; TMEAN = 301.2 K; HDRY = 4.4425E+05 cm; DAYUBC = 5044 DAILY SUMMARY: Day = 165, Simulated Time = 24.0000 hr _____ Node Number=21225Depth (cm)=5.0000050.00000122.00000Water (cm3/cm3)=0.229170.246100.27195 Head (cm)  $= 9.16802E+02 \ 1.41339E+03 \ 8.25222E+02$ Water Flow (cm) = 5.13646E-01-1.03974E-02 1.64906E-03 Plant Sink (cm) = 3.45846E-02 5.43388E-04 0.00000E+00 INFIL RUNOFF PRESTOR EVAPO TRANS DRAIN NEWSTOR STORAGE 28.3501+ 1.5530+ 0.7075 - 0.0207- 0.2918- 0.0016 = 29.5889 Versus 29.5901 Mass Balance = -1.1787E-03 cm; Time step attempts =19266 and successes =19266 Evaporation: Potential = 0.0855 cm, Actual = 0.0207 cm Transpiration: Potential = 1.6236 cm, Actual = 0.2918 cm RHMEAN = 81.4 %; TMEAN = 298.4 K; HDRY = 2.8139E+05 cm; DAYUBC = 4383 DAILY SUMMARY: Day = 166, Simulated Time = 24.0000 hr _______ Node Number Depth (cm) = 2 12 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.17344 0.24526 0.27174 Head (cm) = 2.55971E+03 1.44096E+03 8.28522E+02Water Flow (cm) = 6.09155E-02-1.06895E-02 1.62974E-03Plant Sink (cm) = 6.18528E-02 9.71822E-04 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 29.5901+ 0.0254+ 0.0000 - 0.0371- 0.5777- 0.0016 = 28.9992 Versus 28.9994

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Mass Balance = -2.5940E-04 cm; Time step attempts = 6303 and successes = 6303
Evaporation: Potential = 0.1497 cm, Actual = 0.0371 cm
Transpiration: Potential = 2.8450 cm, Actual = 0.5777 cm
RHMEAN = 64.5 %; TMEAN = 297.0 K; HDRY = 6.0028E+05 cm; DAYUBC =
                                                                    0
DAILY SUMMARY: Day = 167, Simulated Time = 24.0000 hr
_____
Node Number
                =
                       2
                                  12
                                               25
Depth (cm)
Depth (cm) = 5.00000 50.00000 122.00000
Water (cm3/cm3) = 0.13242 0.24414 0.27153
Head (cm)
                = 8.64529E+03 1.47874E+03 8.31904E+02
Water Flow (cm) = 2.69132E-03-1.12694E-02 1.61008E-03
Plant Sink (cm) = 4.69275E-02 1.26293E-03 0.00000E+00
 PRESTOR INFIL RUNOFF
                         EVAPO TRANS
                                          DRAIN NEWSTOR
                                                                  STORAGE
 28.9994+ 0.0000+ 0.0000 - 0.0482- 0.4892- 0.0016 = 28.4604 Versus 28.4604
Mass Balance = -1.9073E-06 cm; Time step attempts = 2715 and successes = 2715
Evaporation: Potential = 0.1926 cm, Actual = 0.0482 cm
Transpiration: Potential = 3.6603 cm, Actual = 0.4892 cm
RHMEAN = 60.9 %; TMEAN = 297.6 K; HDRY = 6.8006E+05 cm; DAYUBC =
                                                                    0
DAILY SUMMARY: Day = 168, Simulated Time = 24.0000 hr
Node Number =
Depth (cm) =
                       2
                                  12
                                               25
                      5.00000 50.00000 122.00000
Water (cm3/cm3) = 0.12360 0.24315 0.27132
                = 1.27009E+04 1.51326E+03 8.35249E+02
Head (cm)
Water Flow (cm) =-6.32383E-04-1.18224E-02 1.59094E-03
Plant Sink (cm) = 1.08808E-02 1.15515E-03 0.00000E+00
 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR
                                                                  STORAGE
 28.4604+ 0.0000+ 0.0000 - 0.0441- 0.2042- 0.0016 = 28.2106 Versus 28.2106
Mass Balance = 5.7220E-06 cm; Time step attempts = 1769 and successes = 1769
Evaporation: Potential = 0.1762 cm, Actual = 0.0441 cm
Transpiration: Potential = 3.3479 cm, Actual = 0.2042 cm
RHMEAN = 61.0 %; TMEAN = 298.2 K; HDRY = 6.7703E+05 cm; DAYUBC =
                                                                    0
DAILY SUMMARY: Day = 169, Simulated Time = 24.0000 hr
_____
Node Number
                -----
                        2
                                  12
                                               25
Depth (cm)
Depth (cm) = 5.00000 50.00000 122.00000
Water (cm3/cm3) = 0.12089 0.24199 0.27111
Head (cm)
                = 1.45195E+04 1.55559E+03 8.38554E+02
Water Flow (cm) =-2.41943E-04-1.23200E-02 1.57230E-03
Plant Sink (cm) = 3.16828E-03 1.36851E-03 0.00000E+00
```

PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 28.2106+ 0.0000+ 0.0000 - 0.0522- 0.1452- 0.0016 = 28.0116 Versus 28.0116 Mass Balance = -2.8610E-05 cm; Time step attempts = 3014 and successes = 3014Evaporation: Potential = 0.2087 cm, Actual = 0.0522 cmTranspiration: Potential = 3.9662 cm, Actual = 0.1452 cm RHMEAN = 62.7 %; TMEAN = 299.0 K; HDRY = 6.4076E+05 cm; DAYUBC = Ω DAILY SUMMARY: Day = 170, Simulated Time = 24.0000 hr _____ Node Number 2 12 == 25 Depth (cm) = Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.12033 0.24103 0.27093 = 1.49441E+04 1.59134E+03 8.41462E+02Head (cm) Water Flow (cm) =-9.97140E-05-1.27311E-02 1.55496E-03 Plant Sink (cm) = 5.09413E-04 1.18743E-03 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 28.0116+ 0.0000+ 0.0000 - 0.0453- 0.0925- 0.0016 = 27.8723 Versus 27.8736 Mass Balance = -1.2512E-03 cm; Time step attempts =12085 and successes =12085 Evaporation: Potential = 0.1811 cm, Actual = 0.0453 cm Transpiration: Potential = 3.4414 cm, Actual = 0.0925 cm RHMEAN = 66.8 %; TMEAN = 299.5 K; HDRY = 5.5254E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 171, Simulated Time = 24.0000 hr ______ == Node Number 12 2 25 Depth (cm) Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.12027 0.24055 0.27075 Head (cm) = 1.49898E+04 1.60990E+03 8.44464E+02Water Flow (cm) =-5.13245E-05-1.28435E-02 1.53961E-03 Plant Sink (cm) = 6.26386E-05 7.15551E-04 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 27.8736+ 0.0508+ 0.0000 - 0.0273- 0.0486- 0.0015 = 27.8470 Versus 27.8475 Mass Balance = -5.3787E-04 cm; Time step attempts =13233 and successes =13233 Evaporation: Potential = 0.1103 cm, Actual = 0.0273 cmTranspiration: Potential = 2.0948 cm, Actual = 0.0486 cm RHMEAN = 77.6 %; TMEAN = 298.2 K; HDRY = 3.4825E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 172, Simulated Time = 24.0000 hr _____ = 2 12 Node Number 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.12993 0.24024 0.27056 Head (cm) = 9.56816E+03 1.62193E+03 8.47545E+02

Water Flow (cm) = 6.86512E - 04 - 1.26980E - 02 1.52299E - 03Plant Sink (cm) = 4.69805E-035.37371E-040.00000E+00PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 27.8475+ 0.2794+ 0.0000 - 0.0205- 0.0555- 0.0015 = 28.0494 Versus 28.0497 Mass Balance = -3.0327E-04 cm; Time step attempts = 6986 and successes = 6986 Evaporation: Potential = 0.0828 cm, Actual = 0.0205 cm Transpiration: Potential = 1.5731 cm, Actual = 0.0555 cm RHMEAN = 81.8 %; TMEAN = 294.8 K; HDRY = 2.7607E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 173, Simulated Time = 24.0000 hr Node Number = 2 12 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.12634 0.23962 0.27037 Head (cm)  $= 1.11854E+04 \ 1.64656E+03 \ 8.50702E+02$ Water Flow (cm) = 1.03727E-03-1.26343E-02 1.50617E-03 Plant Sink (cm) = 9.50152E-03 8.58296E-04 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 28.0497+ 0.0000+ 0.0000 - 0.0327- 0.0953- 0.0015 = 27.9202 Versus 27.9202 Mass Balance = -1.3351E-05 cm; Time step attempts = 1342 and successes = 1342Evaporation: Potential = 0.1309 cm, Actual = 0.0327 cm Transpiration: Potential = 2.4875 cm, Actual = 0.0953 cm RHMEAN = 68.0 %; TMEAN = 294.5 K; HDRY = 5.2865E+05 cm; DAYUBC = Ο DAILY SUMMARY: Day = 174, Simulated Time = 24.0000 hr ______ 25 ___ 2 Node Number 12 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.12279 0.23887 0.27018 Head (cm) = 1.32095E+04 1.67669E+03 8.53833E+02Water Flow (cm) = 4.59365E-04-1.27386E-02 1.48976E-03 Plant Sink (cm) = 5.65882E-03 1.00185E-03 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 27.9202+ 0.0000+ 0.0000 - 0.0382- 0.0829- 0.0015 = 27.7976 Versus 27.7976 Mass Balance = -5.7220E-06 cm; Time step attempts = 982 and successes = 982 Evaporation: Potential = 0.1528 cm, Actual = 0.0382 cm Transpiration: Potential = 2.9036 cm, Actual = 0.0829 cm RHMEAN = 67.3 %; TMEAN = 296.5 K; HDRY = 5.4331E+05 cm; DAYUBC = 0 

DAILY SUMMARY: Day = 175, Simulated Time = 24.0000 hr

______

Node Number=21225Depth (cm)=5.0000050.00000122.00000Water (cm3/cm3)=0.121080.238080.26999 Node Number = 1.43820E+04 1.70976E+03 8.56939E+02Head (cm) Water Flow (cm) = 1.51531E-04-1.28871E-02 1.47370E-03Plant Sink (cm) = 2.33440E-03 1.08437E-03 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 27.7976+ 0.0000+ 0.0000 - 0.0414- 0.0679- 0.0015 = 27.6868 Versus 27.6868 Mass Balance = 1.9073E-06 cm; Time step attempts = 1676 and successes = 1676 Evaporation: Potential = 0.1654 cm, Actual = 0.0414 cm Transpiration: Potential = 3.1427 cm, Actual = 0.0679 cm RHMEAN = 66.7 %; TMEAN = 297.9 K; HDRY = 5.5541E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 176, Simulated Time = 24.0000 hr ______ Depth (cm) = 2 12 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.12043 0.23730 0.26981 122.00000 Head (cm) = 1.48681E+04 1.74278E+03 8.59994E+02Water Flow (cm) = 3.90895E - 05 - 1.30223E - 02 1.45806E - 03Plant Sink (cm) = 6.93191E-04 1.09519E-03 0.00000E+00 EVAPO TRANS PRESTOR INFIL RUNOFF DRAIN NEWSTOR STORAGE 27.6868+ 0.0000+ 0.0000 - 0.0418- 0.0567- 0.0015 = 27.5869 Versus 27.5869 Mass Balance = -5.5313E-05 cm; Time step attempts = 4913 and successes = 4913 Evaporation: Potential = 0.1671 cm, Actual = 0.0418 cm Transpiration: Potential = 3.1741 cm, Actual = 0.0567 cm RHMEAN = 66.7 %; TMEAN = 297.9 K; HDRY = 5.5541E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 177, Simulated Time = 24.0000 hr _____ Node Number 2 12 = 25 Note Nambel21220Depth (cm)=5.0000050.00000122.00000Water (cm3/cm3)=0.120210.236670.26966 Head (cm)  $= 1.50390E+04 \ 1.77034E+03 \ 8.62500E+02$ Water Flow (cm) = 1.93401E-06-1.30965E-02 1.44395E-03Plant Sink (cm) = 8.11823E-05 9.79760E-04 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 27.5869+ 0.0000+ 0.0000 - 0.0374- 0.0452- 0.0014 = 27.5029 Versus 27.5052 Mass Balance = -2.2869E-03 cm; Time step attempts =19154 and successes =19154 Evaporation: Potential = 0.1495 cm, Actual = 0.0374 cm Transpiration: Potential = 2.8396 cm, Actual = 0.0452 cm RHMEAN = 70.6 %; TMEAN = 297.3 K; HDRY = 4.7625E+05 cm; DAYUBC = 0 

DAILY SUMMARY: Day = 178, Simulated Time = 24.0000 hr _____ Node Number = 2 12 25 Depth (cm) = 5.00000 50.00000 122.00000 = 0.11975 0.23603 0.26954 Water (cm3/cm3) Head (cm) = 1.54061E+04 1.79909E+03 8.64480E+02 Water Flow (cm) =-2.24956E-05-1.31830E-02 1.43323E-03 Plant Sink (cm) = 0.00000E+00 9.82117E-04 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 27.5052+ 0.0000+ 0.0000 - 0.0303- 0.0426- 0.0014 = 27.4309 Versus 27.4363 Mass Balance = -5.4379E-03 cm; Time step attempts =64204 and successes =64204 Evaporation: Potential = 0.1498 cm, Actual = 0.0303 cm Transpiration: Potential = 2.8464 cm, Actual = 0.0426 cm RHMEAN = 70.7 %; TMEAN = 298.7 K; HDRY = 4.7547E+05 cm; DAYUBC = 4577 DAILY SUMMARY: Day = 179, Simulated Time = 24.0000 hr Node Number ----2 12 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.27091 0.23610 0.26940 = 5.03087E+02 1.79594E+03 8.66821E+02 Head (cm) Water Flow (cm) = 1.46899E+00-1.30602E-02 1.42358E-03Plant Sink (cm) = 1.72985E-02 2.71791E-04 0.00000E+00 EVAPO TRANS PRESTOR INFIL RUNOFF DRAIN NEWSTOR STORAGE 27.4363+ 2.7014+ 1.1086 - 0.0104- 0.1615- 0.0014 = 29.9644 Versus 29.9665 Mass Balance = -2.0485E-03 cm; Time step attempts =29956 and successes =29956 Evaporation: Potential = 0.0432 cm, Actual = 0.0104 cm Transpiration: Potential = 0.8205 cm, Actual = 0.1615 cm RHMEAN = 85.7 %; TMEAN = 295.4 K; HDRY = 2.1212E+05 cm; DAYUBC = 9502 DAILY SUMMARY: Day = 180, Simulated Time = 24.0000 hr _____ Node Number === 2 12 25 Depth (cm) ----5.00000 50.00000 122.00000 = 0.27622 0.23566 0.26923Water (cm3/cm3) Head (cm) = 4.67850E+02 1.81588E+03 8.69731E+02Water Flow (cm) = 8.76874E-01-1.28159E-02 1.40994E-03Plant Sink (cm) = 5.05350E-02 7.93997E-04 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 29.9665+ 1.1674+ 0.0010 - 0.0303- 0.5238- 0.0014 = 30.5784 Versus 30.5789 Mass Balance = -5.1498E-04 cm; Time step attempts =10334 and successes =10334 Evaporation: Potential = 0.1236 cm, Actual = 0.0303 cmTranspiration: Potential = 2.3481 cm, Actual = 0.5238 cm

RHMEAN = 79.5 %; TMEAN = 297.3 K; HDRY = 3.1495E+05 cm; DAYUBC = 41

DAILY SUMMARY: Day = 181, Simulated Time = 24.0000 hr Node Number ----2 12 25 Depth (cm) Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.24001 0.23531 0.26905 Head (cm) = 7.78475E+02 1.83220E+03 8.72762E+02Water Flow (cm) = 1.77548E-02-1.27183E-02 1.39542E-03Plant Sink (cm) = 4.59592E-02 7.22103E-04 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 30.5789+ 0.0000+ 0.0000 - 0.0275- 0.4884- 0.0014 = 30.0615 Versus 30.0615 Mass Balance = 3.8147E-06 cm; Time step attempts = 2222 and successes = 2222 Evaporation: Potential = 0.1101 cm, Actual = 0.0275 cm Transpiration: Potential = 2.0928 cm, Actual = 0.4884 cm RHMEAN = 79.4 %; TMEAN = 298.4 K; HDRY = 3.1659E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 182, Simulated Time = 24.0000 hr -----Node Number = 2 12 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.20157 0.23482 0.26887 Head (cm)  $= 1.45043E+03 \ 1.85493E+03 \ 8.75783E+02$ Water Flow (cm) =-2.60862E-02-1.26430E-02 1.38110E-03 Plant Sink (cm) = 5.48760E-02 8.62202E-04 0.00000E+00 INFIL RUNOFF PRESTOR EVAPO TRANS DRAIN NEWSTOR STORAGE 30.0615+ 0.0000+ 0.0000 - 0.0329- 0.5917- 0.0014 = 29.4355 Versus 29.4355 Mass Balance = -1.9073E-06 cm; Time step attempts = 912 and successes = 912 Evaporation: Potential = 0.1315 cm, Actual = 0.0329 cm Transpiration: Potential = 2.4989 cm, Actual = 0.5917 cm RHMEAN = 81.2 %; TMEAN = 298.7 K; HDRY = 2.8516E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 183, Simulated Time = 24.0000 hr _____ Node Number Depth (cm) 12 = 2 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.15479 0.23423 0.26870 Head (cm) = 4.10126E+03 1.88335E+03 8.78801E+02Water Flow (cm) =-2.49201E-02-1.26423E-02 1.36701E-03 Plant Sink (cm) = 5.96788E-02 9.92547E-04 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 29.4355+ 0.0000+ 0.0000 - 0.0379- 0.6689- 0.0014 = 28.7274 Versus 28.7274

Mass Balance = 9.5367E-06 cm; Time step attempts = 1131 and successes = 1131 Evaporation: Potential = 0.1514 cm, Actual = 0.0379 cm Transpiration: Potential = 2.8766 cm, Actual = 0.6689 cm RHMEAN = 75.3 %; TMEAN = 299.3 K; HDRY = 3.8921E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 184, Simulated Time = 24.0000 hr _____ Node Number Arrester Arrester 2 12 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.13331 0.23373 0.26852 = 8.34417E+03 1.90741E+03 8.81813E+02 Head (cm) Water Flow (cm) =-1.07068E-02-1.26368E-02 1.35312E-03 Plant Sink (cm) = 2.79033E-02 9.12113E-04 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 28.7274+ 0.0000+ 0.0000 - 0.0348- 0.4630- 0.0014 = 28.2283 Versus 28.2283 Mass Balance = -5.7220E-06 cm; Time step attempts = 1749 and successes = 1749 Evaporation: Potential = 0.1391 cm, Actual = 0.0348 cm Transpiration: Potential = 2.6435 cm, Actual = 0.4630 cm RHMEAN = 76.3 %; TMEAN = 299.3 K; HDRY = 3.7158E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 185, Simulated Time = 24.0000 hr _____ Node Number = Depth (cm) = 2 12 25 5.00000 50.00000 122.00000 = 0.12371 0.23303 0.26835 Water (cm3/cm3) = 1.26328E+04 1.94271E+03 8.84819E+02 Head (cm) Water Flow (cm) =-2.61343E-03-1.26603E-02 1.33945E-03 Plant Sink (cm) = 1.15287E-02 1.15315E-03 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 28.2283+ 0.0000+ 0.0000 - 0.0440- 0.3541- 0.0013 = 27.8289 Versus 27.8289 Mass Balance = -1.9073E-06 cm; Time step attempts = 1837 and successes = 1837 Evaporation: Potential = 0.1759 cm, Actual = 0.0440 cmTranspiration: Potential = 3.3421 cm, Actual = 0.3541 cmRHMEAN = 69.2 %; TMEAN = 301.5 K; HDRY = 5.0451E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 186, Simulated Time = 24.0000 hr Node Number Depth (cm) = 2 12 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.12156 0.23263 0.26817 = 1.40358E+04 1.96282E+03 8.87826E+02 Head (cm) Water Flow (cm) =-8.09525E-04-1.26438E-02 1.32596E-03 Plant Sink (cm) = 2.64665E-03 8.59474E-04 0.00000E+00

PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 27.8289+ 0.0000+ 0.0000 - 0.0328- 0.1628- 0.0013 = 27.6320 Versus 27.6320 Mass Balance = 1.9073E-06 cm; Time step attempts = 1927 and successes = 1927 Evaporation: Potential = 0.1311 cm, Actual = 0.0328 cm Transpiration: Potential = 2.4909 cm, Actual = 0.1628 cm RHMEAN = 72.8 %; TMEAN = 301.5 K; HDRY = 4.3456E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 187, Simulated Time = 24.0000 hr _____ Node Number == 2 12 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.12049 0.23191 0.26801 Head (cm)  $= 1.48216E+04 \ 2.00026E+03 \ 8.90593E+02$ Water Flow (cm) =-3.47604E-04-1.26353E-02 1.31311E-03 Plant Sink (cm) = 1.10634E-03 1.20658E-03 0.00000E+00PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 27.6320+0.0000+0.0000-0.0460-0.1545-0.0013 = 27.4302 Versus 27.4309 Mass Balance = -6.7520E-04 cm; Time step attempts = 8947 and successes = 8947 Evaporation: Potential = 0.1840 cm, Actual = 0.0460 cm Transpiration: Potential = 3.4969 cm, Actual = 0.1545 cm RHMEAN = 71.9 %; TMEAN = 302.6 K; HDRY = 4.5238E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 188, Simulated Time = 24.0000 hr _____ Node Number = 2 12 25 Node Number = Depth (cm) = Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.11999 0.23109 0.26796 Head (cm)  $= 1.52156E+04 \ 2.04399E+03 \ 8.91624E+02$ Water Flow (cm) =-1.40362E-04-1.27817E-02 1.30339E-03 Plant Sink (cm) = 1.31086E-04 1.35687E-03 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 27.4309+0.0000+0.0000-0.0517-0.1174-0.0013 = 27.2604 Versus 27.2694Mass Balance = -9.0199E-03 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.2070 cm, Actual = 0.0517 cmTranspiration: Potential = 3.9325 cm, Actual = 0.1174 cm RHMEAN = 67.2 %; TMEAN = 303.4 K; HDRY = 5.4446E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 189, Simulated Time = 24.0000 hr

Node Number	==	2	12	25
Depth (cm)	=	5.00000	50.00000	122.00000
Water (cm3/cm3)	=	0.11909	0.23033	0.26780

= 1.59490E+04 2.08574E+03 8.94292E+02Head (cm) Water Flow (cm) =-1.19125E-04-1.30304E-02 1.29746E-03 Plant Sink (cm) = 0.00000E+00 1.31715E-03 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 27.2694+ 0.0000+ 0.0000 - 0.0048- 0.0837- 0.0013 = 27.1796 Versus 27.1810 Mass Balance = -1.3809E-03 cm; Time step attempts =16214 and successes =16214 Evaporation: Potential = 0.2009 cm, Actual = 0.0048 cmTranspiration: Potential = 3.8174 cm, Actual = 0.0837 cmRHMEAN = 66.3 %; TMEAN = 302.9 K; HDRY = 5.6432E+05 cm; DAYUBC = 4316 DAILY SUMMARY: Day = 190, Simulated Time = 24.0000 hr _____ Node Number = Depth (cm) = 2 12 25 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.11818 0.22971 0.26765= 1.67597E+04 2.12046E+03 8.97059E+02 Head (cm) =-1.65228E-04-1.30399E-02 1.28559E-03 Water Flow (cm) Plant Sink (cm) = 0.00000E+00 1.20583E-03 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 27.1810+ 0.0000+ 0.0000 - 0.0047- 0.0607- 0.0013 = 27.1143 Versus 27.1154 Mass Balance = -1.0548E-03 cm; Time step attempts =11330 and successes =11330 Evaporation: Potential = 0.1839 cm, Actual = 0.0047 cmTranspiration: Potential = 3.4948 cm, Actual = 0.0607 cmRHMEAN = 64.8 %; TMEAN = 303.2 K; HDRY = 5.9364E+05 cm; DAYUBC = 3020 DAILY SUMMARY: Day = 191, Simulated Time = 24.0000 hr ______ 2 12 Node Number == 25 Depth (cm) Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.11737 0.22930 0.26747 = 1.75200E+04 2.14434E+03 9.00071E+02 Head (cm) Water Flow (cm) =-2.23268E-04-1.29421E-02 1.27283E-03 Plant Sink (cm) = 0.00000E+00 1.03799E-03 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 27.1154+ 0.0000+ 0.0000 - 0.0024- 0.0441- 0.0013 = 27.0676 Versus 27.0676 Mass Balance = -3.8147E-06 cm; Time step attempts = 347 and successes = 347 Evaporation: Potential = 0.1583 cm, Actual = 0.0024 cm Transpiration: Potential = 3.0083 cm, Actual = 0.0441 cm RHMEAN = 69.4 %; TMEAN = 302.3 K; HDRY = 5.0120E+05 cm; DAYUBC = 301 

DAILY SUMMARY: Day = 192, Simulated Time = 24.0000 hr

Node Number = 2 12 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.11664 0.22880 0.26730 Head (cm)  $= 1.82550E+04 \ 2.17365E+03 \ 9.03079E+02$ Water Flow (cm) =-2.74751E-04-1.28343E-02 1.26022E-03 Plant Sink (cm) = 0.00000E+00 1.12871E-03 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 27.0676+ 0.0000+ 0.0000 - 0.0030- 0.0420- 0.0013 = 27.0214 Versus 27.0215 Mass Balance = -4.9591E-05 cm; Time step attempts = 1148 and successes = 1148Evaporation: Potential = 0.1722 cm, Actual = 0.0030 cm Transpiration: Potential = 3.2713 cm, Actual = 0.0420 cm RHMEAN = 71.0 %; TMEAN = 302.6 K; HDRY = 4.6970E+05 cm; DAYUBC = 1120 DAILY SUMMARY: Day = 193, Simulated Time = 24.0000 hr _____ = Node Number 2 12 25 Depth (cm) = Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.11576 0.22812 0.26719 Head (cm)  $= 1.92075E+04 \ 2.21453E+03 \ 9.05107E+02$ Water Flow (cm) =-3.26026E-04-1.28764E-02 1.25112E-03 Plant Sink (cm) = 0.00000E+00 1.26705E-03 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 27.0215+ 0.0000+ 0.0000 - 0.0073- 0.0419- 0.0013 = 26.9710 Versus 26.9754 Mass Balance = -4.3049E-03 cm; Time step attempts = 47465 and successes = 47465Evaporation: Potential = 0.1933 cm, Actual = 0.0073 cm Transpiration: Potential = 3.6722 cm, Actual = 0.0419 cm RHMEAN = 62.0 %; TMEAN = 304.8 K; HDRY = 6.5517E+05 cm; DAYUBC = 3427 DAILY SUMMARY: Day = 194, Simulated Time = 24.0000 hr ______ 2 Node Number 12 = 25 Node Hansel=5.0000050.00000122.00000Depth (cm)=0.115000.228200.26702Water (cm3/cm3)=0.115000.228200.26702Head (cm)  $= 2.00931E+04 \ 2.20960E+03 \ 9.08142E+02$ Water Flow (cm) =-3.77223E-04-1.27035E-02 1.23936E-03 Plant Sink (cm) = 0.00000E+00 5.68322E-04 0.00000E+00 INFIL RUNOFF DRAIN NEWSTOR PRESTOR EVAPO TRANS STORAGE 26.9754+ 0.0000+ 0.0000 - 0.0000- 0.0174- 0.0012 = 26.9567 Versus 26.9567 Mass Balance = 1.9073E-06 cm; Time step attempts = 96 and successes = 96 Evaporation: Potential = 0.0867 cm, Actual = 0.0000 cm Transpiration: Potential = 1.6471 cm, Actual = 0.0174 cm RHMEAN = 82.3 %; TMEAN = 299.0 K; HDRY = 2.6705E+05 cm; DAYUBC = 0

DAILY SUMMARY: Day = 195, Simulated Time = 24.0000 hr _ _ _ _ _ _ _ _ _ _ _ _ _ _ _ _ _ _ _ Node Number = 2 12 25 Depth (cm) 500 5.00000 50.00000 122.00000 Water  $(cm3/cm3) = 0.11423 \quad 0.22788 \quad 0.26690$ Head (cm) =  $2.10386E+04 \ 2.22891E+03 \ 9.10220E+02$ Water Flow (cm) =- $4.15293E-04-1.25306E-02 \ 1.23019E-03$ Plant Sink (cm) = 0.00000E+00 9.10092E-04 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 26.9567+ 0.0000+ 0.0000 - 0.0066- 0.0266- 0.0012 = 26.9223 Versus 26.9264 Mass Balance = -4.0398E-03 cm; Time step attempts =39316 and successes =39316 Evaporation: Potential = 0.1388 cm, Actual = 0.0066 cm Transpiration: Potential = 2.6376 cm, Actual = 0.0266 cm RHMEAN = 66.0 %; TMEAN = 298.7 K; HDRY = 5.6975E+05 cm; DAYUBC = 4884 DAILY SUMMARY: Day = 196, Simulated Time = 24.0000 hr _______ Node Number 2 = 12 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.11356 0.22773 0.26674 Head (cm) = 2.19317E+04 2.23811E+03 9.13253E+02 Water Flow (cm) =-4.50996E-04-1.23677E-02 1.21872E-03 Plant Sink (cm) = 0.00000E+00 7.78250E-04 0.00000E+00 INFIL RUNOFF PRESTOR EVAPO TRANS DRAIN NEWSTOR STORAGE 26.9264+ 0.0000+ 0.0000 - 0.0006- 0.0215- 0.0012 = 26.9030 Versus 26.9030 Mass Balance = -7.6294E-06 cm; Time step attempts = 265 and successes = 265 Evaporation: Potential = 0.1187 cm, Actual = 0.0006 cmTranspiration: Potential = 2.2555 cm, Actual = 0.0215 cmRHMEAN = 73.8 %; TMEAN = 297.9 K; HDRY = 4.1586E+05 cm; DAYUBC = 162 DAILY SUMMARY: Day = 197, Simulated Time = 24.0000 hr _____ Node Number = Depth (cm) = 2 12 25 5.00000 50.00000 122.00000 Water  $(cm3/cm3) = 0.13141 \quad 0.22726 \quad 0.26658$ = 9.00251E+03 2.26722E+03 9.16104E+02 Head (cm) Water Flow (cm) = 7.33753E-04-1.22578E-02 1.20744E-03Plant Sink (cm) = 1.02023E-02 1.08548E-03 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 26.9030+ 0.4572+ 0.0000 - 0.0414- 0.0744- 0.0012 = 27.2432 Versus 27.2437 Mass Balance = -5.0545E-04 cm; Time step attempts =11753 and successes =11753 Evaporation: Potential = 0.1672 cm, Actual = 0.0414 cm Transpiration: Potential = 3.1777 cm, Actual = 0.0744 cm RHMEAN = 72.5 %; TMEAN = 302.0 K; HDRY = 4.4133E+05 cm; DAYUBC = 0

DAILY SUMMARY: Day = 198, Simulated Time = 24.0000 hr ____ ----Node Number 2 12 25 Depth (cm) = 5.00000 50.00000 122.00000 = 0.12466 Water (cm3/cm3) 0.22653 0.26641 Head (cm) = 1.20792E+04 2.31402E+03 9.19163E+02Water Flow (cm) = 1.09789E-03-1.22544E-02 1.19547E-03Plant Sink (cm)  $= 1.45210E-02 \ 1.38450E-03 \ 0.00000E+00$ PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 27.2437+ 0.0000+ 0.0000 - 0.0528- 0.0995- 0.0012 = 27.0902 Versus 27.0902 Mass Balance = 1.9073E-06 cm; Time step attempts = 2116 and successes = 2116 Evaporation: Potential = 0.2112 cm, Actual = 0.0528 cmTranspiration: Potential = 4.0126 cm, Actual = 0.0995 cm RHMEAN = 67.2 %; TMEAN = 304.3 K; HDRY = 5.4522E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 199, Simulated Time = 24.0000 hr ____ Node Number === 2 12 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.12171 0.22588 0.26624 = 1.39354E+04 2.35629E+03 9.22227E+02 Head (cm) Water Flow (cm) = 3.10227E-04-1.22733E-02 1.18363E-03 Plant Sink (cm) = 4.96033E-03 1.32093E-03 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 27.0902+ 0.0000+ 0.0000 - 0.0504- 0.0531- 0.0012 = 26.9855 Versus 26.9855 Mass Balance = -7.6294E-06 cm; Time step attempts = 1211 and successes = 1211 Evaporation: Potential = 0.2015 cm, Actual = 0.0504 cmTranspiration: Potential = 3.8283 cm, Actual = 0.0531 cm RHMEAN = 68.3 %; TMEAN = 303.7 K; HDRY = 5.2164E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 200, Simulated Time = 24.0000 hr _____ 12 Node Number 222 2 25 Depth (cm) 5.00000 = 50.00000 122.00000 = 0.12059 0.22524 Water (cm3/cm3) 0.26607 Head (cm) = 1.47446E+04 2.39991E+03 9.25299E+02Water Flow (cm) = 8.31618E-05-1.22655E-02 1.17191E-03Plant Sink (cm) = 1.51887E-03 1.34575E-03 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 26.9855+ 0.0000+ 0.0000 - 0.0513- 0.0364- 0.0012 = 26.8966 Versus 26.8966 Mass Balance = -2.0981E-05 cm; Time step attempts = 3167 and successes = 3167

Evaporation: Potential = 0.2053 cm, Actual = 0.0513 cm Transpiration: Potential = 3.9003 cm, Actual = 0.0364 cm RHMEAN = 64.5 %; TMEAN = 304.3 K; HDRY = 6.0147E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 201, Simulated Time = 24.0000 hr _____ Node Number 2 12 25 = Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.12026 0.22472 0.26593 Head (cm) = 1.49998E+04 2.43544E+03 9.27965E+02 = 1.39341E-05-1.22339E-02 1.16099E-03 Water Flow (cm) Plant Sink (cm) = 2.61970E-04 1.23158E-03 0.00000E+00 INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE PRESTOR 26.8966+ 0.0000+ 0.0000 - 0.0470- 0.0270- 0.0012 = 26.8215 Versus 26.8230 Mass Balance = -1.5697E-03 cm; Time step attempts =13190 and successes =13190 Evaporation: Potential = 0.1879 cm, Actual = 0.0470 cmTranspiration: Potential = 3.5694 cm, Actual = 0.0270 cm RHMEAN = 65.6 %; TMEAN = 302.6 K; HDRY = 5.7869E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 202, Simulated Time = 24.0000 hr _____ 25 Node Number 2 12 = Depth (cm) 5.00000 50.00000 122.00000 = Water (cm3/cm3) = 0.11981 0.22422 0.26586= 1.53541E+04 2.47028E+03 9.29203E+02Head (cm) Water Flow (cm) =-1.35303E-05-1.22170E-02 1.15290E-03 Plant Sink (cm) = 8.29827E-11 1.22838E-03 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 26.8230+ 0.0000+ 0.0000 - 0.0462- 0.0249- 0.0012 = 26.7508 Versus 26.7601 Mass Balance = -9.3555E-03 cm; Time step attempts =***** and successes =***** Evaporation: Potential = 0.1874 cm, Actual = 0.0462 cm Transpiration: Potential = 3.5601 cm, Actual = 0.0249 cm RHMEAN = 63.1 %; TMEAN = 303.4 K; HDRY = 6.3168E+05 cm; DAYUBC = 1984 DAILY SUMMARY: Day = 203, Simulated Time = 24.0000 hr _____ Node Number = 2 12 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.11875 0.22357 0.26572 = 1.62469E+04 2.51765E+03 9.31851E+02 Head (cm) Water Flow (cm) =-9.08116E-05-1.22758E-02 1.14731E-03 Plant Sink (cm) = 0.00000E+00 1.35880E-03 0.00000E+00

PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 26.7601+ 0.0000+ 0.0000 - 0.0057- 0.0266- 0.0011 = 26.7267 Versus 26.7279 Mass Balance = -1.2016E-03 cm; Time step attempts =23834 and successes =23834 Evaporation: Potential = 0.2073 cm, Actual = 0.0057 cm Transpiration: Potential = 3.9381 cm, Actual = 0.0266 cm RHMEAN = 60.0 %; TMEAN = 303.7 K; HDRY = 7.0047E+05 cm; DAYUBC = 4939 DAILY SUMMARY: Day = 204, Simulated Time = 24.0000 hr _____ 
 Node Number
 =
 2
 12
 25

 Depth (cm)
 =
 5.00000
 50.00000
 122.00000

 Water (cm3/cm3)
 =
 0.11772
 0.22302
 0.26555
 2 12 Node Number _ 25 Head (cm)  $= 1.71850E+04 \ 2.55766E+03 \ 9.34926E+02$ Water Flow (cm) =-1.76856E-04-1.22237E-02 1.13615E-03 Plant Sink (cm) = 0.00000E+00 1.30362E-03 0.00000E+00PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 26.7279+ 0.0000+ 0.0000 - 0.0047- 0.0246- 0.0011 = 26.6975 Versus 26.6975 Mass Balance = -6.1035E-05 cm; Time step attempts = 1073 and successes = 1073Evaporation: Potential = 0.1989 cm, Actual = 0.0047 cm Transpiration: Potential = 3.7782 cm, Actual = 0.0246 cm RHMEAN = 60.5 %; TMEAN = 303.4 K; HDRY = 6.8878E+05 cm; DAYUBC = 1055 DAILY SUMMARY: Day = 205, Simulated Time = 24.0000 hr Node Number = 2 12 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.11681 0.22265 0.26539 Head (cm) = 1.80881E+04 2.58570E+03 9.38012E+02Water Flow (cm) =-2.48807E-04-1.21322E-02 1.12497E-03 Plant Sink (cm) = 0.00000E+00 1.13877E-03 0.00000E+00 INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR PRESTOR STORAGE 26.6975+ 0.0000+ 0.0000 - 0.0032- 0.0208- 0.0011 = 26.6723 Versus 26.6724 Mass Balance = -2.4796E-05 cm; Time step attempts = 813 and successes = 813Evaporation: Potential = 0.1737 cm, Actual = 0.0032 cmTranspiration: Potential = 3.3004 cm, Actual = 0.0208 cm RHMEAN = 64.6 %; TMEAN = 304.0 K; HDRY = 5.9844E+05 cm; DAYUBC = 767 DAILY SUMMARY: Day = 206, Simulated Time = 24.0000 hr ______ Node Number = 2 12 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.11583 0.22213 0.26525

= 1.91311E+04 2.62568E+03 9.40574E+02 Head (cm) Water Flow (cm) =-3.11393E-04-1.20686E-02 1.11565E-03 Plant Sink (cm) = 0.00000E+00 1.24812E-03 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 26.6724+ 0.0000+ 0.0000 - 0.0058- 0.0223- 0.0011 = 26.6432 Versus 26.6448 Mass Balance = -1.5831E-03 cm; Time step attempts =23869 and successes =23869 Evaporation: Potential = 0.1904 cm, Actual = 0.0058 cm Transpiration: Potential = 3.6173 cm, Actual = 0.0223 cm RHMEAN = 60.6 %; TMEAN = 304.8 K; HDRY = 6.8563E+05 cm; DAYUBC = 1540 DAILY SUMMARY: Day = 207, Simulated Time = 24.0000 hr Node Number = Depth (cm) = 2 12 25 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.11501 0.22199 0.26508= 2.00737E+04 2.63645E+03 9.43664E+02 Head (cm) Water Flow (cm) =-3.65993E-04-1.19436E-02 1.10487E-03 Plant Sink (cm) = 0.00000E+00 9.08883E-04 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 26.6448+ 0.0000+ 0.0000 - 0.0017- 0.0159- 0.0011 = 26.6261 Versus 26.6262 Mass Balance = -6.8665E-05 cm; Time step attempts = 1091 and successes = 1091 Evaporation: Potential = 0.1386 cm, Actual = 0.0017 cm Transpiration: Potential = 2.6341 cm, Actual = 0.0159 cm RHMEAN = 68.7 %; TMEAN = 303.2 K; HDRY = 5.1525E+05 cm; DAYUBC = 1028 DAILY SUMMARY: Day = 208, Simulated Time = 24.0000 hr Node Number Depth (cm) 12 = 2 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.11423 0.22189 0.26493 = 2.10385E+04 2.64461E+03 9.46530E+02 Head (cm) Water Flow (cm) =-4.07090E-04-1.17833E-02 1.09486E-03 Plant Sink (cm) = 0.00000E+00 8.58842E-04 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 26.6262+ 0.0000+ 0.0000 - 0.0042- 0.0148- 0.0011 = 26.6060 Versus 26.6068 Mass Balance = -8.3923E-04 cm; Time step attempts = 7423 and successes = 7423 Evaporation: Potential = 0.1310 cm, Actual = 0.0042 cm Transpiration: Potential = 2.4891 cm, Actual = 0.0148 cm RHMEAN = 67.6 %; TMEAN = 303.4 K; HDRY = 5.3579E+05 cm; DAYUBC = 1023 

DAILY SUMMARY: Day = 209, Simulated Time = 24.0000 hr

Node Number = 2 12 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.11496 0.22152 0.26478 Head (cm) = 2.01322E+04 2.67363E+03 9.49467E+02Water Flow (cm) =-3.95081E-04-1.16640E-02 1.08475E-03 Plant Sink (cm) = 0.00000E+00 1.09815E-03 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 26.6068+ 0.1524+ 0.0000 - 0.0419- 0.0187- 0.0011 = 26.6976 Versus 26.6979 Mass Balance = -3.5095E-04 cm; Time step attempts =11696 and successes =11696 Evaporation: Potential = 0.1692 cm, Actual = 0.0419 cm Transpiration: Potential = 3.2148 cm, Actual = 0.0187 cm RHMEAN = 66.0 %; TMEAN = 303.7 K; HDRY = 5.6930E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 210, Simulated Time = 24.0000 hr ____ Node Number = 2 12 25 Depth (cm) Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.11518 0.22128 0.26461 Head (cm)  $= 1.98747E+04 \ 2.69268E+03 \ 9.52578E+02$ Water Flow (cm) =-3.59435E-04-1.15581E-02 1.07415E-03 Plant Sink (cm) = 0.00000E+00 9.88176E-04 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 26.6979+ 0.0000+ 0.0000 - 0.0377- 0.0165- 0.0011 = 26.6426 Versus 26.6427 Mass Balance = -2.4796E-05 cm; Time step attempts = 4265 and successes = 4265Evaporation: Potential = 0.1507 cm, Actual = 0.0377 cmTranspiration: Potential = 2.8640 cm, Actual = 0.0165 cm RHMEAN = 73.3 %; TMEAN = 302.6 K; HDRY = 4.2644E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 211, Simulated Time = 24.0000 hr ______ 2 Node Number = 12 25 Depth (cm) Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.11516 0.22074 0.26450 Head (cm) = 1.98950E+04 2.73707E+03 9.54633E+02Water Flow (cm) =-3.47092E-04-1.14979E-02 1.06494E-03 Plant Sink (cm) = 0.00000E+00 1.30300E-03 0.00000E+00DRAIN NEWSTOR INFIL RUNOFF EVAPO TRANS PRESTOR STORAGE 26.6427+ 0.0000+ 0.0000 - 0.0497- 0.0215- 0.0011 = 26.5705 Versus 26.5747 Mass Balance = -4.1943E-03 cm; Time step attempts =32021 and successes =32021 Evaporation: Potential = 0.1988 cm, Actual = 0.0497 cmTranspiration: Potential = 3.7764 cm, Actual = 0.0215 cm RHMEAN = 63.1 %; TMEAN = 304.3 K; HDRY = 6.3189E+05 cm; DAYUBC = 0

DAILY SUMMARY: Day = 212, Simulated Time = 24.0000 hr _______ Node Number = 2 12 25 Node Number = Depth (cm) = 5.00000 50.00000 122.00000 Water  $(cm3/cm3) = 0.11457 \quad 0.22014 \quad 0.26440$ Head (cm) =  $2.06081E+04 \ 2.78668E+03 \ 9.56691E+02$ Water Flow (cm) =  $-3.56586E-04-1.15273E-02 \ 1.05927E-03$ Plant Sink (cm) =  $0.00000E+00 \ 1.38866E-03 \ 0.00000E+00$ PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 26.5747+ 0.0000+ 0.0000 - 0.0256- 0.0224- 0.0011 = 26.5255 Versus 26.5309 Mass Balance = -5.3711E-03 cm; Time step attempts =52535 and successes =52535 Evaporation: Potential = 0.2118 cm, Actual = 0.0256 cm Transpiration: Potential = 4.0247 cm, Actual = 0.0224 cm RHMEAN = 59.7 %; TMEAN = 304.3 K; HDRY = 7.0617E+05 cm; DAYUBC = 3313 DAILY SUMMARY: Day = 213, Simulated Time = 24.0000 hr 2 Node Number == 12 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.11363 0.21968 0.26424 = 2.18384E+04 2.82593E+03 9.59774E+02 Head (cm) Water Flow (cm) =-4.03760E-04-1.14952E-02 1.05016E-03Plant Sink (cm) = 0.00000E+00 1.23795E-03 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 26.5309+ 0.0000+ 0.0000 - 0.0043- 0.0196- 0.0011 = 26.5059 Versus 26.5061 Mass Balance = -1.2207E-04 cm; Time step attempts = 1964 and successes = 1964 Evaporation: Potential = 0.1888 cm, Actual = 0.0043 cm Transpiration: Potential = 3.5878 cm, Actual = 0.0196 cm RHMEAN = 61.3 %; TMEAN = 302.9 K; HDRY = 6.6971E+05 cm; DAYUBC = 1963 DAILY SUMMARY: Day = 214, Simulated Time = 24.0000 hr _____ Node Number = Depth (cm) = 2 12 25 5.00000 50.00000 122.00000 = 0.11279 0.21930 0.26408 Water (cm3/cm3) = 2.30146E+04 2.85885E+03 9.62861E+02 Head (cm) Water Flow (cm) =-4.46979E-04-1.14174E-02 1.04005E-03 Plant Sink (cm) = 0.00000E+00 1.16034E-03 0.00000E+00PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 26.5061+ 0.0000+ 0.0000 - 0.0030- 0.0181- 0.0010 = 26.4839 Versus 26.4841 Mass Balance = -1.6785E-04 cm; Time step attempts = 1863 and successes = 1863 Evaporation: Potential = 0.1770 cm, Actual = 0.0030 cm Transpiration: Potential = 3.3629 cm, Actual = 0.0181 cm RHMEAN = 65.6 %; TMEAN = 302.9 K; HDRY = 5.7743E+05 cm; DAYUBC = 1837

DAILY SUMMARY: Day = 215, Simulated Time = 24.0000 hr -------Node Number = 2 12 25 Depth (cm) = 5.00000 50.00000 122.00000 Mater (cm3/cm3) = 0.111930.20000 122.00000Mater (cm3/cm3) = 0.111930.21910Head (cm) = 2.43216E+04 2.87668E+03 9.65598E+02 Water Flow (cm) =-4.83881E-04-1.13293E-02 1.03118E-03 Plant Sink (cm) = 0.00000E+00 9.70341E-04 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 26.4841+ 0.0000+ 0.0000 - 0.0049- 0.0149- 0.0010 = 26.4632 Versus 26.4645 Mass Balance = -1.3466E-03 cm; Time step attempts =15450 and successes =15450 Evaporation: Potential = 0.1480 cm, Actual = 0.0049 cm Transpiration: Potential = 2.8123 cm, Actual = 0.0149 cm RHMEAN = 63.6 %; TMEAN = 301.5 K; HDRY = 6.1925E+05 cm; DAYUBC = 1062 DAILY SUMMARY: Day = 216, Simulated Time = 24.0000 hr _____ 2 === 12 Node Number 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.11100 0.21882 0.26381 = 2.58811E+04 2.90154E+03 9.67961E+02 Head (cm) Water Flow (cm) =-5.20698E-04-1.12537E-02 1.02339E-03 Plant Sink (cm) = 0.00000E+00 1.02049E-03 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 26.4645+ 0.0000+ 0.0000 - 0.0058- 0.0156- 0.0010 = 26.4422 Versus 26.4447 Mass Balance = -2.5387E-03 cm; Time step attempts =34023 and successes =34023 Evaporation: Potential = 0.1557 cm, Actual = 0.0058 cm Transpiration: Potential = 2.9576 cm, Actual = 0.0156 cm RHMEAN = 59.8 %; TMEAN = 302.9 K; HDRY = 7.0510E+05 cm; DAYUBC = 4000 DAILY SUMMARY: Day = 217, Simulated Time = 24.0000 hr -----12 Node Number = 2 25 Depth (cm) = 5.00000 50.00000 122.00000 Dependence(cm) = 5.0000050.0000012Water (cm3/cm3) = 0.110030.218450.26366 Head (cm)  $= 2.76480E+04 \ 2.93505E+03 \ 9.70967E+02$ Water Flow (cm) =-5.56512E-04-1.11704E-02 1.01421E-03 Plant Sink (cm) = 0.00000E+00 1.14543E-03 0.00000E+00PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 26.4447+ 0.0000+ 0.0000 - 0.0051- 0.0173- 0.0010 = 26.4213 Versus 26.4218 Mass Balance = -5.0926E-04 cm; Time step attempts = 6418 and successes = 6418

Evaporation: Potential = 0.1747 cm, Actual = 0.0051 cm Transpiration: Potential = 3.3197 cm, Actual = 0.0173 cm RHMEAN = 58.9 %; TMEAN = 304.0 K; HDRY = 7.2532E+05 cm; DAYUBC = 1537 _____ DAILY SUMMARY: Day = 218, Simulated Time = 24.0000 hr ______ Node Number=21225Depth (cm)=5.0000050.00000122.00000Water (cm3/cm3)=0.109070.218140.26350 12 Head (cm) = 2.95833E+04 2.96281E+03 9.74027E+02Water Flow (cm) =-5.89344E-04-1.10876E-02 1.00466E-03 Plant Sink (cm) = 0.00000E+00 1.07512E-03 0.00000E+00PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 26.4218+ 0.0000+ 0.0000 - 0.0049- 0.0160- 0.0010 = 26.3999 Versus 26.4002 Mass Balance = -2.7847E-04 cm; Time step attempts = 3703 and successes = 3703 Evaporation: Potential = 0.1640 cm, Actual = 0.0049 cm Transpiration: Potential = 3.1159 cm, Actual = 0.0160 cm RHMEAN = 58.9 %; TMEAN = 301.5 K; HDRY = 7.2635E+05 cm; DAYUBC = 3413 ______ DAILY SUMMARY: Day = 219, Simulated Time = 24.0000 hr _____ Node Number = Depth (cm) = 2 12 25 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.10828 0.21798 0.26334= 3.13368E+04 2.97773E+03 9.77178E+02 Head (cm) Water Flow (cm) =-6.16780E-04-1.09812E-02 9.94931E-04 Plant Sink (cm) = 0.00000E+00 9.33990E-04 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 26.4002+ 0.0000+ 0.0000 - 0.0022- 0.0138- 0.0010 = 26.3832 Versus 26.3833 Mass Balance = -6.1035E-05 cm; Time step attempts = 1163 and successes = 1163 Evaporation: Potential = 0.1425 cm, Actual = 0.0022 cm Transpiration: Potential = 2.7069 cm, Actual = 0.0138 cm RHMEAN = 66.2 %; TMEAN = 301.8 K; HDRY = 5.6489E+05 cm; DAYUBC = 1097 DAILY SUMMARY: Day = 220, Simulated Time = 24.0000 hr Node Number Depth (cm) 2 12 202 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.10744 0.21771 0.26323 = 3.33774E+04 3.00350E+03 9.79435E+02 Head (cm) Water Flow (cm) =-6.40421E-04-1.09123E-02 9.87676E-04 Plant Sink (cm) = 0.00000E+00 1.00538E-03 0.00000E+00

PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 26.3833+ 0.0000+ 0.0000 - 0.0057- 0.0147- 0.0010 = 26.3618 Versus 26.3646 Mass Balance = -2.8419E-03 cm; Time step attempts =37596 and successes =37596 Evaporation: Potential = 0.1535 cm, Actual = 0.0057 cm Transpiration: Potential = 2.9172 cm, Actual = 0.0147 cm RHMEAN = 61.7 %; TMEAN = 302.9 K; HDRY = 6.6088E+05 cm; DAYUBC = 4753 DAILY SUMMARY: Day = 221, Simulated Time = 24.0000 hr _____ Node Number 202 2 12 
 Node Number
 =
 2
 12
 25

 Depth (cm)
 =
 5.00000
 50.00000
 122.00000

 Water (cm3/cm3)
 =
 0.10633
 0.21697
 0.26312
 25 Head (cm)  $= 3.63595E+04 \ 3.07448E+03 \ 9.81576E+02$ Water Flow (cm) =-6.70952E-04-1.09178E-02 9.81095E-04 Plant Sink (cm) =  $0.00000E+00 \ 1.42870E-03 \ 0.00000E+00$ PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 26.3646+ 0.0000+ 0.0000 - 0.0084- 0.0207- 0.0010 = 26.3345 Versus 26.3373 Mass Balance = -2.8381E-03 cm; Time step attempts =60628 and successes =60628 Evaporation: Potential = 0.2207 cm, Actual = 0.0084 cm Transpiration: Potential = 4.1939 cm, Actual = 0.0207 cm RHMEAN = 49.5 %; TMEAN = 306.2 K; HDRY = 9.6246E+05 cm; DAYUBC = 5566 DAILY SUMMARY: Day = 222, Simulated Time = 24.0000 hr Node Number = 2 12 25 Depth (cm) = Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.15029 0.21686 0.26298 Head (cm) = 4.67403E+03 3.08531E+03 9.84274E+02Water Flow (cm) = 6.12635E-03-1.08690E-02 9.73421E-04Plant Sink (cm) = 3.74175E-02 8.75994E-04 0.00000E+00 INFIL RUNOFF EVAPO TRANS PRESTOR DRAIN NEWSTOR STORAGE 26.3373+ 0.7112+ 0.0000 - 0.0342- 0.1810- 0.0010 = 26.8324 Versus 26.8337 Mass Balance = -1.3142E-03 cm; Time step attempts =17972 and successes =17972 Evaporation: Potential = 0.1382 cm, Actual = 0.0342 cm Transpiration: Potential = 2.6254 cm, Actual = 0.1810 cm RHMEAN = 64.0 %; TMEAN = 303.2 K; HDRY = 6.1114E+05 cm; DAYUBC = DAILY SUMMARY: Day = 223, Simulated Time = 24.0000 hr _____ Node Number = 2 12 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.14468 0.21722 0.26282
Head (cm) =  $5.56587E+03 \ 3.05029E+03 \ 9.87463E+02$ Water Flow (cm) =  $6.86827E-03-1.06993E-02 \ 9.64095E-04$ Plant Sink (cm) = 1.59775E-02 4.05758E-04 0.00000E+00PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 26.8337+ 0.0000+ 0.0000 - 0.0158- 0.0790- 0.0010 = 26.7379 Versus 26.7379 Mass Balance = -3.8147E-06 cm; Time step attempts = 847 and successes = 847 Evaporation: Potential = 0.0631 cm, Actual = 0.0158 cm Transpiration: Potential = 1.1994 cm, Actual = 0.0790 cm RHMEAN = 81.9 %; TMEAN = 300.1 K; HDRY = 2.7336E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 224, Simulated Time = 24.0000 hr ______ Node Number = Depth (cm) = 2 12 25 5.00000 50.00000 122.00000 = 0.13094 0.21722 0.26266 Water (cm3/cm3) = 9.17533E+03 3.05034E+03 9.90650E+02Head (cm) Water Flow (cm) = 3.12339E-03-1.05290E-02 9.54776E-04Plant Sink (cm) = 1.73060E-02 7.46843E-04 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 26.7379+ 0.0000+ 0.0000 - 0.0289- 0.0923- 0.0010 = 26.6158 Versus 26.6158 Mass Balance = -5.7220E-06 cm; Time step attempts = 1417 and successes = 1417 Evaporation: Potential = 0.1156 cm, Actual = 0.0289 cm Transpiration: Potential = 2.1966 cm, Actual = 0.0923 cm RHMEAN = 73.1 %; TMEAN = 301.8 K; HDRY = 4.2926E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 225, Simulated Time = 24.0000 hr _____ Node Number Depth (cm) 2 12 = 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.12340 0.21693 0.26250 = 1.28260E+04 3.07839E+03 9.93834E+02 Head (cm) Water Flow (cm) = 7.51257E-04-1.04302E-02 9.45582E-04 Plant Sink (cm) = 8.88952E-03 1.02883E-03 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 26.6158+ 0.0000+ 0.0000 - 0.0400- 0.0579- 0.0009 = 26.5170 Versus 26.5170 Mass Balance = -1.9073E-06 cm; Time step attempts = 1389 and successes = 1389Evaporation: Potential = 0.1599 cm, Actual = 0.0400 cm Transpiration: Potential = 3.0385 cm, Actual = 0.0579 cm RHMEAN = 59.8 %; TMEAN = 301.5 K; HDRY = 7.0395E+05 cm; DAYUBC = 0 

DAILY SUMMARY: Day = 226, Simulated Time = 24.0000 hr

Node Number = 2 12 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.12113 0.21664 0.26235 Head (cm) = 1.43482E+04 3.10631E+03 9.97022E+02Water Flow (cm) = 1.55027E - 04 - 1.03615E - 02 9.36499E - 04 Plant Sink (cm) = 2.60603E-03 1.02265E-03 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 26.5170+ 0.0000+ 0.0000 - 0.0401- 0.0277- 0.0009 = 26.4482 Versus 26.4482 Mass Balance = -1.9073E-06 cm; Time step attempts = 2057 and successes = 2057 Evaporation: Potential = 0.1603 cm, Actual = 0.0401 cm Transpiration: Potential = 3.0448 cm, Actual = 0.0277 cm RHMEAN = 57.2 %; TMEAN = 300.1 K; HDRY = 7.6624E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 227, Simulated Time = 24.0000 hr ______ Node Number == 2 12 25 Depth (cm) Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.20468 0.21636 0.26221 122.00000 Head (cm) = 1.37200E+03 3.13482E+03 9.99876E+02Water Flow (cm) = 3.52195E-01-1.03023E-02 9.28391E-04 Plant Sink (cm) = 6.66758E-02 1.01154E-03 0.00000E+00 INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR PRESTOR STORAGE 26.4482+ 1.3034+ 0.5508 - 0.0399- 0.4557- 0.0009 = 27.2550 Versus 27.2561 Mass Balance = -1.0586E-03 cm; Time step attempts =15260 and successes =15260 Evaporation: Potential = 0.1631 cm, Actual = 0.0399 cmTranspiration: Potential = 3.0981 cm, Actual = 0.4557 cmRHMEAN = 63.4 %; TMEAN = 301.2 K; HDRY = 6.2507E+05 cm; DAYUBC = 4895 DAILY SUMMARY: Day = 228, Simulated Time = 24.0000 hr _ _ _ _ _ _ _ _ _ _ _ _ _ _ Node Number = 2 12 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.15661 0.21628 0.26205 Head (cm) = 3.89815E+03 3.14259E+03 1.00307E+03 Water Flow (cm) = 3.77400E-02-1.02163E-02 9.19578E-04 Plant Sink (cm) = 5.15796E-02 8.10249E-04 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 27.2561+ 0.0000+ 0.0000 - 0.0322- 0.3590- 0.0009 = 26.8640 Versus 26.8640 Mass Balance = 1.9073E-06 cm; Time step attempts = 1673 and successes = 1673 Evaporation: Potential = 0.1287 cm, Actual = 0.0322 cm Transpiration: Potential = 2.4446 cm, Actual = 0.3590 cm RHMEAN = 69.7 %; TMEAN = 299.5 K; HDRY = 4.9455E+05 cm; DAYUBC = 0 

DAILY SUMMARY: Day = 229, Simulated Time = 24.0000 hr _____ Node Number = 2 12 25 Depth (cm) = 5.00000 50.00000 122.00000 = 0.13305 0.21613 0.26189Water (cm3/cm3) Head (cm) = 8.43124E+03 3.15734E+03 1.00625E+03 Water Flow (cm) = 1.99249E-03-1.01191E-02 9.10815E-04 Plant Sink (cm) = 2.80219E-02 8.72008E-04 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 26.8640+ 0.0000+ 0.0000 - 0.0347- 0.2060- 0.0009 = 26.6224 Versus 26.6224 Mass Balance = -1.3351E-05 cm; Time step attempts = 1924 and successes = 1924 Evaporation: Potential = 0.1389 cm, Actual = 0.0347 cm Transpiration: Potential = 2.6395 cm, Actual = 0.2060 cm RHMEAN = 70.9 %; TMEAN = 299.8 K; HDRY = 4.7128E+05 cm; DAYUBC = DAILY SUMMARY: Day = 230, Simulated Time = 24.0000 hr ________ Node Number 2 12 = 25 Depth (cm) = 5.00000 50.0000 122.00000 Water (cm3/cm3) = 0.12461 0.21595 0.26174 = 1.21066E+04 3.17575E+03 1.00944E+03 Head (cm) Water Flow (cm) =-1.64123E-04-1.00349E-02 9.02166E-04 Plant Sink (cm) = 1.01895E-02 9.01298E-04 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 26.6224+ 0.0000+ 0.0000 - 0.0361- 0.0926- 0.0009 = 26.4929 Versus 26.4929 Mass Balance = -1.5259E-05 cm; Time step attempts = 1494 and successes = 1494 Evaporation: Potential = 0.1443 cm, Actual = 0.0361 cm Transpiration: Potential = 2.7411 cm, Actual = 0.0926 cmRHMEAN = 71.2 %; TMEAN = 300.1 K; HDRY = 4.6583E+05 cm; DAYUBC = 0 _____ DAILY SUMMARY: Day = 231, Simulated Time = 24.0000 hr ______ Node Number = 2 12 25 Depth (cm) = 5.00000 50.00000 122.00000 = 0.12134 0.21558 0.26158Water (cm3/cm3) Head (cm) = 1.41930E+04 3.21333E+03 1.01262E+03 Water Flow (cm) =-1.59513E-04-9.97282E-03 8.93629E-04 Plant Sink (cm) = 3.76949E-03 1.08447E-03 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 26.4929+ 0.0000+ 0.0000 - 0.0437- 0.0510- 0.0009 = 26.3973 Versus 26.3973 Mass Balance = 5.7220E-06 cm; Time step attempts = 2372 and successes = 2372 Evaporation: Potential = 0.1750 cm, Actual = 0.0437 cmTranspiration: Potential = 3.3246 cm, Actual = 0.0510 cm

RHMEAN = 53.6 %; TMEAN = 299.3 K; HDRY = 8.5417E+05 cm; DAYUBC =

DAILY SUMMARY: Day = 232, Simulated Time = 24.0000 hr ______ Node Number Depth (cm) 2 12 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.12046 0.21524 0.26144 Head (cm)  $= 1.48499E+04 \ 3.24906E+03 \ 1.01565E+03$ Water Flow (cm) =-8.91897E-05-9.92654E-03 8.85378E-04 Plant Sink (cm) =  $8.80293E-04 \ 1.06374E-03 \ 0.00000E+00$ PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 26.3973+ 0.0000+ 0.0000 - 0.0434- 0.0268- 0.0009 = 26.3263 Versus 26.3268 Mass Balance = -5.6648E-04 cm; Time step attempts = 7854 and successes = 7854Evaporation: Potential = 0.1734 cm, Actual = 0.0434 cm Transpiration: Potential = 3.2950 cm, Actual = 0.0268 cm RHMEAN = 58.6 %; TMEAN = 301.2 K; HDRY = 7.3246E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 233, Simulated Time = 24.0000 hr 2 Node Number 12 === 25 Note Hanbel=5.0000050.00000122.00000Depth (cm)=5.000000.214650.26138Water (cm3/cm3)=0.120060.214650.26138 Head (cm)  $= 1.51568E+04 \ 3.31226E+03 \ 1.01679E+03$ Water Flow (cm) =-4.82940E-05-9.91710E-03 8.79016E-04 Plant Sink (cm) = 1.04118E-04 1.26570E-03 0.00000E+00 INFIL RUNOFF EVAPO TRANS PRESTOR DRAIN NEWSTOR STORAGE 26.3268+ 0.0000+ 0.0000 - 0.0523- 0.0221- 0.0009 = 26.2516 Versus 26.2586 Mass Balance = -7.0591E-03 cm; Time step attempts =92705 and successes =92705 Evaporation: Potential = 0.2091 cm, Actual = 0.0523 cm Transpiration: Potential = 3.9724 cm, Actual = 0.0221 cm RHMEAN = 58.9 %; TMEAN = 302.3 K; HDRY = 7.2602E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 234, Simulated Time = 24.0000 hr -----Node Number Depth (cm) = 2 12 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.11903 0.21431 0.26129 Head (cm)  $= 1.60025E+04 \ 3.35007E+03 \ 1.01877E+03$ Water Flow (cm) =-8.33346E-05-9.96734E-03 8.76819E-04 Plant Sink (cm) = 0.00000E+00 1.05508E-03 0.00000E+00 INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR PRESTOR STORAGE 26.2586+ 0.0000+ 0.0000 - 0.0113- 0.0164- 0.0009 = 26.2301 Versus 26.2343

Mass Balance = -4.1676E-03 cm; Time step attempts = 60472 and successes = 60472Evaporation: Potential = 0.1770 cm, Actual = 0.0113 cm Transpiration: Potential = 3.3622 cm, Actual = 0.0164 cm RHMEAN = 55.9 %; TMEAN = 303.7 K; HDRY = 7.9777E+05 cm; DAYUBC = 4300 DAILY SUMMARY: Day = 235, Simulated Time = 24.0000 hr _____ Node Number Depth (cm) = 2 12 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.18393 0.21452 0.26114 = 2.03825E+03 3.32612E+03 1.02172E+03 Head (cm) Water Flow (cm) = 4.06601E - 02 - 9.87738E - 03 8.69767E - 04Plant Sink (cm) = 3.46842E-02 4.99306E-04 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 26.2343+ 0.8068+ 0.0060 - 0.0210- 0.1707- 0.0009 = 26.8485 Versus 26.8491 Mass Balance = -6.0463E-04 cm; Time step attempts =13132 and successes =13132 Evaporation: Potential = 0.0847 cm, Actual = 0.0210 cm Transpiration: Potential = 1.6096 cm, Actual = 0.1707 cm RHMEAN = 67.6 %; TMEAN = 300.9 K; HDRY = 5.3764E+05 cm; DAYUBC = 95DAILY SUMMARY: Day = 236, Simulated Time = 24.0000 hr _____ Node Number = Depth (cm) = 2 12 25 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.15516 0.21435 0.26100= 4.05826E+03 3.34520E+03 1.02480E+03 Head (cm) Water Flow (cm) = 3.63705E - 02 - 9.76736E - 03 8.61918E - 04Plant Sink (cm) = 5.91187E-02 8.89620E-04 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 26.8491+ 0.1778+ 0.0000 - 0.0373- 0.3133- 0.0009 = 26.6755 Versus 26.6757 Mass Balance = -2.5368E-04 cm; Time step attempts = 6507 and successes = 6507 Evaporation: Potential = 0.1508 cm, Actual = 0.0373 cm Transpiration: Potential = 2.8658 cm, Actual = 0.3133 cm RHMEAN = 63.4 %; TMEAN = 301.2 K; HDRY = 6.2507E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 237, Simulated Time = 24.0000 hr ______ 2 Node Number = 12 25 Depth (cm) Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.21093 0.21424 0.26086 Head (cm) = 1.23124E+03 3.35737E+03 1.02772E+03 Water Flow (cm) = 4.21028E-01-9.68911E-03 8.54479E-04Plant Sink (cm) = 5.82902E-02 8.28916E-04 0.00000E+00

PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 26.6757+ 1.0447+ 0.1999 - 0.0349- 0.4096- 0.0009 = 27.2751 Versus 27.2759 Mass Balance = -7.8201E-04 cm; Time step attempts = 8911 and successes = 8911 Evaporation: Potential = 0.1426 cm, Actual = 0.0349 cm Transpiration: Potential = 2.7085 cm, Actual = 0.4096 cm RHMEAN = 63.5 %; TMEAN = 302.6 K; HDRY = 6.2263E+05 cm; DAYUBC = 319 DAILY SUMMARY: Day = 238, Simulated Time = 24.0000 hr ______ Node Number 2 12 == 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.16392 0.21422 0.26071 Head (cm) = 3.21558E+03 3.35962E+03 1.03089E+03Water Flow (cm) = 3.45644E-02-9.59828E-03 8.46605E-04Plant Sink (cm) = 5.12241E-027.30158E-040.00000E+00PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 27.2759+ 0.0000+ 0.0000 - 0.0308- 0.3849- 0.0008 = 26.8593 Versus 26.8593 Mass Balance = 0.0000E+00 cm; Time step attempts = 1456 and successes = 1456 Evaporation: Potential = 0.1233 cm, Actual = 0.0308 cm Transpiration: Potential = 2.3432 cm, Actual = 0.3849 cm RHMEAN = 72.6 %; TMEAN = 300.1 K; HDRY = 4.3936E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 239, Simulated Time = 24.0000 hr _____ Node Number ----12 2 25 Depth (cm) Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.13609 0.21413 0.26056 Head (cm) = 7.50311E+03 3.36989E+03 1.03407E+03Water Flow (cm) = 1.81774E-03-9.50811E-03 8.38773E-04Plant Sink (cm) = 3.28775E-02 7.94719E-04 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 26.8593+ 0.0000+ 0.0000 - 0.0336- 0.2587- 0.0008 = 26.5662 Versus 26.5662 Mass Balance = -1.9073E-05 cm; Time step attempts = 1695 and successes = 1695Evaporation: Potential = 0.1345 cm, Actual = 0.0336 cm Transpiration: Potential = 2.5548 cm, Actual = 0.2587 cm RHMEAN = 69.4 %; TMEAN = 301.2 K; HDRY = 5.0123E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 240, Simulated Time = 24.0000 hr _____ 2 12 Node Number = 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.12609 0.21403 0.26041 Head (cm)  $= 1.13089E+04 \ 3.38079E+03 \ 1.03724E+03$ 

Water Flow (cm) =-4.78388E-04-9.42701E-03 8.31022E-04 Plant Sink (cm) = 1.21534E-02 7.93258E-04 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 26.5662+ 0.0000+ 0.0000 - 0.0337- 0.1169- 0.0008 = 26.4147 Versus 26.4148 Mass Balance = -2.0981E-05 cm; Time step attempts = 1434 and successes = 1434Evaporation: Potential = 0.1346 cm, Actual = 0.0337 cm Transpiration: Potential = 2.5576 cm, Actual = 0.1169 cm RHMEAN = 71.9 %; TMEAN = 300.1 K; HDRY = 4.5259E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 241, Simulated Time = 24.0000 hr _____ 12 25 Node Number = 2 Node Number=21225Depth (cm)=5.0000050.00000122.00000Water (cm3/cm3)=0.122330.213910.26026 = 1.35093E+04 3.39359E+03 1.04043E+03Head (cm) Water Flow (cm) =-3.01859E-04-9.34988E-03 8.23362E-04 Plant Sink (cm) = 4.51470E-03 8.03684E-04 0.00000E+00EVAPO TRANS PRESTOR INFIL RUNOFF DRAIN NEWSTOR STORAGE 26.4148+ 0.0000+ 0.0000 - 0.0342- 0.0586- 0.0008 = 26.3211 Versus 26.3211 Mass Balance = -7.6294E-06 cm; Time step attempts = 1211 and successes = 1211 Evaporation: Potential = 0.1368 cm, Actual = 0.0342 cm Transpiration: Potential = 2.5997 cm, Actual = 0.0586 cm RHMEAN = 71.3 %; TMEAN = 299.3 K; HDRY = 4.6448E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 242, Simulated Time = 24.0000 hr Node Number = 2 12 25 Depth (cm) Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.12095 0.21382 0.26011 = 1.44789E+04 3.40437E+03 1.04359E+03 Head (cm) Water Flow (cm) =-1.69212E-04-9.27448E-03 8.15798E-04 Plant Sink (cm) = 1.54562E-03 7.79877E-04 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 26.3211+ 0.0000+ 0.0000 - 0.0333- 0.0318- 0.0008 = 26.2552 Versus 26.2552 Mass Balance = -4.7684E-05 cm; Time step attempts = 2780 and successes = 2780 Evaporation: Potential = 0.1332 cm, Actual = 0.0333 cm Transpiration: Potential = 2.5308 cm, Actual = 0.0318 cm RHMEAN = 72.9 %; TMEAN = 299.5 K; HDRY = 4.3246E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 243, Simulated Time = 24.0000 hr _____ Node Number -----2 12 25

Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.12044 0.21379 0.25997 Head (cm)  $= 1.48629E+04 \ 3.40762E+03 \ 1.04653E+03$ Water Flow (cm) =-1.01430E-04-9.19646E-03 8.08629E-04 Plant Sink (cm) = 4.44550E-04 7.06226E-04 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 26.2552+ 0.0000+ 0.0000 - 0.0302- 0.0189- 0.0008 = 26.2053 Versus 26.2059 Mass Balance = -6.4850E-04 cm; Time step attempts = 7190 and successes = 7190 Evaporation: Potential = 0.1209 cm, Actual = 0.0302 cmTranspiration: Potential = 2.2962 cm, Actual = 0.0189 cm RHMEAN = 69.0 %; TMEAN = 298.4 K; HDRY = 5.0792E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 244, Simulated Time = 24.0000 hr Node Number = Depth (cm) = 2 12 25 5.00000 50.00000 122.00000 Depth (cm)=5.0000050.00000122.00000Water (cm3/cm3)=0.120200.213720.25988 Head (cm) = 1.50458E+04 3.41524E+03 1.04844E+03 Water Flow (cm) =-6.47687E-05-9.13138E-03 8.02716E-04 Plant Sink (cm) = 7.44189E-05 7.46753E-04 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 26.2059+ 0.0000+ 0.0000 - 0.0320- 0.0150- 0.0008 = 26.1581 Versus 26.1621 Mass Balance = -4.0817E-03 cm; Time step attempts = 34709 and successes = 34709Evaporation: Potential = 0.1280 cm, Actual = 0.0320 cm Transpiration: Potential = 2.4318 cm, Actual = 0.0150 cm RHMEAN = 71.6 %; TMEAN = 298.7 K; HDRY = 4.5761E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 245, Simulated Time = 24.0000 hr _____ 2 Node Number = Depth (cm) = = 12 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.11967 0.21365 0.25979 = 1.54730E+04 3.42380E+03 1.05050E+03 Head (cm) Water Flow (cm) =-6.06934E-05-9.08856E-03 7.98745E-04 Plant Sink (cm) = 0.00000E+00 7.50677E-04 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 26.1621+ 0.0000+ 0.0000 - 0.0209- 0.0130- 0.0008 = 26.1275 Versus 26.1320 Mass Balance = -4.4556E-03 cm; Time step attempts =54901 and successes =54901 Evaporation: Potential = 0.1289 cm, Actual = 0.0209 cm Transpiration: Potential = 2.4496 cm, Actual = 0.0130 cm RHMEAN = 70.8 %; TMEAN = 299.5 K; HDRY = 4.7233E+05 cm; DAYUBC = 4177 

DAILY SUMMARY: Day = 246, Simulated Time = 24.0000 hr ______ 25 Node Number == 2 12 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.118840.21365 0.25968 Head (cm) = 1.61698E+04 3.42393E+03 1.05282E+03Water Flow (cm) =-1.11686E-04-9.03736E-03 7.94074E-04 Plant Sink (cm) = 0.00000E+00 6.46717E-04 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 26.1320+ 0.0000+ 0.0000 - 0.0048- 0.0103- 0.0008 = 26.1161 Versus 26.1182 Mass Balance = -2.1648E-03 cm; Time step attempts =22485 and successes =22485 Evaporation: Potential = 0.1112 cm, Actual = 0.0048 cm Transpiration: Potential = 2.1133 cm, Actual = 0.0103 cm RHMEAN = 68.3 %; TMEAN = 300.4 K; HDRY = 5.2226E+05 cm; DAYUBC = 1821 DAILY SUMMARY: Day = 247, Simulated Time = 24.0000 hr Node Number = 2 12 25 Depth (cm) Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.11810 0.21381 0.25954 Head (cm)  $= 1.68287E+04 \ 3.40540E+03 \ 1.05600E+03$ Water Flow (cm) =-1.70108E-04-8.94336E-03 7.87128E-04 Plant Sink (cm) = 0.00000E+004.89497E-040.00000E+00PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 26.1182+ 0.0000+ 0.0000 - 0.0019- 0.0074- 0.0008 = 26.1081 Versus 26.1081 Mass Balance = -1.5259E-05 cm; Time step attempts = 343 and successes = 343 Evaporation: Potential = 0.0840 cm, Actual = 0.0019 cm Transpiration: Potential = 1.5952 cm, Actual = 0.0074 cm RHMEAN = 72.5 %; TMEAN = 299.5 K; HDRY = 4.4130E+05 cm; DAYUBC = 280 DAILY SUMMARY: Day = 248, Simulated Time = 24.0000 hr ____ Node Number == 2 12 25 Depth (cm) = 5.00000 Water (cm3/cm3) = 0.11811 5.00000 50.00000 122.00000 0.21409 0.25940 Head (cm)  $= 1.68214E+04 \ 3.37388E+03 \ 1.05898E+03$ Water Flow (cm) =-1.92894E-04-8.83432E-03 7.80442E-04 Plant Sink (cm) = 0.00000E+00 3.30429E-04 0.00000E+00PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 26.1081+ 0.0762+ 0.0000 - 0.0141- 0.0049- 0.0008 = 26.1646 Versus 26.1650 Mass Balance = -3.6430E-04 cm; Time step attempts =14974 and successes =14974 Evaporation: Potential = 0.0569 cm, Actual = 0.0141 cm Transpiration: Potential = 1.0806 cm, Actual = 0.0049 cm RHMEAN = 76.3 %; TMEAN = 299.8 K; HDRY = 3.6994E+05 cm; DAYUBC = 0

DAILY SUMMARY: Day = 249, Simulated Time = 24.0000 hr Node Number = 2 12 25 Depth (cm) 5.00000 = 50.00000 122.00000 Water (cm3/cm3) = 0.11814 0.21394 0.25927Head (cm)  $= 1.67958E+04 \ 3.39094E+03 \ 1.06184E+03$ Water Flow (cm) =-1.86042E-04-8.75010E-03 7.73868E-04 Plant Sink (cm) = 0.00000E+007.74293E-040.00000E+00PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 26.1650+ 0.0254+ 0.0000 - 0.0329- 0.0111- 0.0008 = 26.1455 Versus 26.1463 Mass Balance = -7.5340E-04 cm; Time step attempts =11155 and successes =11155 Evaporation: Potential = 0.1330 cm, Actual = 0.0329 cm Transpiration: Potential = 2.5267 cm, Actual = 0.0111 cm RHMEAN = 74.7 %; TMEAN = 299.8 K; HDRY = 4.0068E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 250, Simulated Time = 24.0000 hr _____ == 2 Node Number 12 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.17593 0.21374 0.25913 Head (cm)  $= 2.41981E+03 \ 3.41293E+03 \ 1.06480E+03$ Water Flow (cm) = 4.92613E-02-8.70931E-03 7.67562E-04 Plant Sink (cm) =  $5.76839E-02 \ 8.08762E-04 \ 0.00000E+00$ INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR PRESTOR STORAGE 26.1463+ 0.8326+ 0.2088 - 0.0346- 0.2877- 0.0008 = 26.6559 Versus 26.6563 Mass Balance = -4.4060E-04 cm; Time step attempts =11907 and successes =11907 Evaporation: Potential = 0.1411 cm, Actual = 0.0346 cmTranspiration: Potential = 2.6804 cm, Actual = 0.2877 cm RHMEAN = 77.2 %; TMEAN = 299.0 K; HDRY = 3.5476E+05 cm; DAYUBC = 2597 DAILY SUMMARY: Day = 251, Simulated Time = 24.0000 hr Node Number = 2 12 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.263460.21417 0.25902 Head (cm) = 5.57446E+02 3.36503E+03 1.06734E+03Water Flow (cm) = 1.24025E+00-8.62706E-03 7.61851E-04 Plant Sink (cm) = 1.30759E-02 1.84044E-04 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 26.6563+ 1.8634+ 0.3210 - 0.0078- 0.1140- 0.0008 = 28.3971 Versus 28.3987 Mass Balance = -1.6289E-03 cm; Time step attempts =21357 and successes =21357 Evaporation: Potential = 0.0323 cm, Actual = 0.0078 cm

Transpiration: Potential = 0.6138 cm, Actual = 0.1140 cm RHMEAN = 84.7 %; TMEAN = 294.0 K; HDRY = 2.2805E+05 cm; DAYUBC = 3055 DAILY SUMMARY: Day = 252, Simulated Time = 24.0000 hr _____ Node Number = 2 12 25 Depth (cm) Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.22181 0.21411 0.25888 Head (cm) = 1.02910E+03 3.37148E+03 1.07042E+03 Water Flow (cm) = 7.74022E-02-8.52945E-03 7.55446E-04 Plant Sink (cm) = 4.62991E-02 6.55184E-04 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 28.3987+ 0.0254+ 0.0000 - 0.0277- 0.4435- 0.0008 = 27.9521 Versus 27.9524 Mass Balance = -2.7466E-04 cm; Time step attempts = 7188 and successes = 7188Evaporation: Potential = 0.1121 cm, Actual = 0.0277 cm Transpiration: Potential = 2.1296 cm, Actual = 0.4435 cm RHMEAN = 69.7 %; TMEAN = 292.6 K; HDRY = 4.9471E+05 cm; DAYUBC = DAILY SUMMARY: Day = 253, Simulated Time = 24.0000 hr _____ Node Number === 2 12 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.188420.21411 0.25874 Head (cm) = 1.86072E+03 3.37160E+03 1.07359E+03Water Flow (cm) = 1.83273E-03-8.46499E-037.48669E-04Plant Sink (cm) = 4.23819E-025.99229E-040.00000E+00PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 27.9524+ 0.0000+ 0.0000 - 0.0254- 0.4101- 0.0007 = 27.5161 Versus 27.5161 Mass Balance = 7.6294E-06 cm; Time step attempts = 1348 and successes = 1348 Evaporation: Potential = 0.1016 cm, Actual = 0.0254 cmTranspiration: Potential = 1.9299 cm, Actual = 0.4101 cm RHMEAN = 67.4 %; TMEAN = 291.8 K; HDRY = 5.4068E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 254, Simulated Time = 24.0000 hr Node Number 12 25 = 2 Depth (cm) Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.15475 0.21407 0.25860 Head (cm)  $= 4.10576E+03 \ 3.37663E+03 \ 1.07676E+03$ Water Flow (cm) =-9.43086E-03-8.40070E-03 7.41999E-04 Plant Sink (cm) = 4.20743E-02 6.37418E-04 0.00000E+00PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 27.5161+ 0.0000+ 0.0000 - 0.0270- 0.4257- 0.0007 = 27.0626 Versus 27.0626

Mass Balance = 0.0000E+00 cm; Time step attempts = 1285 and successes = 1285 Evaporation: Potential = 0.1081 cm, Actual = 0.0270 cm Transpiration: Potential = 2.0544 cm, Actual = 0.4257 cm RHMEAN = 68.9 %; TMEAN = 293.2 K; HDRY = 5.1119E+05 cm; DAYUBC = Ω DAILY SUMMARY: Day = 255, Simulated Time = 24.0000 hr Node Number 2 12 25 == Depth (cm) = 5.00000 50.00000 122.00000Water (cm3/cm3) = 0.13684 0.21404 0.25845Head (cm) =  $7.29577E+03 \ 3.37931E+03 \ 1.07992E+03$ Water Flow (cm) =-6.05094E-03-8.33924E-03 7.35415E-04 Plant Sink (cm) = 2.28567E-026.10846E-040.00000E+00PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 27.0626+ 0.0000+ 0.0000 - 0.0259- 0.2952- 0.0007 = 26.7408 Versus 26.7408 Mass Balance = 9.5367E-06 cm; Time step attempts = 1293 and successes = 1293 Evaporation: Potential = 0.1037 cm, Actual = 0.0259 cm Transpiration: Potential = 1.9709 cm, Actual = 0.2952 cm RHMEAN = 67.8 %; TMEAN = 294.5 K; HDRY = 5.3329E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 256, Simulated Time = 24.0000 hr ______ Node Number ----2 12 25 Depth (cm) = 5.00000 50.00000 122.00000 Septem (cm)-5.0000050.00000122.00000Water (cm3/cm3)=0.128180.214020.25831Head (cm)  $= 1.03099E+04 \ 3.38186E+03 \ 1.08308E+03$ Water Flow (cm) =-2.49267E-03-8.27679E-03 7.28918E-04 Plant Sink (cm) =  $1.08594E-02 \ 6.03836E-04 \ 0.00000E+00$ PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 26.7408+ 0.0000+ 0.0000 - 0.0257- 0.1806- 0.0007 = 26.5339 Versus 26.5339 Mass Balance = -7.6294E-06 cm; Time step attempts = 1152 and successes = 1152Evaporation: Potential = 0.1026 cm, Actual = 0.0257 cm Transpiration: Potential = 1.9497 cm, Actual = 0.1806 cm RHMEAN = 69.5 %; TMEAN = 295.1 K; HDRY = 4.9862E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 257, Simulated Time = 24.0000 hr _____ Node Number = 2 12 25 Depth (cm) = 5.00000 50.00000 122.00000Water (cm3/cm3) = 0.12453 0.21408 0.25817= 1.21523E+04 3.37492E+03 1.08624E+03 Head (cm) Water Flow (cm) =-1.21440E-03-8.20796E-03 7.22500E-04

Plant Sink (cm) = 4.63772E-03 5.11266E-04 0.00000E+00

INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR PRESTOR STORAGE 26.5339+ 0.0000+ 0.0000 - 0.0217- 0.1015- 0.0007 = 26.4099 Versus 26.4099 Mass Balance = -3.8147E-06 cm; Time step attempts = 807 and successes = 807Evaporation: Potential = 0.0868 cm, Actual = 0.0217 cm Transpiration: Potential = 1.6497 cm, Actual = 0.1015 cm RHMEAN = 73.3 %; TMEAN = 296.5 K; HDRY = 4.2502E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 258, Simulated Time = 24.0000 hr ______ Node Number=21225Depth (cm)=5.0000050.00000122.00000Water (cm3/cm3)=0.196850.214400.25805 Head (cm)  $= 1.58158E+03 \ 3.34013E+03 \ 1.08912E+03$ Water Flow (cm) = 6.74070E - 02 - 8.11542E - 037.16676E - 04Plant Sink (cm) =  $1.64492E-02 \ 2.35027E-04 \ 0.00000E+00$ PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 26.4099+ 0.6604+ 0.0000 - 0.0099- 0.1096- 0.0007 = 26.9501 Versus 26.9506 Mass Balance = -4.6539E-04 cm; Time step attempts =10030 and successes =10030 Evaporation: Potential = 0.0401 cm, Actual = 0.0099 cm Transpiration: Potential = 0.7617 cm, Actual = 0.1096 cm RHMEAN = 83.4 %; TMEAN = 294.8 K; HDRY = 2.4902E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 259, Simulated Time = 24.0000 hr _____ Node Number = Depth (cm) = 2 12 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.21961 0.21481 0.25791 Head (cm) = 1.06619E+03 3.29538E+03 1.09214E+03Water Flow (cm) = 2.68664E - 01 - 7.99101E - 037.10696E - 04Plant Sink (cm) = 8.64297E-03 1.24041E-04 0.00000E+00 INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR PRESTOR STORAGE 26.9506+ 0.4572+ 0.0000 - 0.0052- 0.0698- 0.0007 = 27.3321 Versus 27.3323 Mass Balance = -2.5177E-04 cm; Time step attempts = 7190 and successes = 7190 Evaporation: Potential = 0.0209 cm, Actual = 0.0052 cmTranspiration: Potential = 0.3975 cm, Actual = 0.0698 cm RHMEAN = 87.7 %; TMEAN = 295.7 K; HDRY = 1.8050E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 260, Simulated Time = 24.0000 hr _ _ _ _ _ _ _ _ _ _ _ _ _ _ _ _ _ _ Node Number = 2 12 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.19338 0.21496 0.25778

Head (cm) =  $1.68899E+03 \ 3.27926E+03 \ 1.09517E+03$ Water Flow (cm) =  $7.22733E-02-7.88419E-03 \ 7.04741E-04$ Plant Sink (cm) = 2.55137E-02 3.69254E-04 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 27.3323+ 0.0254+ 0.0000 - 0.0153- 0.2226- 0.0007 = 27.1191 Versus 27.1193 Mass Balance = -2.1553E-04 cm; Time step attempts = 5614 and successes = 5614Evaporation: Potential = 0.0618 cm, Actual = 0.0153 cm Transpiration: Potential = 1.1735 cm, Actual = 0.2226 cm RHMEAN = 79.8 %; TMEAN = 298.2 K; HDRY = 3.0846E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 261, Simulated Time = 24.0000 hr _____ Node Number = Depth (cm) = 2 12 25 5.00000 50.00000 122.00000 = 0.28918 0.21520 0.25766 Water (cm3/cm3) = 3.92199E+02 3.25382E+03 1.09794E+03 Head (cm) Water Flow (cm) = 1.72137E+00-7.79851E-03 6.99282E-04 Plant Sink (cm) = 1.87060E-02 2.72369E-04 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 27.1193+ 2.4629+ 0.1787 - 0.0112- 0.1834- 0.0007 = 29.3869 Versus 29.3877 Mass Balance = -8.2779E-04 cm; Time step attempts =10595 and successes =10595 Evaporation: Potential = 0.0462 cm, Actual = 0.0112 cm Transpiration: Potential = 0.8781 cm, Actual = 0.1834 cm RHMEAN = 88.2 %; TMEAN = 295.1 K; HDRY = 1.7238E+05 cm; DAYUBC = 847 DAILY SUMMARY: Day = 262, Simulated Time = 24.0000 hr _____ Node Number Depth (cm) 2 == 12 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.25159 0.21508 0.25752 = 6.58346E+02 3.26591E+03 1.10109E+03 Head (cm) Water Flow (cm) = 4.59067E - 02 - 7.73663E - 03 6.93290E - 04Plant Sink (cm) = 4.25158E-02 6.20037E-04 0.00000E+00PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 29.3877+ 0.0000+ 0.0000 - 0.0255- 0.4340- 0.0007 = 28.9275 Versus 28.9275 Mass Balance = -9.5367E-06 cm; Time step attempts = 2868 and successes = 2868 Evaporation: Potential = 0.1019 cm, Actual = 0.0255 cm Transpiration: Potential = 1.9360 cm, Actual = 0.4340 cm RHMEAN = 70.0 %; TMEAN = 295.4 K; HDRY = 4.8833E+05 cm; DAYUBC = 0 

DAILY SUMMARY: Day = 263, Simulated Time = 24.0000 hr

Node Number = 2 12 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.24102 0.21539 0.25739 Head (cm) = 7.67073E+02 3.23380E+03 1.10424E+03Water Flow (cm) = 7.89087E-03-7.66557E-03 6.87305E-04Plant Sink (cm) = 1.31741E-02 1.92687E-04 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 28.9275+ 0.0000+ 0.0000 - 0.0079- 0.1376- 0.0007 = 28.7814 Versus 28.7814 Mass Balance = 5.7220E-06 cm; Time step attempts = 1355 and successes = 1355 Evaporation: Potential = 0.0316 cm, Actual = 0.0079 cm Transpiration: Potential = 0.5999 cm, Actual = 0.1376 cm RHMEAN = 86.0 %; TMEAN = 292.9 K; HDRY = 2.0731E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 264, Simulated Time = 24.0000 hr ______ Node Number = 2 12 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.22480 0.21554 0.25725 Head (cm)  $= 9.81433E+02 \ 3.21796E+03 \ 1.10738E+03$ Water Flow (cm) =-3.92876E-03-7.57030E-03 6.81388E-04 Plant Sink (cm) = 2.25485E-02 3.32018E-04 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 28.7814+ 0.0000+ 0.0000 - 0.0135- 0.2387- 0.0007 = 28.5285 Versus 28.5285 Mass Balance = 1.9073E-06 cm; Time step attempts = 600 and successes = 600 Evaporation: Potential = 0.0540 cm, Actual = 0.0135 cm Transpiration: Potential = 1.0268 cm, Actual = 0.2387 cm RHMEAN = 83.9 %; TMEAN = 294.8 K; HDRY = 2.4002E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 265, Simulated Time = 24.0000 hr 2 = Node Number 12 25 Node Namber21225Depth (cm)=5.0000050.00000122.00000Water (cm3/cm3)=0.204290.215580.25712Head (cm) = 1.38147E+03 3.21402E+03 1.11051E+03Water Flow (cm) =-1.44528E-02-7.50332E-03 6.75554E-04 Plant Sink (cm) = 2.96050E-02 4.37132E-04 0.00000E+00 DRAIN NEWSTOR PRESTOR INFIL RUNOFF EVAPO TRANS STORAGE 28.5285+ 0.0000+ 0.0000 - 0.0177- 0.3140- 0.0007 = 28.1961 Versus 28.1961 Mass Balance = -3.8147E-06 cm; Time step attempts = 696 and successes = 696 Evaporation: Potential = 0.0710 cm, Actual = 0.0177 cmTranspiration: Potential = 1.3481 cm, Actual = 0.3140 cm RHMEAN = 81.6 %; TMEAN = 295.7 K; HDRY = 2.7872E+05 cm; DAYUBC = 0

DAILY SUMMARY: Day = 266, Simulated Time = 24.0000 hr _____ Node Number 200 2 12 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.16827 0.21531 0.25699Head (cm) = 2.88870E+03 3.24226E+03 1.11353E+03 Water Flow (cm) =-1.61223E-02-7.48181E-03 6.69998E-04 Plant Sink (cm) = 4.99665E-027.35141E-040.00000E+00PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 28.1961+ 0.0254+ 0.0000 - 0.0299- 0.5309- 0.0007 = 27.6600 Versus 27.6602 Mass Balance = -1.3733E-04 cm; Time step attempts = 5934 and successes = 5934 Evaporation: Potential = 0.1210 cm, Actual = 0.0299 cm Transpiration: Potential = 2.2983 cm, Actual = 0.5309 cm RHMEAN = 72.8 %; TMEAN = 294.8 K; HDRY = 4.3533E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 267, Simulated Time = 24.0000 hr _____ 2 Node Number = 12 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.14214 0.21509 0.25686 Head (cm) = 6.05430E+03 3.26516E+03 1.11665E+03 Water Flow (cm) =-1.22742E-02-7.48074E-03 6.64304E-04 Plant Sink (cm) = 3.40462E-02 6.96039E-04 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 27.6602+ 0.0000+ 0.0000 - 0.0285- 0.4372- 0.0007 = 27.1938 Versus 27.1938 Mass Balance = 7.6294E-06 cm; Time step attempts = 1382 and successes = 1382 Evaporation: Potential = 0.1142 cm, Actual = 0.0285 cm Transpiration: Potential = 2.1696 cm, Actual = 0.4372 cm RHMEAN = 66.9 %; TMEAN = 288.4 K; HDRY = 5.5071E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 268, Simulated Time = 24.0000 hr _____ Node Number = Depth (cm) = 2 12 25 5.00000 50.00000 122.00000 = 0.13312 0.21507 0.25672 Water (cm3/cm3) = 8.40823E+03 3.26740E+03 1.11976E+03 Head (cm) Water Flow (cm) =-5.05599E-03-7.45269E-03 6.58695E-04 Plant Sink (cm) = 1.26309E-02 4.99941E-04 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 27.1938+ 0.0000+ 0.0000 - 0.0206- 0.2313- 0.0007 = 26.9413 Versus 26.9413 Mass Balance = -7.6294E-06 cm; Time step attempts = 1015 and successes = 1015 Evaporation: Potential = 0.0823 cm, Actual = 0.0206 cm Transpiration: Potential = 1.5638 cm, Actual = 0.2313 cm RHMEAN = 67.5 %; TMEAN = 286.8 K; HDRY = 5.3856E+05 cm; DAYUBC = 0

DAILY SUMMARY: Day = 269, Simulated Time = 24.0000 hr _____ Node Number = 2 12 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.127910.21507 0.25659 = 1.04317E+04 3.26700E+03 1.12287E+03 Head (cm) Water Flow (cm) =-2.56791E-03-7.40284E-03 6.53160E-04 Plant Sink (cm) = 7.01299E-03 4.70178E-04 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 26.9413+ 0.0000+ 0.0000 - 0.0194- 0.1656- 0.0007 = 26.7556 Versus 26.7556 Mass Balance = 7.6294E-06 cm; Time step attempts = 789 and successes = 789 Evaporation: Potential = 0.0774 cm, Actual = 0.0194 cm Transpiration: Potential = 1.4711 cm, Actual = 0.1656 cm RHMEAN = 71.0 %; TMEAN = 287.9 K; HDRY = 4.7031E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 270, Simulated Time = 24.0000 hr ______ 2 Node Number ----12 25 Depth (cm) = 5.00000 Water (cm3/cm3) = 0.12420 50.00000 122.00000 0.21499 0.25646 = 1.23466E+04 3.27571E+03 1.12597E+03Head (cm) Water Flow (cm) =-1.40853E-03-7.35974E-03 6.47696E-04 Plant Sink (cm) = 4.59874E-03 5.52170E-04 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 26.7556+ 0.0000+ 0.0000 - 0.0228- 0.1450- 0.0006 = 26.5872 Versus 26.5872 Mass Balance = -1.5259E-05 cm; Time step attempts = 905 and successes = 905 Evaporation: Potential = 0.0910 cm, Actual = 0.0228 cm Transpiration: Potential = 1.7297 cm, Actual = 0.1450 cm RHMEAN = 67.3 %; TMEAN = 290.4 K; HDRY = 5.4202E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 271, Simulated Time = 24.0000 hr _____ Node Number = 12 25 2 Depth (cm) 5.00000 50.00000 122.00000 = Depend (Cm)=5.0000050.00000Water (cm3/cm3)=0.122320.214940.25633 = 1.35193E+04 3.28131E+03 1.12907E+03 Head (cm) Water Flow (cm) =-8.06612E-04-7.32216E-03 6.42297E-04 Plant Sink (cm) = 2.25236E-03 5.19779E-04 0.00000E+00 INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR PRESTOR STORAGE 26.5872+ 0.0000+ 0.0000 - 0.0215- 0.1027- 0.0006 = 26.4624 Versus 26.4624 Mass Balance = 7.6294E-06 cm; Time step attempts = 1508 and successes = 1508

Evaporation: Potential = 0.0859 cm, Actual = 0.0215 cm Transpiration: Potential = 1.6314 cm, Actual = 0.1027 cm RHMEAN = 68.5 %; TMEAN = 291.5 K; HDRY = 5.1815E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 272, Simulated Time = 24.0000 hr _____ 2 12 Node Number -25 Depth (cm) Depth (cm) = 5.00000 50.0000 122.00000 Water (cm3/cm3) = 0.12119 0.21484 0.25620 Head (cm)  $= 1.42996E+04 \ 3.29240E+03 \ 1.13212E+03$ Water Flow (cm) =-4.95427E-04-7.28673E-03 6.36994E-04 Plant Sink (cm) = 1.22339E-03 5.69366E-04 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 26.4624+ 0.0000+ 0.0000 - 0.0236- 0.0869- 0.0006 = 26.3513 Versus 26.3513 Mass Balance = -6.2943E-05 cm; Time step attempts = 2940 and successes = 2940 Evaporation: Potential = 0.0943 cm, Actual = 0.0236 cm Transpiration: Potential = 1.7913 cm, Actual = 0.0869 cm RHMEAN = 67.5 %; TMEAN = 290.1 K; HDRY = 5.3845E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 273, Simulated Time = 24.0000 hr _____ Node Number 2 12 == 25 Depth (cm) = Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.12068 0.21482 0.25608 Head (cm)  $= 1.46763E+04 \ 3.29394E+03 \ 1.13504E+03$ Water Flow (cm) =-3.23358E-04-7.24828E-03 6.31892E-04 Plant Sink (cm) = 4.72221E-04 4.77880E-04 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 26.3513+ 0.0000+ 0.0000 - 0.0198- 0.0582- 0.0006 = 26.2727 Versus 26.2730 Mass Balance = -3.2616E-04 cm; Time step attempts = 4730 and successes = 4730 Evaporation: Potential = 0.0793 cm, Actual = 0.0198 cm Transpiration: Potential = 1.5061 cm, Actual = 0.0582 cm RHMEAN = 67.3 %; TMEAN = 289.3 K; HDRY = 5.4213E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 274, Simulated Time = 24.0000 hr _____ Node Number 2 12 25 -----Depth (cm) Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.12038 0.21481 0.25597 = 1.49050E+04 3.29553E+03 1.13764E+03 Head (cm) Water Flow (cm) =-2.29595E-04-7.20556E-03 6.27237E-04 Plant Sink (cm) = 1.89590E-04 4.73501E-04 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE

26.2730+ 0.0000+ 0.0000 - 0.0196- 0.0479- 0.0006 = 26.2049 Versus 26.2061 Mass Balance = -1.1978E-03 cm; Time step attempts =10498 and successes =10498 Evaporation: Potential = 0.0786 cm, Actual = 0.0196 cm Transpiration: Potential = 1.4929 cm, Actual = 0.0479 cm RHMEAN = 66.0 %; TMEAN = 289.8 K; HDRY = 5.6861E+05 cm; DAYUBC = Ω DAILY SUMMARY: Day = 275, Simulated Time = 24.0000 hr 2 = 12 Node Number 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.12022 0.21459 0.25588 Head (cm)  $= 1.50302E+04 \ 3.31930E+03 \ 1.13983E+03$ Water Flow (cm) =-1.54711E-04-7.19119E-03 6.23356E-04 Plant Sink (cm) = 4.98076E-05 6.58153E-04 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 26.2061+ 0.0254+ 0.0000 - 0.0274- 0.0548- 0.0006 = 26.1486 Versus 26.1505 Mass Balance = -1.8692E-03 cm; Time step attempts =23331 and successes =23331 Evaporation: Potential = 0.1107 cm, Actual = 0.0274 cmTranspiration: Potential = 2.1040 cm, Actual = 0.0548 cm RHMEAN = 65.1 %; TMEAN = 290.9 K; HDRY = 5.8744E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 276, Simulated Time = 24.0000 hr _____ Node Number = Depth (cm) = 2 12 25 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.11988 0.21429 0.25584Head (cm) = 1.53036E+04 3.35142E+03 1.14079E+03Water Flow (cm) =-1.02252E-04-7.20556E-03 6.20081E-04 Plant Sink (cm) = 0.00000E+00 7.69880E-04 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 26.1505+ 0.0000+ 0.0000 - 0.0323- 0.0521- 0.0006 = 26.0655 Versus 26.0718 Mass Balance = -6.3267E-03 cm; Time step attempts =91164 and successes =91164 Evaporation: Potential = 0.1292 cm, Actual = 0.0323 cm Transpiration: Potential = 2.4553 cm, Actual = 0.0521 cm RHMEAN = 63.0 %; TMEAN = 294.0 K; HDRY = 6.3405E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 277, Simulated Time = 24.0000 hr _____ == 2 Node Number 12 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.11917 0.21410 0.25571 Head (cm) = 1.58874E+04 3.37330E+03 1.14392E+03 Water Flow (cm) =-1.18427E-04-7.22341E-03 6.17282E-04

Plant Sink (cm) =  $0.00000E+00 \ 6.69604E-04 \ 0.00000E+00$ PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 26.0718+ 0.0000+ 0.0000 - 0.0032- 0.0377- 0.0006 = 26.0303 Versus 26.0306 Mass Balance = -2.6131E-04 cm; Time step attempts = 2378 and successes = 2378Evaporation: Potential = 0.1132 cm, Actual = 0.0032 cm Transpiration: Potential = 2.1512 cm, Actual = 0.0377 cm RHMEAN = 73.2 %; TMEAN = 295.9 K; HDRY = 4.2692E+05 cm; DAYUBC = 1774 DAILY SUMMARY: Day = 278, Simulated Time = 24.0000 hr -----------2 Node Number 12 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.12283 0.21427 0.25559Head (cm) = 1.31833E+04 3.35371E+03 1.14685E+03 = 5.77165E-05-7.17487E-03 6.12411E-04Water Flow (cm) Plant Sink (cm) = 4.23530E-04 2.76158E-04 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 26.0306+ 0.2286+ 0.0000 - 0.0117- 0.0158- 0.0006 = 26.2311 Versus 26.2313 Mass Balance = -2.1362E-04 cm; Time step attempts = 8482 and successes = 8482Evaporation: Potential = 0.0472 cm, Actual = 0.0117 cm Transpiration: Potential = 0.8965 cm, Actual = 0.0158 cm RHMEAN = 79.2 %; TMEAN = 286.2 K; HDRY = 3.2031E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 279, Simulated Time = 24.0000 hr ______ = 12 Node Number 2 25 Depth (cm) Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.12352 0.21433 0.25547 = 1.27478E+04 3.34691E+03 1.14992E+03 Head (cm) = 2.97047E-04-7.09983E-03 6.07438E-04 Water Flow (cm) Plant Sink (cm) = 1.85303E-03 3.95196E-04 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 26.2313+ 0.0000+ 0.0000 - 0.0166- 0.0268- 0.0006 = 26.1873 Versus 26.1873 Mass Balance = -3.8147E-06 cm; Time step attempts = 557 and successes = 557 Evaporation: Potential = 0.0666 cm, Actual = 0.0166 cm Transpiration: Potential = 1.2654 cm, Actual = 0.0268 cm RHMEAN = 63.6 %; TMEAN = 283.4 K; HDRY = 6.1977E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 280, Simulated Time = 24.0000 hr Node Number 25 2 12 = = 5.00000 50.00000 122.00000 Depth (cm)

Water (cm3/cm3) = 0.12263 0.21427 0.25534Head (cm)  $= 1.33099E+04 \ 3.35387E+03 \ 1.15298E+03$ Water Flow (cm) = 2.77892E-04-7.04999E-03 6.02533E-04Plant Sink (cm) = 2.23228E-035.15127E-040.00000E+00PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 26.1873+ 0.0000+ 0.0000 - 0.0217- 0.0321- 0.0006 = 26.1329 Versus 26.1329 Mass Balance = 1.9073E-06 cm; Time step attempts = 548 and successes = 548 Evaporation: Potential = 0.0868 cm, Actual = 0.0217 cm Transpiration: Potential = 1.6494 cm, Actual = 0.0321 cm RHMEAN = 65.1 %; TMEAN = 284.5 K; HDRY = 5.8882E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 281, Simulated Time = 24.0000 hr == Node Number 2 12 25 Depth (cm) = Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.12118 0.21390 0.25522 Head (cm) = 1.43123E+04 3.39478E+03 1.15602E+03 Water Flow (cm) = 1.45588E - 04 - 7.04065E - 035.97691E - 04Plant Sink (cm) = 1.97517E-03 8.22260E-04 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 26.1329+ 0.0000+ 0.0000 - 0.0349- 0.0400- 0.0006 = 26.0574 Versus 26.0574 Mass Balance = 5.7220E-06 cm; Time step attempts = 1612 and successes = 1612 Evaporation: Potential = 0.1395 cm, Actual = 0.0349 cm Transpiration: Potential = 2.6505 cm, Actual = 0.0400 cmRHMEAN = 64.8 %; TMEAN = 289.5 K; HDRY = 5.9414E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 282, Simulated Time = 24.0000 hr 2 = Node Number 12 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.22088 0.21395 0.25511 Head (cm) = 1.04458E+03 3.39013E+03 1.15859E+03Water Flow (cm) = 3.53213E-01-7.02707E-03 5.93602E-04 Plant Sink (cm) = 2.80250E-02 3.93894E-04 0.00000E+00 INFIL RUNOFF DRAIN NEWSTOR EVAPO TRANS PRESTOR STORAGE 26.0574+ 1.1959+ 0.3027 - 0.0168- 0.1972- 0.0006 = 27.0387 Versus 27.0395 Mass Balance = -7.9346E-04 cm; Time step attempts =16179 and successes =16179 Evaporation: Potential = 0.0685 cm, Actual = 0.0168 cm Transpiration: Potential = 1.3022 cm, Actual = 0.1972 cm RHMEAN = 76.1 %; TMEAN = 294.0 K; HDRY = 3.7383E+05 cm; DAYUBC = 4489 

DAILY SUMMARY: Day = 283, Simulated Time = 24.0000 hr

_____ 2 Node Number 12 25 = Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.33227 0.21436 0.25502 Head (cm)  $= 2.10928E+02 \ 3.34447E+03 \ 1.16077E+03$ Water Flow (cm) = 3.04354E+00-6.94605E-035.89996E-04Plant Sink (cm) = 1.14026E-03 1.61436E-05 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 27.0395+ 3.8036+ 1.6320 - 0.0007- 0.0118- 0.0006 = 30.8300 Versus 30.8320 Mass Balance = -1.9608E-03 cm; Time step attempts =25334 and successes =25334 Evaporation: Potential = 0.0029 cm, Actual = 0.0007 cm Transpiration: Potential = 0.0552 cm, Actual = 0.0118 cm RHMEAN = 87.4 %; TMEAN = 292.9 K; HDRY = 1.8457E+05 cm; DAYUBC = 12324 DAILY SUMMARY: Day = 284, Simulated Time = 24.0000 hr Node Number = Depth (cm) = 2 12 25 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.32837 0.21477 0.25491Head (cm) = 2.24221E+02 3.29925E+03 1.16371E+03Water Flow (cm) = 6.37511E-01-6.82409E-03 5.85727E-04Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 30.8320+ 0.6096+ 0.0000 - 0.0000- 0.0000- 0.0006 = 31.4410 Versus 31.4412 Mass Balance = -1.7166E-04 cm; Time step attempts = 8441 and successes = 8441 Evaporation: Potential = 0.0000 cm, Actual = 0.0000 cmTranspiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 97.3 %; TMEAN = 288.4 K; HDRY = 3.7054E+04 cm; DAYUBC = 0 DAILY SUMMARY: Day = 285, Simulated Time = 24.0000 hr = 2 Node Number 12 25 Note Namber21223Depth (cm)=5.0000050.00000122.00000Water (cm3/cm3)=0.336710.214820.25479Head (cm)  $= 1.96303E+02 \ 3.29406E+03 \ 1.16659E+03$ Water Flow (cm) = 1.12043E+00-6.73712E-035.81285E-04Plant Sink (cm) = 2.45498E-02 3.54371E-04 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 31.4412+ 1.3033+ 0.1445 - 0.0147- 0.2717- 0.0006 = 32.4574 Versus 32.4578 Mass Balance = -3.5095E-04 cm; Time step attempts = 8885 and successes = 8885 Evaporation: Potential = 0.0600 cm, Actual = 0.0147 cm Transpiration: Potential = 1.1407 cm, Actual = 0.2717 cmRHMEAN = 82.4 %; TMEAN = 291.2 K; HDRY = 2.6565E+05 cm; DAYUBC = 943 

DAILY SUMMARY: Day = 286, Simulated Time = 24.0000 hr _____ Node Number ----2 12 25 Depth (cm) = 5.00000 50.00000 122.00000 = 0.31594 Water (cm3/cm3) 0.21505 0.25467 = 2.69982E+02 3.26946E+03 1.16961E+03Head (cm) = 6.45011E-02-6.67134E-03 5.76739E-04 Water Flow (cm) Plant Sink (cm) = 1.57692E-02 2.28436E-04 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 32.4578+ 0.0000+ 0.0000 - 0.0094- 0.1770- 0.0006 = 32.2708 Versus 32.2708 Mass Balance = -3.8147E-06 cm; Time step attempts = 2227 and successes = 2227 Evaporation: Potential = 0.0378 cm, Actual = 0.0094 cm Transpiration: Potential = 0.7181 cm, Actual = 0.1770 cm RHMEAN = 79.9 %; TMEAN = 287.6 K; HDRY = 3.0708E+05 cm; DAYUBC = Ο DAILY SUMMARY: Day = 287, Simulated Time = 24.0000 hr Node Number 2 12 25 = Depth (cm) _ 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.294440.21592 0.25455 Head (cm) = 3.64962E+02 3.17887E+03 1.17263E+03 =-1.57025E-03-6.54331E-03 5.72214E-04 Water Flow (cm) Plant Sink (cm) = 2.97640E-024.36184E-040.00000E+00INFIL RUNOFF TRANS PRESTOR EVAPO DRAIN NEWSTOR STORAGE 32.2708+ 0.0000+ 0.0000 - 0.0178- 0.3385- 0.0006 = 31.9139 Versus 31.9139 Mass Balance = 5.7220E-06 cm; Time step attempts = 836 and successes = 836 Evaporation: Potential = 0.0713 cm, Actual = 0.0178 cm Transpiration: Potential = 1.3553 cm, Actual = 0.3385 cm RHMEAN = 64.0 %; TMEAN = 287.0 K; HDRY = 6.1142E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 288, Simulated Time = 24.0000 hr Node Number 2 12 25 = Depth (cm) = 5.00000 50.00000 122.00000 = 0.27515 Water (cm3/cm3) 0.21878 0.25443 = 4.74774E+02 2.90488E+03 1.17563E+03 Head (cm) Water Flow (cm) =-2.59688E-02-6.09023E-03 5.67754E-04 Plant Sink (cm) = 3.15418E-024.86527E-040.00000E+00PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 31.9139+ 0.0000+ 0.0000 - 0.0189- 0.3590- 0.0006 = 31.5354 Versus 31.5354 Mass Balance = -5.7220E-06 cm; Time step attempts = 96 and successes = 96 Evaporation: Potential = 0.0756 cm, Actual = 0.0189 cm Transpiration: Potential = 1.4363 cm, Actual = 0.3590 cm

RHMEAN = 68.6 %; TMEAN = 285.9 K; HDRY = 5.1747E+05 cm; DAYUBC = (

DAILY SUMMARY: Day = 289, Simulated Time = 24.0000 hr == Node Number 2 12 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.26003 0.22325 0.25431 Head (cm) = 5.84628E+02 2.54114E+03 1.17863E+03Water Flow (cm) =-3.04165E-02-5.02210E-03 5.63354E-04 Plant Sink (cm) = 2.66735E-02 4.19091E-04 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 31.5354+ 0.0000+ 0.0000 - 0.0160- 0.3037- 0.0006 = 31.2152 Versus 31.2152 Mass Balance = 0.0000E+00 cm; Time step attempts = 96 and successes = 96 Evaporation: Potential = 0.0639 cm, Actual = 0.0160 cm Transpiration: Potential = 1.2146 cm, Actual = 0.3037 cm RHMEAN = 60.6 %; TMEAN = 282.3 K; HDRY = 6.8753E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 290, Simulated Time = 24.0000 hr ______ 2 12 Node Number = 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.23568 0.22780 0.25419 Head (cm) = 8.30354E+02 2.23416E+03 1.18162E+03Water Flow (cm) =-4.50130E-02-3.39306E-03 5.59008E-04 Plant Sink (cm) = 4.18424E-026.57422E-040.00000E+00INFIL RUNOFF PRESTOR EVAPO TRANS DRAIN NEWSTOR STORAGE 31.2152+ 0.0000+ 0.0000 - 0.0251- 0.4763- 0.0006 = 30.7132 Versus 30.7133 Mass Balance = -1.3351E-05 cm; Time step attempts = 96 and successes = 96 Evaporation: Potential = 0.1003 cm, Actual = 0.0251 cmTranspiration: Potential = 1.9054 cm, Actual = 0.4763 cm RHMEAN = 57.2 %; TMEAN = 284.8 K; HDRY = 7.6505E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 291, Simulated Time = 24.0000 hr Node Number = 2 12 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.19529 0.23146 0.25407 Depth (cm) Head (cm) = 1.62867E+03 2.02381E+03 1.18460E+03Water Flow (cm) =-5.34549E-02-1.61034E-03 5.54714E-04 Plant Sink (cm) = 6.16202E-02 9.68167E-04 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 30.7133+ 0.0000+ 0.0000 - 0.0369- 0.7015- 0.0006 = 29.9743 Versus 29.9743

Mass Balance = -1.9073E-06 cm; Time step attempts = 396 and successes = 396 Evaporation: Potential = 0.1477 cm, Actual = 0.0369 cmTranspiration: Potential = 2.8060 cm, Actual = 0.7015 cmRHMEAN = 56.2 %; TMEAN = 290.9 K; HDRY = 7.9086E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 292, Simulated Time = 24.0000 hr ______ 2 Node Number = 12 25 Depth (cm) Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.20011 0.23521 0.25396 = 1.48932E+03 1.83664E+03 1.18758E+03 Head (cm) Water Flow (cm) =-3.81245E-02 2.24727E-04 5.50471E-04 Plant Sink (cm) = 5.69596E-03 8.94941E-05 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 29.9743+ 0.0000+ 0.0000 - 0.0034- 0.0648- 0.0006 = 29.9055 Versus 29.9055 Mass Balance = -1.9073E-06 cm; Time step attempts = 96 and successes = 96 Evaporation: Potential = 0.0137 cm, Actual = 0.0034 cm Transpiration: Potential = 0.2594 cm, Actual = 0.0648 cm RHMEAN = 85.6 %; TMEAN = 287.0 K; HDRY = 2.1259E+05 cm; DAYUBC = Ω DAILY SUMMARY: Day = 293, Simulated Time = 24.0000 hr _____ Node Number = Depth (cm) = 2 12 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.18025 0.23782 0.25384 Head (cm) = 2.20229E+03 1.72056E+03 1.19055E+03Water Flow (cm) =-3.27070E-02 1.96532E-03 5.46279E-04 Plant Sink (cm) = 2.97476E-02 4.67389E-04 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 29.9055+ 0.0000+ 0.0000 - 0.0178- 0.3386- 0.0005 = 29.5485 Versus 29.5485 Mass Balance = 0.0000E+00 cm; Time step attempts = 378 and successes = 378 Evaporation: Potential = 0.0713 cm, Actual = 0.0178 cm Transpiration: Potential = 1.3546 cm, Actual = 0.3386 cm RHMEAN = 77.8 %; TMEAN = 291.2 K; HDRY = 3.4484E+05 cm; DAYUBC = 0 _____ DAILY SUMMARY: Day = 294, Simulated Time = 24.0000 hr -----Node Number Depth (cm) 2 12 2.5 = Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.14981 0.23939 0.25372 Head (cm) = 4.74164E+03 1.65550E+03 1.19352E+03 Water Flow (cm) =-2.37145E-02 3.06481E-03 5.42135E-04 Plant Sink (cm) = 3.94784E-02 7.16616E-04 0.00000E+00

PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 29.5485+ 0.0000+ 0.0000 - 0.0273- 0.4916- 0.0005 = 29.0290 Versus 29.0290 Mass Balance = 0.0000E+00 cm; Time step attempts = 1045 and successes = 1045 Evaporation: Potential = 0.1093 cm, Actual = 0.0273 cm Transpiration: Potential = 2.0769 cm, Actual = 0.4916 cm RHMEAN = 70.5 %; TMEAN = 294.3 K; HDRY = 4.7883E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 295, Simulated Time = 24.0000 hr Node Number = 2 12 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.14107 0.24063 0.25361 Head (cm) = 6.27907E+03 1.60677E+03 1.19648E+03Water Flow (cm) =-1.27387E-02 3.68624E-03 5.38041E-04Plant Sink (cm) =  $1.42262E-02 \ 4.13576E-04 \ 0.00000E+00$ PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 29.0290+ 0.0000+ 0.0000 - 0.0158- 0.2392- 0.0005 = 28.7735 Versus 28.7735 Mass Balance = -1.9073E-06 cm; Time step attempts = 789 and successes = 789 Evaporation: Potential = 0.0631 cm, Actual = 0.0158 cm Transpiration: Potential = 1.1986 cm, Actual = 0.2392 cmRHMEAN = 81.6 %; TMEAN = 293.7 K; HDRY = 2.7855E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 296, Simulated Time = 24.0000 hr _____ 12 = 2 Node Number = Depth (cm) = Node Number 25 Depth (cm) = 5.00000 50.00000 122.00000Water (cm3/cm3) = 0.13085 0.24116 0.25350Head (cm) = 9.20952E+03 1.58639E+03 1.19943E+03Water Flow (cm) =-6.61481E-03 3.92143E-03 5.33997E-04 Plant Sink (cm) = 1.30782E-02 6.26938E-04 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 28.7735+ 0.0000+ 0.0000 - 0.0239- 0.2989- 0.0005 = 28.4501 Versus 28.4501 Mass Balance = -1.9073E-06 cm; Time step attempts = 1117 and successes = 1117 Evaporation: Potential = 0.0956 cm, Actual = 0.0239 cm Transpiration: Potential = 1.8170 cm, Actual = 0.2989 cm RHMEAN = 71.4 %; TMEAN = 295.1 K; HDRY = 4.6185E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 297, Simulated Time = 24.0000 hr ------2 Node Number = 12 25 Note Humber-21223Depth (cm)=5.0000050.00000122.00000Water (cm3/cm3)=0.125220.241260.25338Head (cm)=1.17713E+041.58269E+031.20237E+03

Water Flow (cm) =-2.94707E-03 3.70295E-03 5.30003E-04 Plant Sink (cm) = 6.90742E-03 6.74515E-04 0.00000E+00 DRAIN NEWSTOR PRESTOR INFIL RUNOFF EVAPO TRANS STORAGE 28.4501+ 0.0000+ 0.0000 - 0.0257- 0.2520- 0.0005 = 28.1718 Versus 28.1718 Mass Balance = -1.9073E-06 cm; Time step attempts = 1094 and successes = 1094Evaporation: Potential = 0.1029 cm, Actual = 0.0257 cmTranspiration: Potential = 1.9549 cm, Actual = 0.2520 cm RHMEAN = 65.4 %; TMEAN = 287.0 K; HDRY = 5.8172E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 298, Simulated Time = 24.0000 hr ______ Node Number == 2 12 25 Node Number=21225Depth (cm)=5.0000050.00000122.00000Water (cm3/cm3)=0.122990.241220.25327 Head (cm)  $= 1.30839E+04 \ 1.58432E+03 \ 1.20531E+03$ Water Flow (cm) =-1.51583E-03 3.27594E-03 5.26059E-04 Plant Sink (cm) = 2.77569E-03 5.38985E-04 0.00000E+00 INFIL RUNOFF EVAPO TRANS PRESTOR DRAIN NEWSTOR STORAGE 28.1718+ 0.0000+ 0.0000 - 0.0206- 0.1652- 0.0005 = 27.9855 Versus 27.9855 Mass Balance = 3.8147E-06 cm; Time step attempts = 1148 and successes = 1148 Evaporation: Potential = 0.0822 cm, Actual = 0.0206 cm Transpiration: Potential = 1.5621 cm, Actual = 0.1652 cm RHMEAN = 46.6 %; TMEAN = 283.4 K; HDRY = 1.0464E+06 cm; DAYUBC = Ο DAILY SUMMARY: Day = 299, Simulated Time = 24.0000 hr Node Number Depth (cm) = 2 12 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.12191 0.24108 0.25316 = 1.37920E+04 1.58936E+03 1.20824E+03Head (cm) Water Flow (cm) =-9.73897E-04 2.79704E-03 5.22161E-04 Plant Sink (cm) = 1.30809E-03 4.41564E-04 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 27.9855+ 0.0000+ 0.0000 - 0.0168- 0.1180- 0.0005 = 27.8501 Versus 27.8501 Mass Balance = 0.0000E+00 cm; Time step attempts = 1745 and successes = 1745 Evaporation: Potential = 0.0674 cm, Actual = 0.0168 cm Transpiration: Potential = 1.2797 cm, Actual = 0.1180 cm RHMEAN = 51.1 %; TMEAN = 282.6 K; HDRY = 9.2099E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 300, Simulated Time = 24.0000 hr

Node Number = 2 12 25

Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.12125 0.24085 0.25304 = 1.42585E+04 1.59838E+03 1.21116E+03 Head (cm) Water Flow (cm) =-6.98142E-04 2.28469E-03 5.18313E-04 Plant Sink (cm) = 7.44067E-04 4.15432E-04 0.00000E+00PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 27.8501+ 0.0000+ 0.0000 - 0.0158- 0.1003- 0.0005 = 27.7334 Versus 27.7335 Mass Balance = -4.7684E-05 cm; Time step attempts = 2578 and successes = 2578 Evaporation: Potential = 0.0634 cm, Actual = 0.0158 cm Transpiration: Potential = 1.2040 cm, Actual = 0.1003 cm RHMEAN = 53.9 %; TMEAN = 281.8 K; HDRY = 8.4799E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 301, Simulated Time = 24.0000 hr -----Node Number = Depth (cm) = 2 12 25 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.12053 0.24008 0.25296= 1.47934E+04 1.62810E+03 1.21342E+03 Head (cm) Water Flow (cm) =-4.43375E-04 1.56412E-03 5.14880E-04 Plant Sink (cm) = 6.74891E-04 8.40666E-04 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 27.7335+ 0.0000+ 0.0000 - 0.0321- 0.1770- 0.0005 = 27.5239 Versus 27.5246 Mass Balance = -7.5531E-04 cm; Time step attempts =13621 and successes =13621 Evaporation: Potential = 0.1282 cm, Actual = 0.0321 cm Transpiration: Potential = 2.4364 cm, Actual = 0.1770 cm RHMEAN = 48.7 %; TMEAN = 285.7 K; HDRY = 9.8559E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 302, Simulated Time = 24.0000 hr Node Number=21225Depth (cm)=5.0000050.00000122.00000Water (cm3/cm3)=0.120100.239330.2529215125051041658215+031.214355+03 = 1.51259E+04 1.65821E+03 1.21435E+03 Head (cm) Water Flow (cm) =-2.41426E-04 7.26084E-04 5.12502E-04 Plant Sink (cm) = 1.25565E-04 8.37921E-04 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 27.5246+ 0.0000+ 0.0000 - 0.0320- 0.1486- 0.0005 = 27.3436 Versus 27.3453 Mass Balance = -1.7910E-03 cm; Time step attempts =75994 and successes =75994 Evaporation: Potential = 0.1278 cm, Actual = 0.0320 cm Transpiration: Potential = 2.4285 cm, Actual = 0.1486 cm RHMEAN = 47.9 %; TMEAN = 289.5 K; HDRY = 1.0097E+06 cm; DAYUBC = 0 

DAILY SUMMARY: Day = 303, Simulated Time = 24.0000 hr ______ Node Number = 2 12 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.11914 0.23872 0.25286 Head  $(cm) = 1.59113E+04 \ 1.68317E+03 \ 1.21601E+03$ Water Flow (cm) =-1.96278E-04 1.29788E-04 5.11588E-04 Plant Sink (cm) =  $0.00000E+00 \ 6.40584E-04 \ 0.00000E+00$ PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 27.3453+ 0.0000+ 0.0000 - 0.0118- 0.1010- 0.0005 = 27.2320 Versus 27.2335 Mass Balance = -1.5240E-03 cm; Time step attempts =54742 and successes =54742 Evaporation: Potential = 0.0977 cm, Actual = 0.0118 cm Transpiration: Potential = 1.8566 cm, Actual = 0.1010 cm RHMEAN = 55.3 %; TMEAN = 287.6 K; HDRY = 8.1282E+05 cm; DAYUBC = 5076 DAILY SUMMARY: Day = 304, Simulated Time = 24.0000 hr ______ Node Number = 2 12 25 Depth (cm) Depth (cm) = 5.00000 50.00000 Water (cm3/cm3) = 0.11819 0.23791 5.00000 122.00000 0.25275 Head (cm) = 1.67469E+04 1.71678E+03 1.21893E+03Water Flow (cm) =-2.32401E-04-5.46218E-04 5.08266E-04 Plant Sink (cm) = 0.00000E+00 6.98808E-04 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 27.2335+ 0.0000+ 0.0000 - 0.0018- 0.0998- 0.0005 = 27.1315 Versus 27.1315 Mass Balance = -9.5367E-06 cm; Time step attempts = 817 and successes = 817 Evaporation: Potential = 0.1066 cm, Actual = 0.0018 cm Transpiration: Potential = 2.0253 cm, Actual = 0.0998 cm RHMEAN = 65.8 %; TMEAN = 289.8 K; HDRY = 5.7344E+05 cm; DAYUBC = 738 DAILY SUMMARY: Day = 305, Simulated Time = 24.0000 hr Node Number 2 12 25 = 5.00000 50.00000 122.00000 Depth (cm) = 5.00000 Water (cm3/cm3) = 0.11746 0.23730 0.25264 Head (cm) = 1.74402E+04 1.74300E+03 1.22181E+03Water Flow (cm) =-2.67939E-04-1.15298E-03 5.04574E-04 Plant Sink (cm) = 0.00000E+00 5.02057E-04 0.00000E+00 TRANS DRAIN NEWSTOR PRESTOR INFIL RUNOFF EVAPO STORAGE 27.1315+ 0.0000+ 0.0000 - 0.0009- 0.0660- 0.0005 = 27.0640 Versus 27.0641 Mass Balance = -4.0054E-05 cm; Time step attempts = 823 and successes = 823 Evaporation: Potential = 0.0766 cm, Actual = 0.0009 cmTranspiration: Potential = 1.4551 cm, Actual = 0.0660 cm RHMEAN = 73.2 %; TMEAN = 294.3 K; HDRY = 4.2807E+05 cm; DAYUBC = 739

DAILY SUMMARY: Day = 306, Simulated Time = 24.0000 hr -----Node Number = 2 12 25 Depth (cm) -5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.11688 0.23719 0.25253Head (cm) = 1.80135E+04 1.74774E+03 1.22468E+03Water Flow (cm) =-3.07431E-04-1.45814E-03 5.00967E-04 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 27.0641+ 0.0000+ 0.0000 - 0.0000- 0.0000- 0.0005 = 27.0636 Versus 27.0636 Mass Balance = 0.0000E+00 cm; Time step attempts = 96 and successes = 96 Evaporation: Potential = 0.0000 cm, Actual = 0.0000 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 93.0 %; TMEAN = 291.5 K; HDRY = 9.9725E+04 cm; DAYUBC = DAILY SUMMARY: Day = 307, Simulated Time = 24.0000 hr _____ Node Number 2 Married Married 12 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.32265 0.23711 0.25246 = 2.44589E+02 1.75115E+03 1.22664E+03 Head (cm) Water Flow (cm) = 2.87406E+00-1.54093E-03 4.98332E-04 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 INFIL RUNOFF PRESTOR EVAPO TRANS DRAIN NEWSTOR STORAGE 27.0636+ 4.3828+ 2.6784 - 0.0000- 0.0000- 0.0005 = 31.4459 Versus 31.4472 Mass Balance = -1.3142E-03 cm; Time step attempts =29266 and successes =29266 Evaporation: Potential = 0.0000 cm, Actual = 0.0000 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 99.7 %; TMEAN = 289.5 K; HDRY = 4.5820E+03 cm; DAYUBC = 18715 DAILY SUMMARY: Day = 308, Simulated Time = 24.0000 hr _____ Node Number = Depth (cm) = 12 2 25 5.00000 50.00000 122.00000 = 0.29879 Water (cm3/cm3) 0.23688 0.25235 = 3.43786E+02 1.76114E+03 1.22950E+03Head (cm) Water Flow (cm) = 1.26127E-01-1.68435E-03 4.94971E-04Plant Sink (cm) = 8.03227E-03 1.26202E-04 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 31.4472+ 0.0000+ 0.0000 - 0.0048- 0.0914- 0.0005 = 31.3504 Versus 31.3504 Mass Balance = 7.6294E-06 cm; Time step attempts = 690 and successes = 690 Evaporation: Potential = 0.0193 cm, Actual = 0.0048 cm

Transpiration: Potential = 0.3658 cm, Actual = 0.0914 cm RHMEAN = 86.9 %; TMEAN = 290.7 K; HDRY = 1.9254E+05 cm; DAYUBC = DAILY SUMMARY: Day = 309, Simulated Time = 24.0000 hr _____ Node Number Depth (cm) ..... 2 12 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.28442 0.23666 0.25225 Head (cm) = 4.18447E+02 1.77096E+03 1.23234E+03Water Flow (cm) = 4.88517E-02-1.86867E-03 4.91473E-04Plant Sink (cm) = 1.01571E-02 1.59588E-04 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 31.3504+ 0.0000+ 0.0000 - 0.0061- 0.1156- 0.0005 = 31.2282 Versus 31.2282 Mass Balance = 0.0000E+00 cm; Time step attempts = 96 and successes = 96 Evaporation: Potential = 0.0243 cm, Actual = 0.0061 cm Transpiration: Potential = 0.4625 cm, Actual = 0.1156 cm RHMEAN = 78.9 %; TMEAN = 289.8 K; HDRY = 3.2513E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 310, Simulated Time = 24.0000 hr _____ Node Number=21225Depth (cm)=5.0000050.00000122.00000Water (cm3/cm3)=0.267910.236440.25214 Head (cm) = 5.24219E+02 1.78063E+03 1.23515E+03Water Flow (cm) = 8.73315E-03-2.04387E-034.88044E-04Plant Sink (cm) = 2.09441E-02 3.29070E-04 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 31.2282+ 0.0000+ 0.0000 - 0.0125- 0.2384- 0.0005 = 30.9767 Versus 30.9767 Mass Balance = -5.7220E-06 cm; Time step attempts = 96 and successes = 96 Evaporation: Potential = 0.0502 cm, Actual = 0.0125 cm Transpiration: Potential = 0.9537 cm, Actual = 0.2384 cm RHMEAN = 74.9 %; TMEAN = 289.8 K; HDRY = 3.9616E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 311, Simulated Time = 24.0000 hr ______ Node Number Depth (cm) ..... 2 12 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.24396 0.23629 0.25204 Head (cm) = 7.34711E+02 1.78726E+03 1.23793E+03Water Flow (cm) =-2.52884E-02-2.17753E-03 4.84676E-04 Plant Sink (cm) = 3.76489E-02 5.91534E-04 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 30.9767+ 0.0000+ 0.0000 - 0.0226- 0.4286- 0.0005 = 30.5251 Versus 30.5251

Mass Balance = -9.5367E-06 cm; Time step attempts = 96 and successes = 96 Evaporation: Potential = 0.0902 cm, Actual = 0.0226 cm Transpiration: Potential = 1.7144 cm, Actual = 0.4286 cm RHMEAN = 59.4 %; TMEAN = 290.7 K; HDRY = 7.1452E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 312, Simulated Time = 24.0000 hr _____ Node Number 5772 2 12 25 Depth (cm) = 5.00000 50.0000 122.0000 Water (cm3/cm3) = 0.23681 0.23673 0.25194 = 8.16420E+02 1.76785E+03 1.24060E+03 Head (cm) Water Flow (cm) = 1.01979E-02-2.07153E-03 4.81470E-04Plant Sink (cm) = 2.25540E-02 3.54365E-04 0.00000E+00PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 30.5251+ 0.0762+ 0.0000 - 0.0135- 0.2568- 0.0005 = 30.3306 Versus 30.3306 Mass Balance = -8.3923E-05 cm; Time step attempts = 2096 and successes = 2096 Evaporation: Potential = 0.0546 cm, Actual = 0.0135 cm Transpiration: Potential = 1.0374 cm, Actual = 0.2568 cm RHMEAN = 74.0 %; TMEAN = 286.2 K; HDRY = 4.1295E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 313, Simulated Time = 24.0000 hr ______ 12 Node Number = 2 25 Depth (cm) = 5.00000 50.00000 122.00000 = 0.22822 0.23753 0.25184Water (cm3/cm3) Head (cm) = 9.30365E+02 1.73285E+03 1.24333E+03 Water Flow (cm) =-1.52823E-02-1.64315E-03 4.78221E-04 Plant Sink (cm) = 1.49331E-02 2.34627E-04 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 30.3306+ 0.0000+ 0.0000 - 0.0089- 0.1700- 0.0005 = 30.1512 Versus 30.1512 Mass Balance = -1.9073E-06 cm; Time step attempts = 96 and successes = 96 Evaporation: Potential = 0.0358 cm, Actual = 0.0089 cm Transpiration: Potential = 0.6800 cm, Actual = 0.1700 cm RHMEAN = 72.8 %; TMEAN = 282.6 K; HDRY = 4.3557E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 314, Simulated Time = 24.0000 hr _____ 2 Node Number = 12 25 Depth (cm) = 5.00000 50.00000 122.00000Water (cm3/cm3) = 0.21392 0.23831 0.25174Head (cm) = 1.17071E+03 1.70017E+03 1.24604E+03 Water Flow (cm) =-2.23819E-02-1.11382E-03 4.75030E-04

Plant Sink (cm) = 2.33110E-02 3.66259E-04 0.00000E+00

PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 30.1512+ 0.0000+ 0.0000 - 0.0140- 0.2654- 0.0005 = 29.8714 Versus 29.8714 Mass Balance = -5.7220E-06 cm; Time step attempts = 96 and successes = 96 Evaporation: Potential = 0.0559 cm, Actual = 0.0140 cm Transpiration: Potential = 1.0615 cm, Actual = 0.2654 cm RHMEAN = 63.9 %; TMEAN = 287.6 K; HDRY = 6.1356E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 315, Simulated Time = 24.0000 hr _____ 2 Node Number = 12 25 Depth (cm) = 5.00000 50.00000 122.00000Water (cm3/cm3) = 0.20339 0.23913 0.25164Head (cm) = 1.40369E+03 1.66639E+03 1.24871E+03Water Flow (cm) =-2.28220E-02-5.99569E-04 4.71899E-04 Plant Sink (cm) =  $1.87005E-02\ 2.93819E-04\ 0.00000E+00$ PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 29.8714+ 0.0000+ 0.0000 - 0.0112- 0.2129- 0.0005 = 29.6468 Versus 29.6468 Mass Balance = -1.9073E-06 cm; Time step attempts = 96 and successes = 96 Evaporation: Potential = 0.0448 cm, Actual = 0.0112 cm Transpiration: Potential = 0.8516 cm, Actual = 0.2129 cm RHMEAN = 73.2 %; TMEAN = 286.2 K; HDRY = 4.2765E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 316, Simulated Time = 24.0000 hr Node Number = Depth (cm) = 2 12 25 5.00000 50.00000 122.00000 Solution-5.0000050.00000Water (cm3/cm3)=0.190690.239810.25154 Head (cm) = 1.77904E+03 1.63869E+03 1.25136E+03Water Flow (cm) =-2.24515E-02-1.33364E-04 4.68828E-04 Plant Sink (cm) = 2.08185E-02 3.27098E-04 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 29.6468+ 0.0000+ 0.0000 - 0.0125- 0.2370- 0.0005 = 29.3969 Versus 29.3969 Mass Balance = 0.0000E+00 cm; Time step attempts = 204 and successes = 204 Evaporation: Potential = 0.0499 cm, Actual = 0.0125 cm Transpiration: Potential = 0.9480 cm, Actual = 0.2370 cm RHMEAN = 72.8 %; TMEAN = 286.8 K; HDRY = 4.3547E+05 cm; DAYUBC = 0 _____ DAILY SUMMARY: Day = 317, Simulated Time = 24.0000 hr _____ Node Number=21225Depth (cm)=5.0000050.00000122.00000Water (cm3/cm3)=0.177130.240360.25145

= 2.35657E+03 1.61729E+03 1.25398E+03 Head (cm) Water Flow (cm) =-2.10844E-02 2.10831E-04 4.65816E-04 Plant Sink (cm) = 2.13860E-02 3.36013E-04 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 29.3969+ 0.0000+ 0.0000 - 0.0128- 0.2435- 0.0005 = 29.1402 Versus 29.1402 Mass Balance = -5.7220E-06 cm; Time step attempts = 296 and successes = 296 Evaporation: Potential = 0.0513 cm, Actual = 0.0128 cm Transpiration: Potential = 0.9738 cm, Actual = 0.2435 cm RHMEAN = 79.3 %; TMEAN = 288.7 K; HDRY = 3.1857E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 318, Simulated Time = 24.0000 hr Node Number=21225Depth (cm)=5.0000050.00000122.00000Water (cm3/cm3)=0.159560.240680.25135 Head (cm) = 3.59968E+03 1.60485E+03 1.25657E+03 Water Flow (cm) =-1.81821E-02 3.98481E-04 4.62866E-04 Plant Sink (cm) = 2.47265E-02 4.07056E-04 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 29.1402+ 0.0000+ 0.0000 - 0.0155- 0.2896- 0.0005 = 28.8346 Versus 28.8346 Mass Balance = -1.9073E-06 cm; Time step attempts = 569 and successes = 569 Evaporation: Potential = 0.0621 cm, Actual = 0.0155 cmTranspiration: Potential = 1.1797 cm, Actual = 0.2896 cmRHMEAN = 74.5 %; TMEAN = 290.7 K; HDRY = 4.0292E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 319, Simulated Time = 24.0000 hr ______ Node Number Depth (cm) 2 === 12 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.15259 0.24101 0.25126 Head (cm) = 4.36777E+03 1.59204E+03 1.25912E+03 Water Flow (cm) =-1.35100E-02 4.98894E-04 4.59975E-04 Plant Sink (cm) = 1.23979E-02 2.52871E-04 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 28.8346+ 0.0000+ 0.0000 - 0.0096- 0.1662- 0.0005 = 28.6583 Versus 28.6583 Mass Balance = 1.9073E-06 cm; Time step attempts = 426 and successes = 426 Evaporation: Potential = 0.0386 cm, Actual = 0.0096 cm Transpiration: Potential = 0.7329 cm, Actual = 0.1662 cm RHMEAN = 75.9 %; TMEAN = 288.2 K; HDRY = 3.7753E+05 cm; DAYUBC = 0 

DAILY SUMMARY: Day = 320, Simulated Time = 24.0000 hr

Node Number = 2 12 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.15424 0.24144 0.25117 Head (cm) = 4.16512E+03 1.57590E+03 1.26165E+03Water Flow (cm) =-1.08756E-02 6.46804E-04 4.57140E-04 Plant Sink (cm) = 2.23217E-03 4.89570E-05 0.00000E+00PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 28.6583+ 0.0000+ 0.0000 - 0.0019- 0.0312- 0.0005 = 28.6248 Versus 28.6248 Mass Balance = -1.9073E-06 cm; Time step attempts = 96 and successes = 96 Evaporation: Potential = 0.0075 cm, Actual = 0.0019 cm Transpiration: Potential = 0.1419 cm, Actual = 0.0312 cm RHMEAN = 81.0 %; TMEAN = 287.0 K; HDRY = 2.8924E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 321, Simulated Time = 24.0000 hr -----Node Number === 2 12 25 Depth (cm) = Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.15279 0.24172 0.25108 Head (cm) = 4.34198E+03 1.56527E+03 1.26415E+03Water Flow (cm) =-9.77812E-03 7.74623E-04 4.54358E-04 Plant Sink (cm) = 4.49595E-03 9.85389E-05 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 28.6248+ 0.0000+ 0.0000 - 0.0038- 0.0625- 0.0005 = 28.5580 Versus 28.5580 Mass Balance = -3.8147E-06 cm; Time step attempts = 160 and successes = 160Evaporation: Potential = 0.0150 cm, Actual = 0.0038 cmTranspiration: Potential = 0.2856 cm, Actual = 0.0625 cm RHMEAN = 75.4 %; TMEAN = 289.5 K; HDRY = 3.8754E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 322, Simulated Time = 24.0000 hr _____ 2 = Node Number 12 25 Depth (cm) = 5.00000 50.0000 122.00000Water (cm3/cm3) = 0.21858 0.24165 0.25099Head (cm) = 1.08421E+03 1.56806E+03 1.26650E+03Water Flow (cm)  $= 2.76622E-01 \ 6.96917E-04 \ 4.51752E-04$ Plant Sink (cm) = 2.29264E-02 3.60216E-04 0.00000E+00 INFIL RUNOFF DRAIN NEWSTOR PRESTOR EVAPO TRANS STORAGE 28.5580+ 0.8605+ 0.1809 - 0.0137- 0.2610- 0.0005 = 29.1433 Versus 29.1436 Mass Balance = -2.9373E-04 cm; Time step attempts = 5788 and successes = 5788 Evaporation: Potential = 0.0561 cm, Actual = 0.0137 cm Transpiration: Potential = 1.0653 cm, Actual = 0.2610 cm RHMEAN = 78.0 %; TMEAN = 291.2 K; HDRY = 3.4017E+05 cm; DAYUBC = 582

DAILY SUMMARY: Day = 323, Simulated Time = 24.0000 hr ____ Node Number ...... 2 12 25 Note Number=21223Depth (cm)=5.0000050.00000122.00000Water (cm3/cm3)=0.240880.241300.25091Head (cm) = 7.68619E+02 1.58129E+03 1.26885E+03Water Flow (cm) = 5.64072E-01 3.61947E-04 4.49176E-04Plant Sink (cm) = 3.49560E-02 5.49223E-04 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 29.1436+ 0.8890+ 0.0000 - 0.0209- 0.3979- 0.0004 = 29.6133 Versus 29.6134 Mass Balance = -4.3869E-05 cm; Time step attempts = 2951 and successes = 2951 Evaporation: Potential = 0.0846 cm, Actual = 0.0209 cmTranspiration: Potential = 1.6078 cm, Actual = 0.3979 cm RHMEAN = 63.1 %; TMEAN = 281.2 K; HDRY = 6.3139E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 324, Simulated Time = 24.0000 hr _____ Node Number Depth (cm) 2 12 25 ____ Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.22270 0.24117 0.25082 = 1.01463E+03 1.58608E+03 1.27126E+03 Head (cm) Water Flow (cm) = 4.64660E-02 3.54897E-05 4.46558E-04Plant Sink (cm) = 1.64243E-02 2.58056E-04 0.00000E+00 INFIL RUNOFF PRESTOR EVAPO TRANS DRAIN NEWSTOR STORAGE 29.6134+ 0.0000+ 0.0000 - 0.0098- 0.1870- 0.0004 = 29.4161 Versus 29.4161 Mass Balance = -1.9073E-06 cm; Time step attempts = 96 and successes = 96Evaporation: Potential = 0.0394 cm, Actual = 0.0098 cm Transpiration: Potential = 0.7479 cm, Actual = 0.1870 cm RHMEAN = 57.0 %; TMEAN = 278.2 K; HDRY = 7.7126E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 325, Simulated Time = 24.0000 hr Node Number = Depth (cm) = 2 12 25 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.20650 0.240940.25074 = 1.32881E+03 1.59504E+03 1.27364E+03Head (cm) Water Flow (cm) = 7.96343E-03-2.11825E-04 4.43989E-04 Plant Sink (cm) = 2.01514E-02 3.16615E-04 0.00000E+00PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 29.4161+ 0.0000+ 0.0000 - 0.0121- 0.2294- 0.0004 = 29.1742 Versus 29.1742 Mass Balance = -1.9073E-06 cm; Time step attempts = 112 and successes = 112Evaporation: Potential = 0.0483 cm, Actual = 0.0121 cm Transpiration: Potential = 0.9176 cm, Actual = 0.2294 cm RHMEAN = 59.2 %; TMEAN = 279.5 K; HDRY = 7.1968E+05 cm; DAYUBC = 0
DAILY SUMMARY: Day = 326, Simulated Time = 24.0000 hr == Node Number 2 12 25 Depth (cm) = 5.00000 50.00000 122.00000 = 0.17689 0.24037Water (cm3/cm3) 0.25066 = 2.36867E+03 1.61676E+03 1.27598E+03 Head (cm) Water Flow (cm) =-1.03211E-02-6.14733E-04 4.41472E-04 Plant Sink (cm) = 3.81520E-02 5.99440E-04 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 29.1742+ 0.0000+ 0.0000 - 0.0229- 0.4343- 0.0004 = 28.7165 Versus 28.7165 Mass Balance = 0.0000E+00 cm; Time step attempts = 475 and successes = 475 Evaporation: Potential = 0.0914 cm, Actual = 0.0229 cm Transpiration: Potential = 1.7373 cm, Actual = 0.4343 cm RHMEAN = 66.2 %; TMEAN = 285.4 K; HDRY = 5.6475E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 327, Simulated Time = 24.0000 hr ______ 2 Node Number = 12 25 Depth (cm) = 5.00000 Water (cm3/cm3) = 0.15987 50.00000 122.00000 0.23997 0.25057 Head (cm) = 3.57038E+03 1.63248E+03 1.27830E+03 Water Flow (cm) =-1.27634E-02-1.04487E-03 4.39005E-04 Plant Sink (cm) = 2.45811E-024.03297E-040.00000E+00PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 28.7165+ 0.0000+ 0.0000 - 0.0154- 0.2873- 0.0004 = 28.4134 Versus 28.4134 Mass Balance = 1.3351E-05 cm; Time step attempts = 569 and successes = 569 Evaporation: Potential = 0.0615 cm, Actual = 0.0154 cm Transpiration: Potential = 1.1688 cm, Actual = 0.2873 cm RHMEAN = 77.1 %; TMEAN = 290.4 K; HDRY = 3.5660E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 328, Simulated Time = 24.0000 hr Node Number == 2 12 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.14785 0.239580.25049 Head (cm) = 5.03331E+03 1.64801E+03 1.28059E+03Water Flow (cm) =-8.74414E-03-1.36858E-03 4.36588E-04 Plant Sink (cm) = 1.69942E-02 3.72866E-04 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 28.4134+ 0.0000+ 0.0000 - 0.0142- 0.2352- 0.0004 = 28.1636 Versus 28.1636 Mass Balance = -3.8147E-06 cm; Time step attempts = 691 and successes = 691

Evaporation: Potential = 0.0569 cm, Actual = 0.0142 cmTranspiration: Potential = 1.0806 cm, Actual = 0.2352 cm RHMEAN = 65.7 %; TMEAN = 285.1 K; HDRY = 5.7608E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 329, Simulated Time = 24.0000 hr _____ Node Number 20072 2 12 25 Depth (cm) Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.13560 0.23902 0.25041 Head (cm) = 7.64284E+03 1.67081E+03 1.28284E+03Water Flow (cm) =-5.00548E-03-1.72866E-03 4.34219E-04 Plant Sink (cm) = 1.52148E-02 5.31457E-04 0.00000E+00EVAPO TRANS PRESTOR INFIL RUNOFF DRAIN NEWSTOR STORAGE 28.1636+ 0.0000+ 0.0000 - 0.0203- 0.2681- 0.0004 = 27.8748 Versus 27.8748 Mass Balance = 3.8147E-06 cm; Time step attempts = 1015 and successes = 1015 Evaporation: Potential = 0.0811 cm, Actual = 0.0203 cm Transpiration: Potential = 1.5403 cm, Actual = 0.2681 cm RHMEAN = 60.3 %; TMEAN = 284.0 K; HDRY = 6.9338E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 330, Simulated Time = 24.0000 hr _____ Node Number = 2 12 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.13004 0.23855 0.25033 Head (cm) = 9.52588E+03 1.69009E+03 1.28507E+03Water Flow (cm) =-2.77135E-03-2.08593E-03 4.31898E-04 Plant Sink (cm) = 7.21283E-03 4.24928E-04 0.00000E+00 INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR PRESTOR STORAGE 27.8748+ 0.0000+ 0.0000 - 0.0162- 0.1684- 0.0004 = 27.6898 Versus 27.6897 Mass Balance = 5.7220E-06 cm; Time step attempts = 789 and successes = 789 Evaporation: Potential = 0.0648 cm, Actual = 0.0162 cm Transpiration: Potential = 1.2315 cm, Actual = 0.1684 cm RHMEAN = 61.5 %; TMEAN = 283.4 K; HDRY = 6.6580E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 331, Simulated Time = 24.0000 hr Node Number = 2 12 25 Depth (cm) Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.12783 0.23822 0.25026 Head (cm) = 1.04687E+04 1.70354E+03 1.28727E+03Water Flow (cm) =-1.85611E-03-2.32369E-03 4.29622E-04 Plant Sink (cm) = 3.22952E-03 2.72409E-04 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE

27.6897 + 0.0000 + 0.0000 - 0.0104 - 0.0923 - 0.0004 = 27.5866 Versus 27.5866 Mass Balance = 3.8147E-06 cm; Time step attempts = 491 and successes = 491 Evaporation: Potential = 0.0416 cm, Actual = 0.0104 cm Transpiration: Potential = 0.7895 cm, Actual = 0.0923 cm RHMEAN = 60.6 %; TMEAN = 274.3 K; HDRY = 6.8619E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 332, Simulated Time = 24.0000 hr ______ Node Number = 2 12 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.12863 0.23817 0.25018 Head (cm)  $= 1.01133E+04 \ 1.70580E+03 \ 1.28943E+03$ Water Flow (cm) =-1.61657E-03-2.39318E-03 4.27396E-04 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 INFIL RUNOFF DRAIN NEWSTOR PRESTOR EVAPO TRANS STORAGE 27.5866+ 0.0000+ 0.0000 - 0.0000- 0.0000- 0.0004 = 27.5862 Versus 27.5862 Mass Balance = -3.8147E-06 cm; Time step attempts = 96 and successes = 96 Evaporation: Potential = 0.0000 cm, Actual = 0.0000 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cmRHMEAN = 91.7 %; TMEAN = 270.9 K; HDRY = 1.1831E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 333, Simulated Time = 24.0000 hr ----------Node Number = Depth (cm) = 2 12 25 5.00000 50.00000 122.00000  $= 0.12937 \quad 0.23812 \quad 0.25011$ Water (cm3/cm3) = 9.79905E+03 1.70814E+03 1.29156E+03 Head (cm) Water Flow (cm) =-1.64331E-03-2.36748E-03 4.25217E-04 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 STORAGE PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR 27.5862+ 0.0000+ 0.0000 - 0.0000- 0.0000- 0.0004 = 27.5858 Versus 27.5858 Mass Balance = 0.0000E+00 cm; Time step attempts = 96 and successes = 96 Evaporation: Potential = 0.0000 cm, Actual = 0.0000 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cmRHMEAN = 90.8 %; TMEAN = 272.6 K; HDRY = 1.3263E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 334, Simulated Time = 24.0000 hr _____ 2 Node Number 222 12 25 Depth (cm) = 5.00000 50.00000 122.00000Water (cm3/cm3) = 0.19841 0.23768 0.25004Head (cm) = 1.53634E+03 1.72660E+03 1.29351E+03 Water Flow (cm) = 9.94003E - 02 - 2.48913E - 03 4.23239E - 04

Plant Sink (cm) = 2.43605E-02 3.83348E-04 0.00000E+00PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 27.5858+ 0.7194+ 0.0679 - 0.0146- 0.2239- 0.0004 = 28.0663 Versus 28.0666 Mass Balance = -3.2616E-04 cm; Time step attempts = 8914 and successes = 8914Evaporation: Potential = 0.0591 cm, Actual = 0.0146 cmTranspiration: Potential = 1.1222 cm, Actual = 0.2239 cm RHMEAN = 66.6 %; TMEAN = 279.5 K; HDRY = 5.5622E+05 cm; DAYUBC = 1295 DAILY SUMMARY: Day = 335, Simulated Time = 24.0000 hr _____ 2 Node Number ----12 25 Note Number=21223Depth (cm)=5.0000050.00000122.00000Water (cm3/cm3)=0.185200.237490.24997 Head (cm) = 1.98556E+03 1.73453E+03 1.29559E+03Water Flow (cm) = 6.06096E - 02 - 2.65035E - 03 4.21144E - 04Plant Sink (cm) = 8.01104E-03 1.25868E-04 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 28.0666+ 0.0000+ 0.0000 - 0.0048- 0.0784- 0.0004 = 27.9830 Versus 27.9830 Mass Balance = 0.0000E+00 cm; Time step attempts = 411 and successes = 411 Evaporation: Potential = 0.0192 cm, Actual = 0.0048 cm Transpiration: Potential = 0.3648 cm, Actual = 0.0784 cm RHMEAN = 75.7 %; TMEAN = 280.4 K; HDRY = 3.8214E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 336, Simulated Time = 24.0000 hr _____ Node Number Depth (cm) == 2 12 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.17723 0.23747 0.24993 Head (cm) = 2.35136E+03 1.73579E+03 1.29656E+03Water Flow (cm) = 2.91083E - 02 - 2.66955E - 03 4.19375E - 04Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 27.9830+ 0.0000+ 0.0000 - 0.2655- 0.0000- 0.0004 = 27.7171 Versus 27.7188 Mass Balance = -1.7700E-03 cm; Time step attempts =61892 and successes =61892 Evaporation: Potential = 1.0621 cm, Actual = 0.2655 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 76.1 %; TMEAN = 284.3 K; HDRY = 3.7493E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 337, Simulated Time = 24.0000 hr _____ Node Number = 2 12 25 Depth (cm) = 5.00000 50.00000 122.00000

Water (cm3/cm3) = 0.17252 0.237410.24986 Head (cm) = 2.61407E+03 1.73830E+03 1.29850E+03Water Flow (cm) = 1.45058E-02-2.65371E-03 4.18226E-04Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 INFIL RUNOFF EVAPO TRANS PRESTOR DRAIN NEWSTOR STORAGE 27.7188+ 0.0000+ 0.0000 - 0.0000- 0.0000- 0.0004 = 27.7184 Versus 27.7186 Mass Balance = -1.6975E-04 cm; Time step attempts = 2595 and successes = 2595 Evaporation: Potential = 0.0734 cm, Actual = 0.0000 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 91.2 %; TMEAN = 287.3 K; HDRY = 1.2674E+05 cm; DAYUBC = DAILY SUMMARY: Day = 338, Simulated Time = 24.0000 hr Node Number = 2 12 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.16933 0.23736 0.24981Head (cm) = 2.81676E+03 1.74039E+03 1.30010E+03Water Flow (cm) = 8.67969E-03-2.63615E-03 4.16605E-04Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 27.7186+ 0.0000+ 0.0000 - 0.0175- 0.0000- 0.0004 = 27.7006 Versus 27.7018 Mass Balance = -1.1578E-03 cm; Time step attempts =36919 and successes =36919 Evaporation: Potential = 1.3539 cm, Actual = 0.0175 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 78.3 %; TMEAN = 293.2 K; HDRY = 3.3486E+05 cm; DAYUBC = 4558 DAILY SUMMARY: Day = 339, Simulated Time = 24.0000 hr _____ 2 Node Number = 12 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.16686 0.23730 0.24975 = 2.98924E+03 1.74283E+03 1.30192E+03 Head (cm) Water Flow (cm) = 5.58278E-03-2.62113E-03 4.14838E-04Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 INFIL RUNOFF EVAPO TRANS PRESTOR DRAIN NEWSTOR STORAGE 27.7018+ 0.0000+ 0.0000 - 0.0082- 0.0000- 0.0004 = 27.6932 Versus 27.6936 Mass Balance = -4.0245E-04 cm; Time step attempts =12591 and successes =12591 Evaporation: Potential = 1.2723 cm, Actual = 0.0082 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cmRHMEAN = 72.7 %; TMEAN = 290.4 K; HDRY = 4.3702E+05 cm; DAYUBC = 3374 

DAILY SUMMARY: Day = 340, Simulated Time = 24.0000 hr

_____ Node Number = 2 12 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.16475 0.23725 0.24969 = 3.14941E+03 1.74496E+03 1.30349E+03 Head (cm) Water Flow (cm) = 3.52947E-03-2.61064E-03 4.13258E-04Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 27.6936+ 0.0000+ 0.0000 - 0.0087- 0.0000- 0.0004 = 27.6845 Versus 27.6848 Mass Balance = -3.1662E-04 cm; Time step attempts =33731 and successes =33731 Evaporation: Potential = 0.4185 cm, Actual = 0.0087 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 64.1 %; TMEAN = 284.3 K; HDRY = 6.1043E+05 cm; DAYUBC = 3686 DAILY SUMMARY: Day = 341, Simulated Time = 24.0000 hr Node Number = 2 12 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.16312 0.23719 0.24962 122.00000 = 3.28128E+03 1.74760E+03 1.30542E+03 Head (cm) Water Flow (cm) = 2.11835E-03-2.60143E-03 4.11407E-04Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 27.6848+ 0.0000+ 0.0000 - 0.0000- 0.0000- 0.0004 = 27.6844 Versus 27.6844 Mass Balance = 0.0000E+00 cm; Time step attempts = 96 and successes = 96 Evaporation: Potential = 0.4965 cm, Actual = 0.0000 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 77.8 %; TMEAN = 283.2 K; HDRY = 3.4344E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 342, Simulated Time = 24.0000 hr _____ Node Number=21225Depth (cm)=5.0000050.00000122.00000Water (cm3/cm3)=0.161620.237140.24957 Head (cm) = 3.41081E+03 1.74990E+03 1.30710E+03 Water Flow (cm) = 1.05698E - 03 - 2.59525E - 03 4.09763E - 04Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 27.6844+ 0.0000+ 0.0000 - 0.0119- 0.0000- 0.0004 = 27.6720 Versus 27.6727 Mass Balance = -6.1035E-04 cm; Time step attempts =22587 and successes =22587 Evaporation: Potential = 1.3779 cm, Actual = 0.0119 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 60.8 %; TMEAN = 277.9 K; HDRY = 6.8150E+05 cm; DAYUBC = 3756 

DAILY SUMMARY: Day = 343, Simulated Time = 24.0000 hr _____ Node Number == 2 12 25 5.00000 50.00000 Depth (cm) === 122.00000 = 0.16021 Water (cm3/cm3) 0.23708 0.24951 = 3.53807E+03 1.75229E+03 1.30882E+03 Head (cm) Water Flow (cm) = 1.34990E-04-2.59049E-03 4.08100E-04 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 27.6727+ 0.0000+ 0.0000 - 0.0074- 0.0000- 0.0004 = 27.6649 Versus 27.6652 Mass Balance = -3.0899E-04 cm; Time step attempts =12591 and successes =12591 Evaporation: Potential = 0.5743 cm, Actual = 0.0074 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 55.9 %; TMEAN = 276.8 K; HDRY = 7.9680E+05 cm; DAYUBC = 2744 DAILY SUMMARY: Day = 344, Simulated Time = 24.0000 hr ______ Node Number == 2 12 25 5.00000 50.00000 122.00000 Depth (cm) = Water (cm3/cm3) = 0.159050.23703 0.24945 = 3.64865E+03 1.75479E+03 1.31061E+03 Head (cm) Water Flow (cm) =-5.55225E-04-2.58650E-03 4.06367E-04 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 27.6652+ 0.0000+ 0.0000 - 0.0046- 0.0000- 0.0004 = 27.6602 Versus 27.6603 Mass Balance = -1.2207E-04 cm; Time step attempts = 1559 and successes = 1559 Evaporation: Potential = 0.8985 cm, Actual = 0.0046 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 60.9 %; TMEAN = 276.5 K; HDRY = 6.7873E+05 cm; DAYUBC = 1521 DAILY SUMMARY: Day = 345, Simulated Time = 24.0000 hr ______ Node Number = 2 12 25 Depth (cm) = 5.00000 50.00000 122.00000 = 0.20205 Water (cm3/cm3) 0.23697 0.24939 Head (cm) = 1.43792E+03 1.75719E+03 1.31232E+03Water Flow (cm) = 9.85603E - 02 - 2.58394E - 03 4.04743E - 04Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 27.6603+ 0.6604+ 0.0000 - 0.0733- 0.0000- 0.0004 = 28.2470 Versus 28.2472 Mass Balance = -2.0981E-04 cm; Time step attempts = 5796 and successes = 5796 Evaporation: Potential = 0.2961 cm, Actual = 0.0733 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm

RHMEAN = 85.5 %; TMEAN = 277.3 K; HDRY = 2.1495E+05 cm; DAYUBC = (

DAILY SUMMARY: Day = 346, Simulated Time = 24.0000 hr ______ Node Number === 2 12 25 Depth (cm) Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.22142 0.23692 0.24933 Head (cm) = 1.03566E+03 1.75963E+03 1.31404E+03Water Flow (cm) = 1.86809E-01-2.58198E-03 4.03104E-04Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 28.2472+ 0.3302+ 0.0000 - 0.0000- 0.0000- 0.0004 = 28.5770 Versus 28.5772 Mass Balance = -1.4687E-04 cm; Time step attempts = 2615 and successes = 2615 Evaporation: Potential = 0.0000 cm, Actual = 0.0000 cmTranspiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 98.9 %; TMEAN = 282.6 K; HDRY = 1.4559E+04 cm; DAYUBC = 0 DAILY SUMMARY: Day = 347, Simulated Time = 24.0000 hr _____ 2 Node Number 12 = 25 Node Number-21212Depth (cm)=5.0000050.00000122.00000Water (cm3/cm3)=0.248930.236860.24927Head (cm) = 6.83867E+02 1.76199E+03 1.31573E+03Water Flow (cm) = 4.48607E - 01 - 2.58036E - 03 4.01509E - 04Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR PRESTOR STORAGE 28.5772+ 0.6350+ 0.0000 - 0.0000- 0.0000- 0.0004 = 29.2118 Versus 29.2119 Mass Balance = -1.4496E-04 cm; Time step attempts = 6032 and successes = 6032Evaporation: Potential = 0.0000 cm, Actual = 0.0000 cmTranspiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 94.8 %; TMEAN = 281.2 K; HDRY = 7.3844E+04 cm; DAYUBC = 0 DAILY SUMMARY: Day = 348, Simulated Time = 24.0000 hr _____ Node Number Depth (cm) = 2 12 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.23723 0.23681 0.24921 Head (cm) = 8.11259E+02 1.76441E+03 1.31748E+03Water Flow (cm) = 8.56206E-02-2.57853E-03 3.99863E-04 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 29.2119+ 0.0000+ 0.0000 - 0.0000- 0.0000- 0.0004 = 29.2115 Versus 29.2115

Mass Balance = -1.9073E-06 cm; Time step attempts = 444 and successes = 444 Evaporation: Potential = 0.0000 cm, Actual = 0.0000 cmTranspiration: Potential = 0.0000 cm, Actual = 0.0000 cmRHMEAN = 93.6 %; TMEAN = 279.0 K; HDRY = 9.0333E+04 cm; DAYUBC = 0 DAILY SUMMARY: Day = 349, Simulated Time = 24.0000 hr ______ Node Number Depth (cm) 2 ----12 25 5.00000 50.00000 122.00000 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.23145 0.23675 0.24915 Head (cm) = 8.85274E+02 1.76678E+03 1.31921E+03 Water Flow (cm) = 4.20373E-02-2.57669E-03 3.98243E-04Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 29.2115+ 0.0000+ 0.0000 - 0.0000- 0.0000- 0.0004 = 29.2111 Versus 29.2111 Mass Balance = 1.9073E-06 cm; Time step attempts = 96 and successes = 96 Evaporation: Potential = 0.0000 cm, Actual = 0.0000 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 92.0 %; TMEAN = 280.1 K; HDRY = 1.1449E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 350, Simulated Time = 24.0000 hr ______ Node Number = 2 12 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.33129 0.23670 0.24910 = 2.14219E+02 1.76897E+03 1.32079E+03 Head (cm) Water Flow (cm) = 2.25161E+00-2.57498E-03 3.96758E-04Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 29.2111+ 2.9460+ 0.8386 - 0.0000- 0.0000- 0.0004 = 32.1567 Versus 32.1572 Mass Balance = -4.5013E-04 cm; Time step attempts = 4365 and successes = 4365Evaporation: Potential = 0.0000 cm, Actual = 0.0000 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 97.6 %; TMEAN = 283.2 K; HDRY = 3.3848E+04 cm; DAYUBC = 826 DAILY SUMMARY: Day = 351, Simulated Time = 24.0000 hr _____ Node Number Depth (cm) 2 12 = 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.32711 0.23679 0.24904 Head (cm) = 2.28609E+02 1.76508E+03 1.32240E+03Water Flow (cm) = 6.27459E-01-2.55267E-03 3.95275E-04Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00

PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 32.1572+ 0.7947+ 0.0435 - 0.1985- 0.0000- 0.0004 = 32.7531 Versus 32.7533 Mass Balance = -2.1744E-04 cm; Time step attempts = 5491 and successes = 5491Evaporation: Potential = 0.8018 cm, Actual = 0.1985 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cmRHMEAN = 74.1 %; TMEAN = 280.9 K; HDRY = 4.1098E+05 cm; DAYUBC = 441 DAILY SUMMARY: Day = 352, Simulated Time = 24.0000 hr _____ 2 25 Node Number == 12 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.29808 0.23895 0.24899 Head (cm) = 3.47174E+02 1.67349E+03 1.32408E+03Water Flow (cm) =-1.40435E-01-1.91169E-03 3.93733E-04 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 32.7533+ 0.0000+ 0.0000 - 0.3478- 0.0000- 0.0004 = 32.4051 Versus 32.4051 Mass Balance = -1.5259E-05 cm; Time step attempts = 183 and successes = 183 Evaporation: Potential = 1.3912 cm, Actual = 0.3478 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 62.2 %; TMEAN = 282.3 K; HDRY = 6.5133E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 353, Simulated Time = 24.0000 hr ------= Node Number 2 12 - 25 Depth (cm) = 5.00000 50.00000 122.00000Water (cm3/cm3) = 0.27771 0.24491 0.24893Depth (cm) = Head (cm) = 4.58453E+02 1.45252E+03 1.32573E+03Water Flow (cm) =-1.56933E-01 1.36366E-03 3.92214E-04 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 32.4051+ 0.0000+ 0.0000 - 0.3040- 0.0000- 0.0004 = 32.1007 Versus 32.1007 Mass Balance = -2.2888E-05 cm; Time step attempts = 112 and successes = 112 Evaporation: Potential = 1.2161 cm, Actual = 0.3040 cmTranspiration: Potential = 0.0000 cm, Actual = 0.0000 cmRHMEAN = 55.1 %; TMEAN = 277.0 K; HDRY = 8.1676E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 354, Simulated Time = 24.0000 hr ------2 Node Number = 12 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.26197 0.25210 0.24888 Head (cm) = 5.69084E+02 1.23633E+03 1.32737E+03

Water Flow (cm) =-1.47545E-01 7.77413E-03 3.90720E-04 Plant Sink (cm) =  $0.00000E+00 \ 0.00000E+00 \ 0.00000E+00$ PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 32.1007+ 0.0000+ 0.0000 - 0.2643- 0.0000- 0.0004 = 31.8360 Versus 31.8360 Mass Balance = -5.7220E-06 cm; Time step attempts = 208 and successes = 208 Evaporation: Potential = 1.0572 cm, Actual = 0.2643 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 55.2 %; TMEAN = 279.8 K; HDRY = 8.1379E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 355, Simulated Time = 24.0000 hr _____ Node Number = 2 12 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.24090 0.25819 0.24882 Head (cm)  $= 7.68377E+02 \ 1.08585E+03 \ 1.32896E+03$ Water Flow (cm) =-1.64699E-01 1.53792E-02 3.89258E-04 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 31.8360+ 0.0000+ 0.0000 - 0.4404- 0.0000- 0.0004 = 31.3952 Versus 31.3953 Mass Balance = -6.8665E-05 cm; Time step attempts = 1793 and successes = 1793 Evaporation: Potential = 1.7618 cm, Actual = 0.4404 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 59.1 %; TMEAN = 281.8 K; HDRY = 7.2201E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 356, Simulated Time = 24.0000 hr _____ Node Number Depth (cm) ----2 12 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.22994 0.26249 0.24877 Head (cm) = 9.06028E+02 9.94067E+02 1.33049E+03Water Flow (cm) =-1.69275E-01 2.19179E-02 3.87850E-04 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 INFIL RUNOFF EVAPO TRANS PRESTOR DRAIN NEWSTOR STORAGE 31.3953+ 0.0000+ 0.0000 - 0.2832- 0.0000- 0.0004 = 31.1117 Versus 31.1118 Mass Balance = -1.3924E-04 cm; Time step attempts = 3014 and successes = 3014Evaporation: Potential = 1.1328 cm, Actual = 0.2832 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 76.1 %; TMEAN = 279.8 K; HDRY = 3.7365E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 357, Simulated Time = 24.0000 hr ______

Node	Number	=	2	12	25

Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.22811 0.26515 0.24873 Head (cm)  $= 9.31935E+02 \ 9.42339E+02 \ 1.33166E+03$ Water Flow (cm) =-1.21570E-01 2.61100E-02 3.86614E-04 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 31.1118+ 0.0000+ 0.0000 - 0.2536- 0.0000- 0.0004 = 30.8578 Versus 30.8579 Mass Balance = -1.5068E-04 cm; Time step attempts =13126 and successes =13126 Evaporation: Potential = 1.0145 cm, Actual = 0.2536 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cmRHMEAN = 50.1 %; TMEAN = 273.2 K; HDRY = 9.4659E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 358, Simulated Time = 24.0000 hr _____ Node Number = Depth (cm) = 2 12 25 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.23293 0.26648 0.24869Head (cm) = 8.65520E+02 9.17973E+02 1.33284E+03Water Flow (cm) =-5.62108E-02 2.76312E-02 3.85661E-04 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 30.8579+ 0.0000+ 0.0000 - 0.0861- 0.0000- 0.0004 = 30.7714 Versus 30.7698 Mass Balance = 1.6365E-03 cm; Time step attempts =31460 and successes =31460 Evaporation: Potential = 0.8438 cm, Actual = 0.0861 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cmRHMEAN = 56.3 %; TMEAN = 272.3 K; HDRY = 7.8801E+05 cm; DAYUBC = 4123 DAILY SUMMARY: Day = 359, Simulated Time = 24.0000 hr ______ Node Number=21225Depth (cm)=5.0000050.00000122.00000Water (cm3/cm3)=0.234720.267160.24864---------------------------------------------------------------------------------------------------------------------------------------------------------------Head (cm) = 8.42465E+02 9.05720E+02 1.33437E+03Water Flow (cm) =-2.61356E-02 2.73555E-02 3.84407E-04 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 30.7698+ 0.0000+ 0.0000 - 0.0137- 0.0000- 0.0004 = 30.7557 Versus 30.7558 Mass Balance = -3.2425E-05 cm; Time step attempts = 1090 and successes = 1090 Evaporation: Potential = 0.3685 cm, Actual = 0.0137 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 71.5 %; TMEAN = 270.7 K; HDRY = 4.5959E+05 cm; DAYUBC = 1051 

DAILY SUMMARY: Day = 360, Simulated Time = 24.0000 hr Node Number ----2 12 25 = 5.00000 50.00000 122.00000 = 0.23540 0.26731 0.24860 Depth (cm) Water (cm3/cm3) = 8.33858E+02 9.02953E+02 1.33567E+03Head (cm) Water Flow (cm) =-1.93708E-02 2.60475E-02 3.83248E-04 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 30.7558+ 0.0000+ 0.0000 - 0.0207- 0.0000- 0.0004 = 30.7347 Versus 30.7325 Mass Balance = 2.2011E-03 cm; Time step attempts =32583 and successes =32583 Evaporation: Potential = 0.9538 cm, Actual = 0.0207 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 54.1 %; TMEAN = 272.0 K; HDRY = 8.4111E+05 cm; DAYUBC = 3283 DAILY SUMMARY: Day = 361, Simulated Time = 24.0000 hr _____ Node Number = 2 12 25 Depth (cm) Depth (cm) = 5.00000 Water (cm3/cm3) = 0.23525 122.00000 50.00000 0.26727 0.24855 Head (cm) = 8.35781E+02 9.03744E+02 1.33717E+03Water Flow (cm) =-1.65602E-02 2.41968E-02 3.81931E-04 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 EVAPO TRANS PRESTOR INFIL RUNOFF DRAIN NEWSTOR STORAGE 30.7325+ 0.0000+ 0.0000 - 0.0144- 0.0000- 0.0004 = 30.7177 Versus 30.7178 Mass Balance = -8.3923E-05 cm; Time step attempts = 1763 and successes = 1763Evaporation: Potential = 0.4802 cm, Actual = 0.0144 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 65.6 %; TMEAN = 275.9 K; HDRY = 5.7886E+05 cm; DAYUBC = 1729 DAILY SUMMARY: Day = 362, Simulated Time = 24.0000 hr _____ Node Number 12 25 = 2 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.23498 0.26712 0.24850 = 8.39176E+02 9.06454E+02 1.33855E+03Head (cm) Water Flow (cm) =-1.51478E-02 2.23436E-02 3.80719E-04 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 30.7178+ 0.0000+ 0.0000 - 0.0179- 0.0000- 0.0004 = 30.6996 Versus 30.7016 Mass Balance = -2.0580E-03 cm; Time step attempts =17589 and successes =17589 Evaporation: Potential = 0.6857 cm, Actual = 0.0179 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 60.3 %; TMEAN = 277.6 K; HDRY = 6.9387E+05 cm; DAYUBC = 3196

DAILY SUMMARY: Day = 363, Simulated Time = 24.0000 hr ______ Node Number == 2 12 25 Depth (cm) -5.00000 50.00000 122.00000 = 0.23456 0.26690 0.24845 Water (cm3/cm3) Head (cm) = 8.44445E+02 9.10265E+02 1.33997E+03Water Flow (cm) =-1.38181E-02 2.05021E-02 3.79473E-04 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 DRAIN NEWSTOR PRESTOR INFIL RUNOFF EVAPO TRANS STORAGE 30.7016+ 0.0000+ 0.0000 - 0.0167- 0.0000- 0.0004 = 30.6845 Versus 30.6850 Mass Balance = -4.8256E-04 cm; Time step attempts = 7593 and successes = 7593 Evaporation: Potential = 1.0483 cm, Actual = 0.0167 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cmRHMEAN = 56.2 %; TMEAN = 271.2 K; HDRY = 7.8989E+05 cm; DAYUBC = 2651 DAILY SUMMARY: Day = 364, Simulated Time = 24.0000 hr _____ 2 Node Number = 12 25 Depth (cm) = 5.00000 50.00000 122.00000 Water (cm3/cm3) = 0.23295 0.26680 0.24843 = 8.65245E+02 9.12165E+02 1.34055E+03 Head (cm) Water Flow (cm) =-1.62426E-02 1.89465E-02 3.78407E-04 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 30.6850+ 0.0000+ 0.0000 - 0.0000- 0.0000- 0.0004 = 30.6846 Versus 30.6905 Mass Balance = -5.8079E-03 cm; Time step attempts =81374 and successes =81374 Evaporation: Potential = 0.0000 cm, Actual = 0.0000 cm Transpiration: Potential = 0.0000 cm, Actual = 0.0000 cm RHMEAN = 77.0 %; TMEAN = 267.3 K; HDRY = 3.5867E+05 cm; DAYUBC = 0 DAILY SUMMARY: Day = 365, Simulated Time = 24.0000 hr _____ Node Number = Depth (cm) = 2 12 25 5.00000 50.00000 122.00000  $= 0.23195 \qquad 0.26659 \qquad 0.24841$ Water (cm3/cm3) = 8.78565E+02 9.15958E+02 1.34129E+03Head (cm) Water Flow (cm) =-2.20775E-02 1.78685E-02 3.78144E-04 Plant Sink (cm) = 0.00000E+00 0.00000E+00 0.00000E+00 PRESTOR INFIL RUNOFF EVAPO TRANS DRAIN NEWSTOR STORAGE 30.6905+ 0.0000+ 0.0000 - 0.0471- 0.0000- 0.0004 = 30.6430 Versus 30.6454 Mass Balance = -2.4109E-03 cm; Time step attempts =46475 and successes =46475 Evaporation: Potential = 0.3317 cm, Actual = 0.0471 cm

Transpiration: Potential = 0.000 RHMEAN = 69.3 %; TMEAN = 269.5 K; 1	0 с Н	m, Actual = 0. DRY = 5.0354E4	.0000 cm -05 cm; DAYUBC = 3466
UNSAT- SIMUL	H V ATI	Cersion 2.03 ON SUMMARY	
Title: Muskogee AFC Alternative Final Cov	er		
Transpiration Scheme is:	=	1	
Potential Evapotranspiration	=	1.8318E+02	[ cm ]
Potential Transpiration	===	5.5447E+02	[cm]
Actual Transpiration	=	5.0138E+01	[cm]
Potential Evaporation	=	1.7826E+02	[ cm ]
Actual Evaporation	==	2.0543E+01	[ Cm ]
Evaporation during Growth	=	5.8760E+00	[ cm ]
Total Runoff	=	1.8631E+01	[ Cm ]
Total Infiltration	=	7.0929E+01	[ Cm ]
Total Drainage at Base of Profile	=	4.7357E-01	[ Cm ]
Total Applied Water	===	8.9560E+01	[cm]
Actual Rainfall	==	8.9560E+01	[cm]
Actual Irrigation		0.0000E+00	[cm]
Total Final Moisture Storage	2002	3.0645E+01	[cm]
Mass Balance Error	222	-2.4141E-01	[cm]
Total Successful Time Steps	=	3792940	
Total Attempted Time Steps	===	3792940	
Total Time Step Reductions (DHMAX)	=	5994	
Total Changes in Surface Boundary	=	369002	
Total Time Actually Simulated	=	3.6500E+02	[days]

Total water flow (cm) across different depths at the end of 3.6500E+02 days:

DEPTH	FLOW	DEPTH	FLOW	DEPTH	FLOW
0.000	5.0386E+01	2.500	5.0388E+01	7.000	3.1715E+01
10.500	2.3062E+01	13.500	1.7996E+01	16.500	1.4468E+01
19.500	1.1913E+01	23.000	9.6498E+00	27.500	7.5851E+00
32.500	5.9489E+00	37.500	4.4827E+00	45.000	2.1859E+00
55.000	3.3955E-01	65.000	3.8104E-01	75.000	4.1270E-01
82.500	4.2959E-01	86.500	4.3685E-01	89.750	4.4230E-01
91.750	4.4503E-01	94.000	4.4823E-01	98.000	4.5303E-01
105.000	4.6003E-01	112,500	4.6645E-01	116.500	4.6948E-01
120.000	4.7203E-01	122.000	4.7357E-01		

## **APPENDIX C**

## DAILY CLIMATOLOGICAL DATA SUMMARY

me Zone: Midnight-Midnight CST unty: Tulsa evation: 604 feet	h) SOLAR 4" SOIL TEMPERATURES X (MJ/m2) SOD BARE MAX MIN	88 100001432 10 100320 10 100000 10000					4# # # # # # # # # # # # # # # # # # #	4*	32 1 12.24 37 7 37 6 44 34			6 8.60 36.1 36.1 42 34		1		L* 11.38* 38.9 40.6 46 37			2011月1日 - 11月1日 - 11日日 - 1		**************************************	With: Rainfall > 0.01 inch: 15*	Rainfall > 0.10 inch: 7* Avg Wind Speed > 10 mph: 2* Max Wind Speed > 30 mph: 2*	* Denotes incomplete record
E C I	WIND SPEED (mp DIR AVG MA	SSW NEW DIMENSION		ESE 2.4 11.		WWW 3.0 14.	LESE* 5.3* 14. SSE* 5.3* 14.		WNW 6.0 18.			N = 5.2 19	SE 3.2 11.	S30 2.8 11		N * 5.5* 19. Reduk 8 1% 36	N  11.5  30.	NNE 5.6 14.	SM 9 1 31	INWINE E ELZI	N * 51.6* 31	Number of Days Tmax > 90: 0*	Tmin < 0: 2* Tmin < 32: 25* Tmin < 0: 1*	
001 1.0 NE Bixby 51-57	PRESSURE (in) STN MSL			29.51 30.16		29.55 30.20			29.37 30.02	+ yc ve + Ly oc   		29.45 30.10	29.69 30.34	29.713 30.37		29.69* 30.34*	29.44 30.09	29.37 30.02	29.00 29.64	123745 30.10	29,534,30,184	HDD: 930* CDD: 0*	100* 18*	
January 2 Nearest City: 2 Longitude: 95-	IUMIDITY (%) RAIN MAX MIN AVG (in)			97 58 85 0.31	95 38 71 0.20	98 40 76 0.00		100* 93* 97* 0.01* 100* 93* 97* 0.01*		98* 72* 85*1 0 00		0.012 12 0.004	94 34 65 0.00				86 37 61 0.00		81 39 61 0.00		MUN Averages -> 1	egree Days - Total Total	umidity - Highest: Lowest:	vеу
TA SUMMARY	DEG DAYS I HDD CDD	145.100 5-3		34 0	24 0		* 29* 0*	* 30 * 0 *		* 33* 0*					* 07 + 0+   * 07 + 0+	<u> </u>	22 0				- Mont	I *C	4.40 in. F 2.07 in.	cological Sur "2
IET CLIMATOLOGICAL DA' 3) Bixby :ude: 35-57-44	TEMPERATURE (°F) MAX MIN AVG DEWPT	24 0 12.7 8 9		48 14 29.4 25.1 56 22 35.7 26.8	57 25 39.3 29.5 43 33 38 0 31 3	49 25 35.1 27.3 45 22 35.1 27.3	45* 28* 36.1* 30.0*	43* 27* 36.9* 36.2*		37* 27* 33.1* 29.0*	46 28 31 8 28 9 46 28 34 8 26 4	<u>เหลือหาอี่นี้หาอี่สำคัญ</u>	36 12 24.2 13.2		500 00 00 00 00 00 00 00 00 00 00 00 00	<u>VIEOXIZOXIZENT 22 ION</u>	57 28 45.9 32.4 39 54 35 6 52.4	43 36 38.6 37.5			45* 25* 34.7* 27.4	rature - Highest: 57 Lowest: 0	all: Monthly Total: Greatest 24 Hr:	, 2001 Oklahoma Climat
MESOI (BIXI Latit	DAY	2 2	<b>E</b>	4 13755	6	8	10 (11)	12 13	14	16	1.8 1.8	16 D.C.	20 33	22	20 24		26 25	28	30			Тетре	Rainf	© 1993

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MESONE: (BIXB) Latitud	T CLIMATOLOGICAL DA Bixby de: 35-57-44	TA SUMMARY	February Nearest ( Longi Fuds	20 21ty: 2.	01 0 NE Bixby	Tin Cou	le Zone: Mí nty: Tulsa	dníght-Mídníght CST	
			anna t Griort	C-CC	/ C - T	. Б.д.С	varion:	004 ICEL	
DAY	TEMPERATURE (°F) MAX MIN AVG DEWPT	DEG DAYS HDD CDD	HUMIDITY (%) MAX MIN AVG	RAIN (in)	PRESSURE (in) STN MSL	WIND SPEED (mph DIR AVG MAX	(MJ/m2)	4" SOIL TEMPERATURES SOD BARE MAX MIN	T
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!	Lowest: 1	* 4	)	Total C	DD: 2*	Tmax > 90: 0*	Rainf	$11 \ge 0.01$ inch: $11*$	
Rainfal	ll: Monthly Total:	4.44 in.	Humidity - Hic	thest: 1	*00	Tmin < 32, 17*	AVC Wind	ALL > U.LU INCA: /* Sneed > 10 mub. 5*	
	Greatest 24 Hr:	2.12 in.	101	west:	27*	Tmin < 0: 0*	Max Wind	Speed $\geq$ 30 mph: 6*	
© 1993, 2	2001 Oklahoma Clima	cological Su	rvey				* Dei	otes incomplete record	٦

ά Powtscript generated on Tuesday, March 27, 2001 at 1/11 j ď 2

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dnight CST	L TEMPERATURES 3ARE MAX MIN	NA NA NA	NAL NA NA NA	NA NA NA NA NA NA	NA NA NA	NA NA NA	NA NA NA NA NA NA	NA NA NA NA NA NA	NA NA NA NA NA NA	NA NA NA	NA NA NA	INALINA NA	538* 57× 50*	53.5 62 47	51.0 57 45		16:30:50 144	45.6 47 44 47 D 47 44	52.2 62 44 33 3 62 44	131.9* (J.5.4) /J.5.*	l inch: 8*	0 1ncn: 2* 10 mph: 3* 30 mph: 4*	omplete record
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001 .0 NE 51-57	PRESSU	29 21	29.13	29.42 29.68	29.63	29.48 29.46	29.26	29.11	29.04	29.44 38.25	29.63	29159	1.29.433	29.29	29.62	79 F9		29.34	29.20	129, 399	HDD: 56 CDD:	+66 19*	
2 City: 2 le: 95-	RAIN (in)	00.00			0.00	0.00	0.00	0.06	0.00	0.00	0.00			0,00			<u>160103</u>	0.05	0.00		Total Total	ghest: owest:	
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A SUMMARY	DEG DAYS HDD CDD	2.7 0		1 18 0	22 0 18	21 0 19 0	16 0 8 0	16 0	12 0 12 0	22 0 53 0	21 0	20 0 0		2 0		25 0	<u>mižoutrov</u>	23 0 19 0	13 0 15 0	noM ->		1.06 in. 0.59 in.	ological Su
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T CLIMATOLOGI Bixby le: 35-57-44	TEMPERATURE MAX MIN AVG	47 29 38.9		56 28 40 40 4	56 29 42.0	55 32 43.3 61 30 44 4	65 34 51.2 64 80 53.6	59 39 50.3	66 40 55.4 53 41 47/0	52 33 41.8	50 39 44.0	55 39 46 8 62 29 46 1	<u> </u>	76 50 62.7 74 44 69 8	59 36 45.5	45 32 40 5 48 32 40 5	<u>ĔŎŦĔŎŦĿĔŎ</u>	45 40 42.1 50 42 45.6	65 39 52.3 62 390 51.8	58*136* 4618	ature - Highei Lowest	ll: Monthly To Greatest	2001 Oklahoma
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3 i xby	RE (in) MSL	29/18/9	29.72 29.76	29.92 29.93	29.77 29.82	29.82 29.79	29.70 79.80	29.98*	29.91	30 27	30.47	30.27	29.80	29,93	29.88 30.10	30.31	第11日本 30-13×	30.14	30.17 30.1833	30.09	2,91,9,8,4	**		
01 0 NE I 1-57	PRESSUN	29.24	29.08 29.12	29.27 29.28	29.13 29.118	29.18 29.14	29.05 59.05	29.34*	29.26	29.61 29.61	29,82	29.62	29.15	29.128	29.23 29.45	29.65 24.65	29.48*	29.49	29.52 29.52	29.44	291334	DD: 100	99* L5*	
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TOLOGIC 57-44	RATURE AVG	11233.001	64.6	73.8 1073.77	75.7	75.5 8076/08/9	73.9	* 56.3*	62.7	53.2	47.0	52.6 64.0	72.4	31.71.12 	(1.3	55.7	* 68.3*	69.8	67.2 168/03	68.3	*); 6(6,1,0 <u>)</u> *	Híghes Lowest	thly To atest 2	lahoma
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MESONEJ (BIXB) Latituć	DAY				6	8 11110 11110	10	12 [13]0	14	16 16		18			<u>23</u>	24 110-24	26	27.1	28 129	30		Tempera	Rainfal	0 1993,2

MESONE (BIXB) Latitu	st CLIMATOLOGICA. Bixby de: 35-57-44 remberarine (	L DATA	SUMMARY	May Nearest Longitu	2 : City: 2 ide: 95-	001 .0 NE Bixby 51-57	111 CO Ble	ne Zone: Mi inty: Tulsa svation:	dnight-Midnight C 604 feet	ST
DAY	MAX MIN AVG DI	EWPT		DAN NIM XAM (%) YTTUIMUH	rAIN (in)	PRESSURE (in) STN MSL	WIND SPEED (mp) DIR AVG MAN	I) SOLAR (MJ/m2)	4" SOIL TEMPER. SOD BARE MA.	ATURES X MIN
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0 <u>1111</u>	77 60 69.3 6	52.1		94 63 78		29.23 29.87	SSE 8.6 23.2	14.79	68.2 69.4 7	3
8	81 45 64.4 5	51.8	2 0 2	99 34 69		29.51 30.17	NNE 7.0 23 6	24.23		Distance 3
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16	86* 68* 77.2* (	57.7*	0* 12*	85* 61* 73	* 0.00*	29.08* 29.73*	S * 7.7* 20.5	*  23.69*		
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	800 800 800 F	8.8°	ION +>+	thuy Average		29:29* 29 93*	S. * . 6.8* 50.3	* 21 35 *	69.8* 72.2* 8	)*\!!66*
Тетрег	ature - Highest: Lowest:	: 87* 45*		Degree Days	- Total I Total (	HDD: 26* CDD: 157*	Number of Days Tmax > 90: 0*	With: Rainf:	all > 0.01 inch: ]	
Rainfa	11: Monthly Tote	al: 6	5.92 in.	Humidity - H	iqhest:	99*	Tmin < 32: 0*	Rainf: Avr Wind	all > 0.10 inch: ] Sneed > 10 mmh.	* 0
	Greatest 24	Hr: 1	1.47 in.		Lowest:	25*	Tmin < 0: 0*	Max Wind	Speed > 30 mph:	: * າ ຕຳ
0 1993,	2001 Oklahoma Cl	limato]	logical Su	гvеу				* Der	lotes incomplete r	cecord
Postacript gen	ersted on Monday, June 25, 2001	at 9,16								

at 9,16 2007 25,

idnight-Midnight CST a 604 feet	4" SOIL TEMPERATURES SOD BARE MAX MIN	000 13 10 10 10 10 10 10 10 10 10 10 10 10 10	25.9 76.6 82 73 75.9 76.6 82 73 75.9 76.75	77.9 78.2 85 72 177.8.3 78.3 84	79.2 79.3 87 73 800.1 80.80 89 89	79.7 81.6 90 74	79.8 83.8 92 76	78.2 81.4 87 76	77.3 78.3 87 71 77.6 778 86 773	77.8 80.2 90 71 761 80 80 7	78.0 80.6 84 77	76.4 78.1 85 71	78.7 83.3 91 76   79.00 84.1 91	79.0 84.3 91 78 77 85.0 92 78	75.3 78.7 85 76	78.5 79.9 86 75	11,175 5. 119,171,187 1173	Eall > 0.01 inch: 7	$3 \text{ Speed} \geq 30 \text{ mph}: 3$	snotes incomplete record
me Zone: M unty: Tuls evation:	h) SOLAR X (MJ/m2)	2 29.31 0 29.72	94 22.97	5 26.92 9. 26.04	9 24.26 4 826 40	7 26.33 20 20 05	9 29.80 0 23.41	5   11.66 2 [[29.75]	7 30.87. 7 25.46	6 30.40	2 20.10	0 27.68 9 29.30	6 29.46 9 29.47	6 27.75 50 127.31	2 8.09 9.19.75	1 21.63	0.1 255.06	With: Rainf	Avg Wind Max Wind	* De
T1 CO E1	ND SPEED (mp. R AVG MA	V 14 5 16		5.4 16.	$4 \cdot 0  23 \cdot 10$	5.2 19.	3 11.4 26. 1 11.4 26.	10.2 30. V 3.0 100	3 6.5 18.	8.2 24.	5.6 25.	3 3.2 17. 3 4.20114.	V 5.0 20.	3 4.9 17. 14.6 16.	8.0 30.	3 4.5 16.	1	wher of Days ax > 90: 8		
Bixby	RE (in) WIN MSL DIF	29.187. [S91 29.78 [NNF		29.92 SE	30.00 ENE	29.88 S	29.66   SSF	29.72   S 30.01 -   WSN	30.13 SSF	30.04 S	30.12 E	30.08 NNE	30.07 SSV	30.13 SSF	30.08 SE	29.96 SSF	29, 96 (1) 388			
2001 : 2.0 NE 95-51-57	LN PRESSU 1) STN	00 29 22	0 29.03	00 29.28 00 29.22				39   29.08 01   29.36	00 29.48 00 29.49	00 29.39 00 29.41	2 29.47	00 29.43 00 29.39	00 29.42 00 29.44	00 29.48	50 29.43 6 29.34	10 29.31	6 1 29 31	ul HDD: 11 CDD: 34	.: 99 .: 31	
me arest City ngitude: 9	Y (%) RAJ N.AVG (ir	8 76 0.0		8 72 0.0				2 82 0.E	$\frac{4}{159}$ 0.0	2 62 0.0	8 78 0.0	6 69 0.0	0 67 0.0 0 66 00	<u>1 68 0.0</u> 2 70 000	3 87 1.5 51 822 1000	0 81 0.0	erages ->	Days - Tota Tota	Y - Highest Lowest	
Lo Lo	TIUMIDIT MAX MI	11. (1999) 4 8 86 3 5 86 3	7 84 6	3 90 4 3 58 4				2 96 6 0 99 33	$\begin{array}{c c} 1 & 91 & 3 \\ \hline 2 & 0 & 0 \\ \hline \end{array}$	3 82 4 4 89 4	2   96 5 6   96 5	6 99 3 91 0 98 30	$\begin{array}{c c} 1 & 97 & 4 \\ \hline 1 & 96 & 4 \end{array}$	1 95 4 3	7 95 7 0 0 95 7	2 96 6	Monthly Av	Degree	. Humidit	Survey
DATA SUMMAH	DEG DAY PT HDD CI	.3 .3 .3								.9 0 1 8 8 0 1	.0 0 1 .8 1 1 0 1	. 8 . 9 . 0					1	91 50	: 3.07 in c: 1.50 in	atological * 3:40
LLIMATOLOGICAL 1 xby 35-57-44	EMPERATURE (°F) MIN AVG DEWI	68 72.0 57. 68 72.0 57.	77 81.3 71.	69 78.1 67.	ска та с 70 68 та с 70					67 79.0 63.	69 76.7 69.	57 71.9 58.	63 77.4 63.	63 77.4 64.	68 71.4 67.	/0 /6.8 /0.	66761919.66	re - Highest: Lowest:	Monthly Total: Greatest 24 Hr	1 Oklahoma Clin on Madmeeday, July 25, 2001
MESONET C (BIXB) Bi Latitude:	T DAY MAX	2 78 2 78		6 87 87 89						18 90	20 85	22 84 323 84	24 90	26 88	28 76	CB UC	8.1	Temperatu	Rainfall:	© 1993,200 Postscript generated

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Midnight-Midnight CST Ja 604 feet	4" SOIL TEMPERATURES SOD BARE MAX MIN	10000000000000000000000000000000000000						827 88 88 88 4 89 8 8 8 8 8 8 8 8 8 8 8 8 8		81.5 84.4 87 82			84.5 87.2 92 83 85.0 88			88.000000000000000000000000000000000000	85.9 89.6 94 86 86 3 80 1 94 86	85.7 89.4 93 86		84.075.87757 832 84.8 88.3 92 85 85.1 80.0 85			fall 2 0.01 inch: 2	d Speed > 10 mph: 0 d Speed > 10 mph: 0 d Speed > 30 mph: 1	enotes incomplete record
me Zone: N wurty: Tuls evation:	h) SOLAR X (MJ/m2)	8.01.01.01	9 28.30	8 28.34	2 29.23	5 29.37	9 27.19	8 228 41 9 27 05	<u>ám mizi sz</u>	3 21.62	3 21.35.		1 21.75 94 27491	8 27.85	0 28.74	8. 27.90	1 27.48	4 21.47	6 23.17	9 26.31 5 26.31 5 27 20	94 14 25 457	With:	Rain	Avg Win Max Win	Ř *
Ti G G G	VIND SPEED (mp JIR AVG MA		3 4.7 16.	3SW 5.8 18.	5 6.3 19.	SW 6.9 20.		3. 5.8 18.	INEX 947 201	ISE 5.9 17. 18/2017-31	SE 9.9 23.		8.0 20.	SSE 6.3 19.			SE 4.3 1/.	ISE 4.7 13.	SE 6.5 28.	$\frac{1}{2}$ $\frac{1}{2}$ $\frac{1}{2}$ $\frac{1}{2}$ $\frac{1}{2}$ $\frac{1}{2}$ $\frac{1}{2}$ $\frac{1}{2}$		lumber of Days	max > 90: 26	min < 0: 0	
01	PRESSURE (in) V STN MSL I	294,381,1301,03,41	29.44 30.09	29.39 30.04 8	29.33 29.98 2	29.33 29.98   5	29.23 29.88 2	29.22 29.87 [5	13341 29195 N	29.35 30.00 1	29.22 29.87 8	29.26 29.591	<u> </u>	29.30 29.95 5	29.18 29.82 5		<u> </u>	29.28 29.92 F		29.28 29.93 5 29.28 29.93 5 29.35 30 00	29,130,129,941	D: 0 N	D: 610 1	6 9	
Ċ±ty: 2. de: 95-5	RAIN (in)	in the store of the		0.00	0.00	0.00	0.00	0.00			0.18			0.00	0.00			0.00	0.06			- Total HI	Total CI	lghest: 5 jowest: 3	
July Nearest Longitu	HUMIDITY (%) MAX MIN AVG	0.914.54.60	99 50 75 98 51 73	97 39 67 94 54 72	97 47 72		95 40 70				93 56 75		<u></u>	89 44 66	89 36 58	97 30 67 58		95 44 71	92 44 73	92 40 64	L uthly Average	Degree Days		Humidity - H	ırvey
A SUMMARY	DEG DAYS HDD CDD	<b>TELEVIOLE</b>		0 18	0 20	0 20	0 20				0 19		1211011123		0 22				0 22	0 23	IOM =>			0.24 in. 0.18 in.	ological Su
GICAL DAT 4	RE ( [°] F) G DEWPT	19.11711.14	.6 /0.0	.7 71.2 1911,121,181	.2 74.3	.1 72.2	.8 72.7	.2 73.4	7 бт П		.6 73.6	.0 74.6	17.12.17	.0 71.4	.5 70.5	.5 73.3	10 11 2 2 1	.9 73.8	.7 73.5	.9 73.0	.7 .722	1est: 101	est: 66	Total: : 24 Hr:	na Climato
JET CLIMATOLO 3) Bixby .ude: 35-57-4.	TEMPERATU MAX MIN AV(	0.000			97 73 85	97 74 86	97 72 84 97 73 86	101 77 88	86 70 77	aughan 66 an th	94 74 82	98 76 88.	297 79 187 2			100 75 87.	6.0 1 1 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	9/ /5 84, 1951 78 850	97 77 83.	100 76 87 99 777 88	95 74 84	rature - High	ГОМЕ	all: Monthly Greatest	,2001 Oklahon
MESON (BIXE Latit	DAY			4 300.50	6 6	8	10	12	14 14	15 E	16	18	6 0 0	21.21	22	24	22	12.7 C	28 4150	30 31.		Tempe.	ŕ	Rainf	© 1993

Thuxsday, August 23, 2001 at 16,50

MESON (BIXB Latit	ET CLIMATOL( ) Bixby ide: 35-57-4	JGICAL DATI	A SUMMARY	Augu Near Long	st est Cj itude:	20 1ty: 2.	101 1-57	lixby			Time Count Fleva	Zone: Mić Y: Tulsa Fion: 6	lnight-M 504 feet	lidnight	CST	[
DAY	TEMPERATI MAX MIN AV	JRE (°F) /G DEWPT	DEG DAYS HDD CDD	HUMIDITY MAX MIN	(%) AVG	RAIN (in)	PRESSUE STN	KE (in) MSL	WIND DIR	SPEED ( AVG	(mph) MAX	SOLAR (MJ/m2)	4" SC SOD	IL TEMPE BARE M	RATURES AX MIN	10 17
	19.9 1 72 80	1.9. 69.8	11.00 × 20.01	17E 26	58.0	10.0.0	129/45	30110 E	<b>BSB</b>	5.77	6.23	27.65	85.2	21.3 Million	94 1 85	
2	97 74 86	5.3 71.9	0 21	88 40 66 75	64 82	0.00	29.49	30,14	SSE	5.8 1	7.9	26.43	85.8	89.9	94 86	
4.000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000	101 70 85	5.6 69.0	0.20	94 30	63	0.00	29.32	29.97	ESE	2.6 1	2.3	27.19	85,2	89.6	94.85	
100	1001 74 W 8		0		1611	00.00	29.35	30100	E CONTRACT	E 6 6	T. 4.	27.26	186.0.S	90.h3	9.5	巖.
	99 99 94 97 70 84	t.6 66.2 [.00703]	BT 0	94 29 15 40	58 677 8	0.00	29.42 29:42	30.07	SE	1.9.1 13.00	.6.1 7.9	26.84	85.2	89.8	94 86 94 86	
8 8 8	98 73 86	5.4 70.0	0 20	92 38	61 19	0.00	29.32	29.97	S	4.5 1	9.0	23.24	85.4	89.7	93 86	
10	98 74 82	3.9 71.6	0 21	96 40	72	0.28	29.32	29.97	л Н	4.2 2	64.0	21.16	85.3	89.2	93 86	<u>ب</u>
TT	188 233 23		<b>EXERCISE 16</b>	66	<b>84</b>	0.00	2.9.136	30401	NEW	22,012,01	22.02	LA DA	6318	1951/1	8.8	
12 (113)	94 73 8j	L.8 73.4	0 19	<u>97 46</u>	78	0.00	29.41	30.06	N	3.4 1	5.5 22	20.74	84.9	85.9	91 82	
14	95 66 80	0.9 63.0	0 15	93 28	59	0.00	29.37	30.02	SSE		2000 0000 0000 0000 0000 0000 0000 000	26.68	83.3	87.0		
12	14.001196	31X51.68.40	ALL OT STATE	561143		06.73	<b>WZ9421</b>	29, 85	COB COB		12148	17.56	<u>800.14</u>	<u>0.65.14.000</u>	89.4482	
16 378/99/2010	91 68 7{	9.1 68.1	0 15	99 37	76 862-00	0.00	29.35	30.00	ENE	3.1 1	1.2	24.50	81.8 orange and a second	83.0	88 79	
	CA CA CA AC				73			20 05 W	NGS N			24169	82.12 0 0 0	01.7		<u> </u>
and the second		aismed in		84 33		0000	29:29	29:94	ENE		4.4	26.25	02.0 	0.0.0 1.86.5	93 500	
20	98 67 82	2.8 65.6		88 32	60	0.00	29.29	29.94	S	6.5	21.6	23.31	81.3	86.0	92 81	ار_ ا
						0:00	29130	29,95		9.12		23,85	<b>81.</b> .8		91.1.82	A.
203		0.5, 00.5				0.0.	29.30 ×	29.95	* 2	9.4× 2	(4.5×	×00.22	82.0×	86.6×		<u>ل</u>
24	92 73 82	2.3 68.0	0 17	86 46	63	0.07	29.26	29.91	SSE	8.7 2	26.2	16.25	79.4	80.5	84 78	<b>_</b>
読みせる語						0149	0.0.0257	29.190.50			6.6	16.53	6.62	82.2	88	
27	92 69 /: 84 71 80	9.3 /0.3		94 48	/ 6 1174	0.00	29.34	29.99	SSW	4.0 ]	1.1 9.6	22.32	$\frac{81.1}{82.4}$	82.2	89 77 94 4 8	
2.8	93 68 81	1.2 70.0	0 T C	98 40	72	0.00	29.30	29.95	S	3.8 ]	5.4	24.74	82.1	86.3	94 79	<b>~</b>
0 r				00 00			2.01.20	29.94				233024	81.12		92113180	
	89 / 3 /		0 14   01234	90 62	( p	0.00	62.62	29.90	NE		(J.6	20.60	80.7	د.ده های ۵.۸۵		
	95*171* 8	3.15 × 6.8 × 8.4	19WU A Street	luhuy Aver	agee		1219/133.4	29,984	S	11.1	2.8*	22,864	83,04	9.8 S X 1	92 <del>+</del> 82	
Tempe	rature - Hig Lov	ghest: 104 vest: 66		Degree Da	л. Т.	Total H Total C	IDD: (	* (	Numbe Tmax	1 0f Da 90: Da	175 W1 28* 0*	th: Rainfa Dainfa	11 <u>&gt;</u> 0.	01 inch:	5 ★ ★ ★	
Rainf	all: Monthly Greater	Y Total: st 24 Hr:	1.89 in. 0.73 in.	Humidity	- Higl Lou	hest: west:	99* 26*		Tmin Tmin		* *	Avg Wind Max Wind	Speed >	10 mph: 30 mph:	* * * H O H	
€ 1993	, 2001 Oklaho	oma Climato	ological St	лгиеу								* Den	lotes in	complete	record	

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MESONF (BIXB) Latitu	ET CLIMATOLOGICAL DAT Bixby de: 35-57-44	A SUMMARY	Septembe Nearest ( Longitude	c 20 Jity: 2.	01 0 NE Bixby 1-57	ΗŪΜ	ime Zone: M Dunty: Tuls levation:	idnight-Midnight CST a 604 feet
DAY	TEMPERATURE (°F) MAX MIN AVG DEWPT	DEG DAYS HDD CDD	HUMIDITY (%) MAX MIN AVG	RAIN (in)	PRESSURE (in) STN MSL	WIND SPEED (m DIR AVG M	ph) SOLAR AX (MJ/m2)	4" SOIL TEMPERATURES SOD BARE MAX MIN
				0000	29.1251.1291.901	ESEL 210 114		
	92 67 78.5 94 68 80.0 68.9	GT 0	98 42 /5		29.20 29.85 29.23 29.87	SE 3.4 L3   E	24.31	80.9 86.5 93 80 81.0 86 5 92 81
4		0 15	95 45 73	0.00	29.32 29.97	ESE 3.0 12	.9   16.01	
	R 72 79 0 70 7	0.15	97 55 85 97 53 77	10 03 0	20003410290999 29 27 29 87		1 1 1 6 8 5	
N NO LANSING	91 66 79 9 70 10 10	and the second second	2. 23 23 23	<b>WEAD</b>	<u> </u>		<u>iên nîti</u> čiya	
8 8	77 62 69.2 64.4	0 4	99 69 85	0.86	29.13 29.78	ESE 3.9 22	.4 8.91	76.0 76.8 80 73
		0 0	99 33 73	0.00	29.57 30.22	SE 2.0 13	.7 24.64	
	184 52 607 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	0.010	170.01.39.74 M	0.0.0	29.52.30.17	ESENTIAL	1. 23.83	1997 1997 1997 1997 1997 1997 1997 1997
12 113	86 55 69.0 58.7	05	99 36 74	0.00	29.43 30.08		.6 22.79	
14		6 0	800 46 76		29.39 30.04		.7 1 16.82	III MOORTANIA IN THE TAKE A PARTICULAR AND AND A PARTICULAR AND AND A PARTICULAR AND A PART
1019 III	1.76 66 70 15 65 4		0.00 M 65 84 M	v.0.26	29,07,130,02,0	SSE. CONCEPTER	15. 17.181	1 10 74 13 10 15 19 179 179 173
16 25	77 68 71.6 68.4	0 7	98 78 90	0.18	29,35 30.00	<b>ESE 4.8 12</b>	.8 6.02	74.3 74.7 78 72
				EONO.				
T B T B	// 55 /1.4 57.5		98 75 88 94 49 76	1.04	29.20 29.84	1 SW 7.0 29	.6 10.54	<b>74.7 75.2 78</b> 72
20	77 58 67.0 62.9	0 3	99 67 87	0.00	29.32 29.97	SSE 4.8 35	.6 8.80	72.8 71.5 76 68
21m	1.80 62 7012 65 6		98 64 96	0010	291.0.8 13.0 103.1	BSE 3.3.9	.9 13, 20	1.173.77.1.73.1.73.1.79
22 mm22	85 60 70.6 66.2		100 58 88	0.00	29.37 30.02		.0 16.97	74.0 74.5 82 68
				0.01				
24 385-6 000	69 50 60.8 41.9		88 43 54	0.00	29.5/ 30.23	INNE 9.9 ZI	TC./T 8.	
26	77 40 58.1 48.6		99 38 76	0.00	29.49 30.14	SSE 2.0 12	.2 21.56	67.6 67.9 78 60
27.2	7.8. 48 162.3 51.4	N. 12 3 10 10	EL 02 130	0.00	29:43 30:08	SSE 3.16 16	6 20.50	67.3.4.68.4.75.1.62
28	80* 50* 63.8* 53.8*	0* 0*	99* 37* 75*	*00.0	29.44* 30.09*	ESE* 2.7* 14	.0* 20.27*	67.5* 69.9* 77* 64*
30	78 44 59.7 49.5			0.00		ESE 1.9 10	.8 20.94	66.1 69.0 76 63
	82* 59* 690,0* 610,2*	IOW CENTRAL INC.	hthuy Averages		29.374.30.024	ESE* 41.77.4.46	8.4 1.778514	174 44 15 07+ 82+ 72+
тещрел	rature - Highest; 94 Lowest: 40	* *	Degree Days -	Total I Total (	IDD: 29* IDD: 194*	Number of Day Tmax > 90: 5	s With: * Rain * Dain	fall > 0.01 inch: 10*
Rainfe	all: Monthly Total: Greatest 24 Hr:	2.82 in. 1.04 in.	Humidity - Hi Lo	ghest: ] owest:	30*	Tain < 0: 0	* Avg Win * Max Win	d Speed > 30 mph: 2*
O 1993,	2001 Oklahoma Climat	ological Su	ırvey				Ŭ *	enotes incomplete record

Et .	TURES MIN		64 53	68 68	52 58		60 111111111111	64	63	62	57		50	11975-040 7.2	53 1875 55	56	60 61	110 640	59 14 - 23	л. С.С.	491	51	00 1		H-H-H-H-H-H-H-H-H-H-H-H-H-H-H-H-H-H-H-				<u>.</u>	scord
dnight CS	L TEMPERA BARE MAX	2.4.6.10.0.9.2	68.5 74	70.9 75	57.8 69 52.8 69	az jan ka	54.6 69 46 81 69	56.3 68	53.18	54.8 68	51.0 66	38.17 6.2	55.6 62			60.9 67		36.8 271	54.0 67 54.0 67		54.31.160	54.9 60	57.00 162 17 E					L inch:	LO mph: 30 mph:	omplete r
lnight-Mi 504 feet	4" SOI		65.8	68.3 °	62.1 (		62.4 (	65.8 (	64.15	64.8 (	62.0 (		58.0		59.5	60.8 (		165.3	63.8 (		E C T T	56.1	E7 1 5		R. R. B. Martin		r r		Speed > 1	otes inco
Zone: Mic :Y: Tulsa ition: (	SOLAR (MJ/m2)	NO NEW KINESS	20.33	15.82	20.21		18.65	1.14			17.06	NICE TIL	18.81		1. 96.91	15.48	70 07	<u>TĂNÔ6 MI</u>	16.68	16.65	C. C. C. D. D. C.	15.93		70.CT			th:	Kainta Fainfa	Avg Wind Max Wind	* Den
Time Count Eleva	(mph) MAX		25.3	25.2	12.5		21.9	32.4	E TE	32.4 33 33	14.7	41.41	14.2	ר אניים איז אניים		23.3		24.3	30.8	15.2	12:17:1	25.5					Days Wi	э с	0 M O	
	D SPEED AVG		6.0 6.0	9.3	2.0			7.4		0.1	3.5	10.13	2.5		<u>. 5. 5</u>	5.6	с р г		8.9 8.9	ananana sa	NULL CONTRACTOR	9.4	с у 1911 р.		2.00 C 2.8		oer of	× × ×   ×	 	
	DIR		ß				N S S E	S		NNW NNW	ESE	N.	MNM		MESE	S		S IS	MNN	NE	BE	ر م	U D	2 S	S S		Imun Num	Tmax	Lint Lint	
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* Denotes incomplete record

#### MUSKOGEE COMMUNITY RECYCLING AND DISPOSAL FACILITY MUSKOGEE COUNTY, OKLAHOMA ODEQ PERMIT NO. 3551020

## TIER I PERMIT MODIFICATION

### ALTERNATIVE FINAL COVER QUALITY ASSURANCE/ QUALITY CONTROL PLAN

Prepared for

Waste Management of Oklahoma, Inc.

Approved October 18, 2008

**Revised October 2023** 



Prepared by

Weaver Consultants Group, LLC CA 3804 PE 06/30/2023 6420 Southwest Boulevard, Suite 206 Fort Worth, Texas 76109 817-735-9770

WCG Project No. 0086-364-11-19

# CONTENTS

LIST	COF TA	BLES	D-ii
1	INTF	RODUCTION	D-1
	1.1	Purpose	D-1
	1.2	Definitions	D-1
2	CON	STRUCTION QUALITY ASSURANCE FOR EARTHWORK	D-4
	2.1	Introduction	D-4
	2.2	Earthwork Construction	D-4
		2.2.1 Intermediate Cover Layer	D-4
		2.2.2 Vegetation Support Layer	D-5
		2.2.3 Vegetation Layer	D-6
		2.2.4 Establishment of Vegetation	D-6
	2.3	Survey and Final Topography	D-7
	2.4	Construction Testing	D-7
		2.4.1 Standard Operating Procedures	D-7
		2.4.2 Test Frequencies	D-7
	2.5	Reporting	D-8
3	DOC		D-9
	3.1	Preparation of FCCR	D-9
		10/13/2023	

<u>Table</u>	<u>e</u>	<u>Page</u>
2-1	Recommended Tests and Observations for the Vegetation Support Layer	D-8

## 1.1 Purpose

This Quality Assurance/Quality Control (QA/QC) Plan has been prepared to provide the Owner, Design Engineer, Construction Quality Assurance Professional of Record, and the Contractor the means to govern the construction quality of the Evapotranspiration (ET) monolithic soil alternative final cover system for the Muskogee Community Recycling and Disposal Facility (RDF). This plan has been prepared consistent with OAC 252:515 and the plan addresses the soil components of the ET monolithic soil alternative final cover system.

This QA/QC Plan is divided into the following parts:

- Section 1 Introduction
- Section 2 Construction Quality Assurance for Earthwork
- Section 3 Documentation

## 1.2 Definitions

Whenever the terms listed below are used, the intent and meaning shall be interpreted as indicated.

### ASTM

American Society for Testing and Materials.

#### Construction Quality Assurance (CQA)

A planned system of activities that provides the Owner and permitting agency assurance that the facility was constructed as specified in the design (EPA, 1986). Construction quality assurance includes observations and evaluations of materials, and workmanship necessary to determine and document the quality of the constructed facility. Construction quality assurance (CQA) refers to measures taken by the CQA organization to assess if the installer or contractor is in compliance with the plans and specifications for a project.

### **Construction Quality Assurance Professional of Record (POR)**

The POR is an authorized representative of the Owner and has overall responsibility for construction quality assurance and confirming that the facility was constructed in general accordance with plans and specifications approved by the permitting agency. The POR must be registered as a Professional Engineer in Oklahoma and experienced in geotechnical testing and its interpretations. Experience and education should include geotechnical engineering, engineering geology, soil mechanics, geotechnical laboratory testing, construction quality assurance, and quality control testing, and hydrogeology. The POR must show competency and experience in certifying like installations, and be approved by the permitting agency, and be presently employed by or practicing as a geotechnical engineer in a recognized geotechnical/environmental engineering organization. The credentials of the POR must meet or exceed the minimum requirements of the permitting agency.

The POR may also be known in applicable regulations and guidelines as the CQA Engineer, Resident Project Representative, or the Geotechnical Professional (GP).

### Construction Quality Assurance (CQA) Monitors

These are representatives of the POR who work under direct supervision of the POR. The CQA monitor is responsible for quality assurance monitoring and performing onsite tests and observations. The CQA monitor is on site full-time during construction and reports directly to the POR. Field observations, testing, or other activities associated with CQA may be performed by the CQA monitor(s) on behalf of the POR.

#### **Contract Documents**

These are the official set of documents issued by the Owner. The documents include bidding requirements, contract forms, contract conditions, specifications, contract drawings, addenda, and contract modifications.

#### **Contract Specifications**

These are the qualitative requirements for products, materials, and workmanship upon which the contract is based.

#### Contractor

This is the person or persons, firm, partnership, corporation, or any combination, private or public, who, as an independent contractor, has entered into a contract with the Owner, and who is referred to throughout the contract documents by singular number and masculine gender.

#### Design Engineer

These individuals or firms are responsible for the design and preparation of the project construction drawings and specifications. Also referred to as "designer" or "engineer."

#### Earthwork

This is a construction activity involving the use of soil materials as defined in the construction specifications and Section 2 of this plan.

#### Nonconformance

This is a deficiency in characteristic, documentation, or procedure that renders the quality of an item or activity unacceptable or indeterminate. Examples of non-conformances include, but are not limited to, physical defects, test failures, and inadequate documentation.

#### Operator

This is the organization that operates the disposal unit (Waste Management of Oklahoma, Inc.).

#### **Operator's Representative**

This is the person that is an official representative of the operator responsible for planning, organizing, and controlling the design and construction activities.

#### Quality Assurance

This is a planned and systematic pattern of procedures and documentation to ensure that items of work or services meet the requirements of the contract documents. Quality assurance includes quality control. Quality assurance will be performed by the POR and CQA monitor.

#### **Quality Control**

These actions provide a means to measure and regulate the characteristics of an item or service to comply with the requirements of the contract documents. Quality control will be performed by the contractor.

#### Final Cover Certification Report (FCCR)

Construction report for the final cover system prepared and sealed by the POR and submitted to the ODEQ.

### 2 CONSTRUCTION QUALITY ASSURANCE FOR EARTHWORK

## 2.1 Introduction

This section of the QA/QC Plan addresses the construction of the soil components of the ET monolithic soil alternative final cover system and outlines the program to be implemented with regard to materials selection and evaluation, laboratory test requirements, field test requirements and treatment of problems.

The scope of earthwork and related construction quality assurance includes the following elements:

- Intermediate Cover Layer
- Vegetation Support Layer
- Vegetation Layer

## **2.2** Earthwork Construction

The following paragraphs describe general construction procedures to be used for various earthwork components of the ET monolithic soil alternative final cover system.

### 2.2.1 Intermediate Cover Layer

After the landfill reaches the permitted grade in each area to receive final cover, operational cover soils will be placed according to the facility operation plan. Twelve inches (min) of soil will be placed by the owner over the completed waste fill prior to installing the ET monolithic soil alternative final cover system.

The contractor will re-work the cover soils (or intermediate cover layer) to provide a smooth, uniformly graded surface upon completion. The intermediate cover layer soil shall be free of rocks and any irreducible material greater than 2 inches in diameter. The intermediate cover layer soil shall be free of clods greater than 4 inches in diameter.

QA/QC for preparation of the intermediate cover layer will be performed under the supervision of the CQA personnel. QA/QC procedures to be performed during preparation of the intermediate cover layer will include observation of the final grading of the intermediate cover layer. Upon completion of intermediate cover layer grading activities, the POR will determine that the intermediate cover layer has been prepared to

provide a uniform surface. Visual examination of the intermediate cover layer preparation by the CQA monitor will generally be sufficient to evaluate its suitability as a foundation for the overlying vegetation support layer soils.

The CQA monitor will approve the prepared intermediate cover layer prior to the placement of the vegetation support layer. Approval will be based on a review of test information, if applicable, and CQA monitoring of the intermediate cover layer preparation.

Surveying will be performed to verify that the completed intermediate cover layer is a minimum 12 inches thick. The intermediate cover layer will be probed on a 100foot grid to verify that a minimum 12-inch-thick soil layer is present at each location. This survey will only verify that a minimum 12-inch-thick soil layer is in place, not the total thickness of the intermediate cover layer. As an option to the intermediate cover layer thickness verification, the vegetation support layer may be installed as a 36-inch-thick, single-lift soil layer installed per the Section 2.2.2 requirements.

## 2.2.2 Vegetation Support Layer

The vegetation support layer will consist of a minimum 24-inch-thick soil layer (measured perpendicular to the intermediate cover layer surface) that will extend along the top and sideslopes of the landfill. The vegetation support layer material will consist of relatively homogeneous clay, silty clay, sandy clay, or clayey sand. Material used for the vegetation support layer shall classify as CL, CH, ML, SM, or SC according to the Unified Soil Classification System (USCS). The soil will be free of debris and rocks greater than 2-inches in diameter. The vegetation support layer soil shall be free of clods greater than 4 inches in diameter. The vegetation support layer material should be placed in one 24-inch lift. The material will be compacted by "tracking-in" the material by 2 to 4 passes of the low pressure earth moving equipment. The earth moving equipment used to place this layer will exert a ground pressure of 16 psi or less.

The vegetation support layer will be placed as single lift and compacted to a density that is between 75 and 85 percent of the maximum dry density determined by standard Proctor (ASTM D 698) at a moisture content less than the optimum moisture content.

Overcompacted vegetative support layer soil will be disked or ripped (or any other method approved by the POR) and recompacted to a density that is between 75 and 85 percent of the maximum dry density. If a density test fails (i.e., density is less than 75 percent or more than 85 percent of the maximum dry density), additional tests may be performed to delineate the overcompacted or undercompacted area. The area to be re-worked will encompass the area between passing density tests.

The testing and survey requirements for the vegetation support layer are listed in Table 2-1. Sufficient testing of the borrow area soils will be completed, as needed, to verify that the soils to be used will be classified as CL, CH, ML, SM, or SC according to the USCS.
The vegetation support layer construction will be conducted in a systematic and timely fashion. Delays will be avoided in completion of the vegetation support layer and construction of the overlying vegetation layer.

Placement of the vegetative support layer will cease during rainfall events to prevent overcompaction. Before proceeding with the construction after a rainfall event greater than 0.5 inches, the contractor will complete at a minimum, a 10-foot by 10-foot test pad (which will be part of the approved ET monolithic soil alternative final cover system after testing is completed) to verify that overcompaction will not occur as construction continues. Test pad results will be reported in the Final Cover Construction Report (FCCR). Compaction requirements will be verified by a minimum of 2 field density tests for the test pad area.

#### 2.2.3 Vegetation Layer

The soil vegetation layer will be placed over the vegetation support layer. This layer consists of soil capable of supporting vegetative growth. The soil is placed in one lift (12-inch minimum thickness) over the entire surface of the final cover system and is compacted in place with a dozer (ground pressure of 16 psi or less). The surface of the vegetation layer should be graded to achieve the desired final grades and disked parallel to the proposed contours, in preparation for seeding and to prevent excessive loss due to heavy rainfall.

The soil cover should be placed under the continuous observation of QA/QC personnel to determine that the minimum thickness is applied and that no damage occurs to other structures (e.g., gas system components) installed in the final cover.

Placement of the vegetation layer will cease during rainfall events to prevent overcompaction of the vegetation layer. This layer will not be reworked after a rainfall event until the CQA monitor confirms that the soil can be effectively disked. To prevent erosion, the CQA monitor will ensure that the procedures detailed in the Vegetation Plan (Appendix E) are followed.

Surveying will be performed to observe that the finished vegetation support layer has been constructed to a minimum thickness of 12 inches (refer to the Table 2-1 for additional information). Waste Management of Oklahoma, Inc., will notify the ODEQ of the source of the soil for this layer at least two weeks prior to construction.

## 2.2.4 Establishment of Vegetation

Vegetation will be established consistent with the specifications listed in the Site's Approval-Vegetation Plan (refer to Appendix E). Final approval of the ET monolithic soil alternative final cover system will occur after Waste Management of Oklahoma, Inc., submits documentation to the ODEQ that demonstrates that vegetation has been established over at least 95 percent of the cover system with no bare areas larger than 1 square foot.

## 2.3 Survey and Final Topography

Upon completion of the installation of the ET monolithic soil alternative final cover system, a final topographic survey of the cover is to be completed by a qualified land surveyor. The final topographic map should be included in the FCCR and should include all final contours, location of gas vents, gas monitoring wells, groundwater monitoring wells, drainage structures, fences and gates, access roads and all other pertinent site features.

A final cover system thickness drawing showing thicknesses for each layer (i.e., intermediate cover layer, vegetation support layer and vegetation layer) and thickness of each layer at each of the survey measurement grid points will be provided. Coordinates defining the perimeter of the final cover system will be called out on one of the final drawings.

## 2.4 Construction Testing

#### 2.4.1 Standard Operating Procedures

CQA monitors will perform field and laboratory tests in accordance with applicable standards specified in the project technical specifications. Standard operating procedures for soil testing will be prepared that describe test procedures and methods used by site testing personnel for the following ASTM test methods. In some instances the standard operating procedure will be prepared or modified by the POR during construction.

The following test standards apply as called out in this manual and in the technical specifications:

<u>Standard</u>	<b>Test Description</b>
ASTM D 698	Moisture-density relations of soils and soil- aggregate mixtures, using 5-½ lb hammer and 12-inch drop
ASTM D 2922	Density of soil and soil-aggregate in place by nuclear methods (shallow depth)
ASTM D 2487	Classification of soils for engineering purposes
ASTM D 2488	Description and identification of soils (visual- manual procedure)

## 2.4.2 Test Frequencies

The test frequencies for vegetation support layer are listed in the following table. Additional testing must be conducted whenever work or materials are suspect, marginal, or of poor quality. Additional testing may also be performed to provide additional data for engineering evaluation. The minimum number of tests is interpreted to mean minimum number of passing tests, and any tests that do not meet the requirements will not contribute to the total number of tests performed to satisfy the minimum test frequency.

Parameter	Frequency	Test Method	
Moisture density relationship (includes Atterberg Limits, No. 200 sieve, and permeability)	1 for each soil type	ASTM D 698	
Soil Classification	1 per soil type	ASTM D 2487 and 2488	
Field Density and Moisture	3 tests per acre	ASTM D 2922 and ASTM D 2216 or ASTM D 3017	
Thickness Verification ¹	A 100-foot grid will be established by a registered Oklahoma surveyor.	Top of intermediate cover layer, top of vegetation support layer, and top of vegetation layer will be surveyed to verify thickness of vegetation support layer and vegetation layer.	

# Table 2-1Recommended Tests and Observationsfor the Vegetation Support Layer

¹ Refer to Section 2.2.1 for additional intermediate cover layer survey requirements.

## 2.5 Reporting

The POR on behalf of the Owner shall submit to the ODEQ a FCCR for approval of each final cover area. Section 3 describes the documentation requirements.

## **3** DOCUMENTATION

The QA/QC Plan depends on thorough monitoring and documentation of all construction activities. Therefore, the POR and CQA monitor will document that all quality assurance requirements have been addressed and satisfied. Documentation will consist of daily recordkeeping, testing and installation reports, nonconformance reports (if necessary), progress reports, photographic records, design and specification revisions, to be included in the FCCR. Standard report forms will be provided by the POR prior to construction.

## 3.1 Preparation of FCCR

The POR, on behalf of the Owner, shall submit to the ODEQ an FCCR for approval of each final cover area. The construction methods and test procedures documented in the FCCR will be consistent with this QA/QC Plan.

At a minimum, the FCCR will contain the following:

- A summary of all construction activities
- A summary of all laboratory and field test results
- Sampling and testing location drawings
- A description of significant construction problems and the resolution of these problems
- As-built record drawings
- A statement of compliance with the construction contract documents and design intent, signed, and stamped by a professional engineer(s) registered in the state of Oklahoma.

The as-built record drawings will accurately site the constructed location of all work items. All surveying and base maps required for the development of the record drawings will be prepared by the surveyor. The POR must review and verify that as-builts are correct. As-builts will be included in the FCCR.

It is understood that the ET monolithic soil alternative final cover system will not be approved until the vegetation layer meets the specifications listed in Section 2.2.4.

## APPENDIX E

## **VEGETATION PLAN**

Includes pages E-1 through E-6



#### CONTENTS



## LIST OF TABLES

<u>Tabl</u>	<u>e</u>	Page
2-1	Typical Seeding Mixtures	E-4
2-2	Oklahoma Department of Agriculture, Food, and Forestry OAC 35:30-25-4. Limitations on Noxious Weed Seeds	E-5

#### **1** INTRODUCTION

The purpose of this vegetation plan is to set forth the procedures to be used in establishing interim and permanent vegetation at the Muskogee Recycling and Disposal Facility located in Muskogee County, Oklahoma. The site operates under Oklahoma Department of Environmental Quality (ODEQ) permit number 3551020.

This Vegetation Plan was developed using the ODEQ regulations pursuant to OAC 252:515-19-54.

## 2.1 Seeding Area

This section has been developed to address the seeding of portions of the site that (1) have not received final cover and (2) will remain undisturbed for more than 90 days. As final cover is constructed these areas will be sealed upon completion of the alternate final cover system according to this plan.

## 2.2 Interim Vegetation Establishment

Table 2-1 lists typical seeding mixtures that will be used for the site during each season. Also included is Table 2-2, which lists Limitations on Noxious Weed Seeds. Similar mixtures and application rates may also be used. It is understood that a variety of application rates and types of seed mixtures will produce adequate vegetative cover. The seeding mixture listed in Table 2-1 is only provided for a reference. The seed may be applied to the landfill slopes by various typical application methods such as hydromulch or seed drilling. Fertilizer will be applied to the seeded area, as needed. However, the following typical application method will be used.

- Additional soil will be added to the sideslopes, as needed, and the soil will be processed using a disc to prepare the soil for seeding.
- Fertilizer will be applied using a commercial spreader at a rate of approximately 150 pounds per acre (lb/acre) and the soil will be simultaneously disked using a disc-harrow. The fertilizer rate may vary, however, an initial rate of 10 (nitrogen) -20 (phosphate) -10 (potassium) may be used.
- The seed mixture will then be applied using a commercial spreader and the area simultaneously disked using a disc harrow.

• After discing the seeded area, hay will be mulched at a rate of approximately 3 bales (1,000 lbs each) per acre. To further minimize erosion potential and facilitate moisture retention, the hay will then be "crimped" using a roller to integrate the hay into the soil.

Maintenance of the interim vegetation will consist of protection, replanting, maintaining existing grades, and repair of erosion damage. Protection will include limiting site traffic or other uses immediately after seeding is completed.

In addition, water and fertilizer will be applied to the seeded areas to facilitate the establishment of vegetation during the months of May through September. Areas that have recently been seeded will be watered until vegetation is established. Typically, water will be applied to these areas on a weekly basis. Water will be obtained from an on-site source (i.e., pond). Both a primary and secondary water source will be identified so that water will be available, as needed. Typical water sources may include local contractors and area municipalities. Fertilizer will be applied at least every two months, or as needed to establish vegetation. The typical nutrient mixture will be 10-20-10, unless site-specific soil tests are performed which indicate a need for a different nutrient mixture. Manufactures recommendations regarding the application of fertilizer will be followed.

After vegetation is established in an area it will be inspected on a monthly basis during the active life of the site. The vegetation cover will be capable of selfregeneration and will require no maintenance. If bare spots develop, then the area will be re-seeded and maintained (e.g., watered and fertilized) as noted above until the vegetation is re-established.

## 2.3 Dust Control Plan

The main source of dust created at the site is from the site entrance road. Water will be applied to the entrance road on a daily basis or as needed to control dust. Other areas of the site will also be watered, as needed, to control dust. The site manager will routinely inspect the site to identify and control sources of dust.

## Table 2-1Typical Seeding Mixtures1

#### Spring/Summer Planting Season (Optimal Time for Planting-April 1 through May 30)

Spring Seeding Mixture	Minimum Percent Pure Live Seed Required	Pounds Per Live Seeds Required Per Acre
Common Bermuda Grass	85	12
Blue Stem	65	4
Side Oats Grama	65	6
Rye	85	4
	Total	26

#### Fall/Winter Planting Season (Optimal Time for Planting-September 1 through February 15)

Fall Seeding Mixture	Minimum Percent Pure Live Seed Required	Pounds Per Live Seeds Required Per Acre
Winter Wheat	75	40
Fescue or Rye	85	15
	Total	55

¹The mixtures shown in Table 2-1 are for reference purposes only. Other mixtures may also be used.

#### Table 2-2

#### Oklahoma Department of Agriculture, Food, and Forestry OAC 35:30-25-4. Limitations on Noxious Weed Seeds¹

Noxious Weed Seed	Limitations of Noxious Weed Seed (Number of Seeds per Pound)
Bindweed, Field (Convolvulus arvensis)	Prohibited
Yerba De Tajo (Eclipta alba)	Prohibited
Red Horned Poppy (Claucium corniculatum)	Prohibited
Knapweed, Russian (Centaurea picris)	Prohibited
Musk Thistle (Carduus nutans L.)	Prohibited
Nutgrass (Cyperus rotundus)	Prohibited
Scotch Thistle (Onoprodum acanthium)	Prohibited
Serrated Tussock (Nassella trichotoma)	Prohibited
Sicklepod (cassia obtusifolia)	Prohibited
Thistle, Canada (Cirsium arvense)	Prohibited
Whitetop or Hoary Cress (Cardaria draba)	Prohibited
Wild Oat [Avena fatus, Avena sterilis, and other wild noncultivated Avena spp., (In Wheat Only)]	Prohibited
Cockebur (Xanthium spp.)	3 per lb.
Jointed Goatgrass (Aegilops cylindrica)	5 per lb.
Moonflower or Giant Morningglory (Calonyction muricatum)	5 per lb.
Ballonvine (Cardiospermum halicacbum)	9 per lb.
Sericea Lespedeza (Lespedeza Cuneata)	9 per lb.
Wild Oat (Avena fatua, Avena sterilis, and other wild noncultivated Avena spp., Except in Wheat)	9 per lb.
Wild Buckwheat (Polygonum convolvulus)	18 per lb.
Onion, Wild or Garlic (Allium spp.)	18 per lb.
Wild Morningglory (Ipomoea spp.)	27 per lb.
Bindweed, Hedge (Convolvulus sepium)	27 per lb.
Johnsongrass (Sorghum halepense) (Except: Johnsongrass will be permitted in Yellow bluestem, Caucasian bluestem, and chaffy grasses not to exceed 300 per lb.)	45 per lb.
Quackgrass (Agrophyron repens)	45 per lb.
Blueweed, Texas (Helianthus ciliaris)	45 per lb.
Wild Mustard (Brassica spp.)	45 per lb.
Corncockle (Agrostemma githago)	45 per lb.
Plantain, Bracted (Plantago aristata)	45 per lb.
Giant Foxtail (Setaria faberi)	54 per lb.
Dodder (Cuscuta spp.)	90 per lb.
Darnel (Lolium temulentum)	90 per lb.
Dock (Rumex spn.)	90 per lb.
Horsenettle (Solanum carolinense)	90 per lb.
Nightshade Purnle (Solanum elaeagnifolium)	90 per lb.
Plantain Buckhorn (Plantago lanceolata)	90 per lb.
Sorrel Sheen or Red (Rumey acetosella)	90 per lb.
Chart or Charse (Bromus sacalinus)	200 per 10.
Current of Chess (Diolinus Secannus)	200 per 10.
bluestem, Caucasian bluestem, and chaffy grasses, the sum total noxious weeds shall not exceed 500 per lb.)	200 per lb.

¹ It is unlawful to sell, offer for sale, or expose for sale any agricultural or vegetable seed in Oklahoma if the noxious weed seed per pound is in excess of the following limitations.

## **3 VEGETATION PLAN – FINAL COVER AREAS**

This section has been developed to address the establishment of vegetation on the portions of the landfill, which have received final cover. Permanent vegetation will be established in disposal areas that have reached final grades and completed the installation of a final cover system.

Permanent vegetation will be established using the typical seeding mixtures and application methods discussed in Section 2. Currently the eastern slope of the eastern landfill unit has approximately 3 acres of established final cover. For future areas that will receive final cover, the initial seeding event will occur as follows:

- For final cover that is constructed in the winter, the initial seeding event will consist of a Fall/Winter seed mix, followed by permanent vegetation using a Spring/Summer seeding mixture.
- For final cover that is constructed in the spring, the initial seeding event will consist of a Spring/Summer seed mix, followed by permanent vegetation using a Fall/Winter seeding mixture.

This plan has been developed to be consistent with OAC 252:515-19-54 (i.e., permanent vegetation will be established during the first growing season after closure).

Maintenance of the permanent vegetation will typically consist of protection, replanting, maintaining existing grades, repair of erosion damage, and mowing. After the seeds have sprouted, Waste Management of Oklahoma, Inc., will inspect the slopes for areas with no grass or with thin grass. These areas will be reseeded, watered, and fertilized to establish an acceptable permanent vegetation layer. The maintenance procedures listed in Section 2 will be followed. If there are areas where establishing vegetation is unsuccessful an alternative plan will be developed.

## MUSKOGEE COMMUNITY RECYCLING AND DISPOSAL FACILITY MUSKOGEE COUNTY, OKLAHOMA ODEQ PERMIT NO. 3551020

#### **APPENDIX K**

## QUALITY ASSURANCE/QUALITY CONTROL PLAN FOR LINER AND LEACHATE COLLECTION SYSTEM INSTALLATION AND TESTING

Prepared for

Waste Management of Oklahoma, Inc.

October 2023



Prepared by

**Weaver Consultants Group, LLC** CA 3804 PE – 06/30/2025 6420 Southwest Boulevard, Suite 206 Fort Worth, TX 76109 817-735-9770

WCG Project No. 0086-364-11-19

## CONTENTS

LIST	Г ОГ ТА	BLES	K-iv
1	INTF	RODUCTION	K-1
	1.1	Purpose	K-1
	1.2	Definitions	K-1
	1.3	Meetings	K-5
2	CON	STRUCTION QUALITY ASSURANCE FOR EARTHWORK	K-6
	2.1	Introduction	К-6
	2.2	Earthwork Construction	К-6
		2.2.1 Subgrade	К-6
		2.2.2 General Fill	K-7
		2.2.3 Soil Liner	K-8
		2.2.3.1 Soil Liner Material	K-8
		2.2.3.2 Liner Construction	K-10
		2.2.4 Leachate Collection/Protective Cover	K-12
		2.2.5 Anchor Trench Backfill	K-13
	2.3	Construction Testing	K-13
		2.3.1 Testing Procedures	K-13
		2.3.2 Test Frequencies	K-15
3	CON	STRUCTION QUALITY ASSURANCE FOR GEOSYNTHETICS	K-17
	3.1	Introduction	K-17
	3.2	Geosynthetics Quality Assurance	K-17
		3.2.1 General	K-17
	3.3	Geomembrane	K-18
		3.3.1 Delivery	K-18
		3.3.2 Conformance Testing	K-19
		3.3.3 Geomembrane Installation	K-20
		3.3.4 Construction Testing	K-26
		3.3.5 Repairs	<b>—</b> K-30
		3.3.6 Wrinkles 10/13/2023	K-31
		3.3.7 Folded Material	K-31
		3.3.8 Geomembrane Anchor Trench	K-32
		3.3.9 Geomembrane Acceptance	K-32
		3.3.10 Bridging	K-32
	3.4	Geotextiles	K-32
		3.4.1 Delivery	K-33

Weaver Consultants Group, LLC Rev. 0, 10/2023 Appendix K  $Q: \verb|WASTE MANAGEMENT\verb|MUSKOGEE LANDFILL\verb|EXPANSION 2022\verb|APPLICATION\verb|VOLUME 3\verb|APPENDIX K.DOC||APPLICATION||APPENDIX K.DOC||APPENDIX K.DOC||APPLICATION||APPENDIX K.DOC||APPENDIX K.DOC||APPLICATION||APPENDIX K.DOC||APPENDIX K.$ 

	3.5 3.6 3.7	<ul> <li>3.4.2 Testing</li> <li>3.4.3 Geotextile Installation</li> <li>3.4.4 Repairs</li> <li>Geosynthetic Clay Liner (GCL)</li> <li>3.5.1 Delivery</li> <li>3.5.2 Conformance Testing</li> <li>3.5.3 GCL Installation</li> <li>Equipment on Geosynthetic Materials</li> <li>Reporting</li> </ul>	K-33 K-34 K-35 K-35 K-35 K-36 K-38 K-40 K-41
4	<b>CONS</b> 4.1 4.2	TRUCTION QUALITY ASSURANCE FOR PIPINGIntroductionPipe and Fittings4.2.1General4.2.2Delivery4.2.3Conformance Testing4.2.4Pipe and Fitting Installation	K-42 K-42 K-42 K-42 K-43 K-43 K-44
5	<b>CONS</b> 5.1 5.2	<b>TRUCTION QUALITY ASSURANCE FOR DRAINAGE MATERIAL</b> Drainage Aggregate Installation	<b>K-46</b> K-46 K-46
6	<b>DOCU</b> 6.1 6.2 6.3 6.4 6.5	MENTATION Daily Record of Construction Progress Observation and Test Data Sheets Photographs Design and Specification Changes LIT Report JONATHAN V. G UUEEN 24456 TAHOMA 10/13/2023	K-47 K-47 K-48 K-48 K-48

TABLES

S	Page No.
Pre-Construction Testing Schedule for Soil Liner Material	К-9
Required Soil Liner Material Properties	K-10
Minimum Earthwork Construction Testing Frequencies	K-16
Required Testing for 60-mil-thick Smooth and Textured (Both Sides)	)
HDPE Geomembranes	K-20
Minimum Required Properties of 60-mil-thick Smooth and Textured	
(Both Sides) HDPE Geomembranes	K-23
Required Testing and Properties of Geotextile	K-35
Required Testing for GCL Materials	K-38
Required Properties for Reinforced GCL Material	K-39
	Pre-Construction Testing Schedule for Soil Liner Material Required Soil Liner Material Properties Minimum Earthwork Construction Testing Frequencies Required Testing for 60-mil-thick Smooth and Textured (Both Sides) HDPE Geomembranes Minimum Required Properties of 60-mil-thick Smooth and Textured (Both Sides) HDPE Geomembranes Required Testing and Properties of Geotextile Required Testing for GCL Materials Required Properties for Reinforced GCL Material

## 1.1 Purpose

This Quality Assurance/Quality Control (QA/QC) Plan provides the procedures to govern the construction quality of landfill liners in accordance with OAC 252:515, as promulgated by the Oklahoma Department of Environmental Quality (ODEQ) and the EPA Technical Guidance Document: Quality Assurance and Quality Control for Waste Containment Facilities (EPA/600/R-93182, September 1993). The QA/QC Plan, together with the construction drawings and specifications, addresses the soils and geosynthetics components of the liner and leachate collection system in accordance with OAC 252:515-11 and 252:515-13.

This QA/QC Plan is divided into the following parts:

- Section 1 Introduction
- Section 2 Construction Quality Assurance for Earthwork
- Section 3 Construction Quality Assurance for Geosynthetics
- Section 4 Construction Quality Assurance for Piping
- Section 5 Construction Quality Assurance for Drainage Material
- Section 6 Documentation

## **1.2** Definitions

Whenever the terms listed below are used, the intent and meaning will be interpreted as indicated.

#### ASTM

The American Society for Testing and Materials

#### Construction Quality Assurance (CQA)

A planned system of activities that provides the Operator and permitting agency assurance that the facility was constructed as specified in the design. Construction quality assurance includes observations and evaluations of materials, and workmanship necessary to determine and document the quality of the constructed facility. Construction quality assurance (CQA) refers to measures taken by the CQA organization to assess if the installer or contractor is in compliance with the plans and specifications for a project.

#### Construction Quality Assurance Professional of Record (POR)

The POR is an authorized representative of the Operator and has overall responsibility for construction quality assurance that confirms that the facility was constructed in accordance with plans and specifications approved by the permitting agency. The POR must be registered as a Professional Engineer in Oklahoma and experienced in geotechnical testing and its interpretations. Experience and education must include geotechnical engineering, engineering geology, soil mechanics, geotechnical laboratory testing, construction quality assurance and quality control testing, and hydrogeology. The POR must show competency and experience in certifying like installations, and be approved by the permitting agency, and be presently employed by or practicing as a geotechnical engineer in a recognized geotechnical/environmental engineering organization. POR or his designated representative will be on-site during all liner system construction.

The POR may also be known in applicable regulations and guidelines as the CQA Engineer, Resident Project Representative, or the Geotechnical Professional (GP).

#### Construction Quality Assurance (CQA) Monitors

These are representatives of the POR who work under direct supervision of the POR. The CQA monitor is responsible for quality assurance monitoring and performing onsite tests and observations. Field observations, testing, or other activities associated with CQA may be performed by the CQA monitor(s) on behalf of the POR.

#### **Contract Documents**

These are the official set of documents issued by the Operator. The documents include bidding requirements, contract forms, contract conditions, specifications, contract drawings, addenda, and contract modifications.

#### **Contract Specifications**

These are the qualitative requirements for products, materials, and workmanship upon which the contract is based.

#### Contractor

This is the person or persons, firm, partnership, corporation, or any combination, private or public, who, as an independent contractor, has entered into a contract with the Operator, and who is referred to throughout the contract documents by singular number and masculine gender.

#### Design Engineer

These individuals or firms are responsible for the design and preparation of the project construction drawings and specifications. Also referred to as "designer" or "engineer."

#### Earthwork

This is a construction activity involving the use of soil materials as defined in the construction specifications and Section 2 of this plan.

#### Film Tear Bond (FTB)

A failure in the geomembrane sheet material on either side of the seam and not within the seam itself.

#### Geomembrane Liner (GM)

This is a synthetic lining material, also referred to as geomembrane, membrane liner, or sheet. The term Flexible Membrane Liner (FML) is also used for GM.

#### Geosynthetic Clay Liner (GCL)

This is a composite lining material consisting of bentonite adhered to a covering geosynthetic material.

#### **Geosynthetics Contractor**

This individual is also referred to as the "contractor" or "installer" and is the person or firm responsible for geosynthetic construction. This definition applies to any person installing FML, GCL, geotextile, or geocomposite, even if not his primary function.

#### Independent Testing Laboratory

A laboratory that is independent of ownership or control by the permittee or any party to the construction of the liner system or the manufacturer of the liner system products used.

#### Liner Installation and Testing Plan (LIT)

Certification report for the composite liner, prepared and sealed by the POR, that is submitted to ODEQ for approval.

#### Manufacturing Quality Assurance (MQA)

A planned system of activities that provides assurance that the raw materials were constructed (manufactured) as specified.

#### Manufacturing Quality Control (MQC)

A planned system of inspection that is used to directly monitor and control the manufacture of a material.

#### Municipal Solid Waste Landfill Regulations (MSWLR)

The state regulations for which management of solid waste is regulated. For Oklahoma, it is the Title 252 Department of Environmental Quality Chapter 515 Management of Solid Waste Regulations.

#### Nonconformance

This is a deficiency in characteristic, documentation, or procedure that renders the quality of an item or activity unacceptable or indeterminate. Examples of non-conformances include, but are not limited to, physical defects, test failures, and inadequate documentation.

#### Operator

The organization that will operate the disposal unit.

#### **Operator's Representative**

The official representative of the operator responsible for planning, organizing, and managing the construction activities.

#### Organics

Organic matter is material that may be capable of decay (e.g., plant material), the product of decay, or both.

#### Permittee's Representative

This is the person that is an official representative of the permittee responsible for planning, organizing, and controlling the design and construction activities.

#### Panel

This is a unit area of the FML which will be seamed in the field.

#### Quality Assurance (QA)

This is a planned and systematic pattern of procedures and documentation to ensure that items of work or services meet the requirements of the contract documents. Quality assurance includes quality control. Quality assurance will be performed by the POR and CQA monitor.

#### Quality Control (QC)

These actions provide a means to measure and regulate the characteristics of an item or service to comply with the requirements of the contract documents. Quality control will be performed by the contractor.

## 1.3 Meetings

To facilitate construction and to define construction goals and activities, coordination between the Design Engineer, Owner, Operator, POR, CQA Monitor, and Contractor is essential. To meet this objective, meetings will be held prior to and throughout the construction process. Per OAC 252:515-11-5, ODEQ shall be notified at least 48 hours in advance of the Pre-construction Meeting and at least two weeks before liner system construction begins. The pre-construction notification will (1) define the area to be constructed and (2) include the names of the contractors, and the QA and QC officials. The notification will also include pre-construction test information.

## 2 CONSTRUCTION QUALITY ASSURANCE FOR EARTHWORK

## 2.1 Introduction

This section of the Quality Assurance/Quality Control (QA/QC) Plan addresses the construction of the earthwork components of the liner system and outlines the QA/QC Plan program to be implemented with regard to materials selection and evaluation, laboratory test requirements, field test requirements, and treatment of problems.

The landfill is designed to include a Subtitle D composite liner for the undeveloped liner area. The liner system for the undeveloped area will consist of a 2-foot-thick compacted clay liner and a 60-mil-thick high density polyethylene (HDPE) Flexible Membrane Liner (FML). An alternative liner option is also available for the which consists of replacing the 2 foot compacted clay layer with a geosynthetic clay liner (GCL) and a 1-foot-thick compacted clay layer ( $k \le 1x10^{-7}$  cm/s). Refer to Section 3 of this QA/QC Plan for more information regarding the Construction Quality Assurance for the FML and GCL.

## **2.2** Earthwork Construction

The following paragraphs describe general construction procedures to be used for various earthwork components within the landfill. The earthwork construction specifications will be developed based on the material and construction procedures outlined in this section of the QA/QC Plan for each specific liner construction.

## 2.2.1 Subgrade

Subgrade refers to a surface which is exposed after stripping topsoil or excavating to establish the grade directly beneath the composite liner. The subgrade must be constructed to allow for the composite liner to conform to the permitted Top of Liner Plan.

Prior to beginning liner construction, the subgrade area will be stripped to a depth sufficient to remove all loose surface soils or soft zones within the exposed excavation. The upper 6 inches of the subgrade will be compacted to a minimum of 90 percent of the maximum dry density as determined by the Standard Proctor (ASTM D698), unless the subgrade is part of the perimeter berm. Perimeter berm soils shall be compacted to 95 percent of the maximum dry density. The liner

subgrade area will be proof rolled with heavy, rubber tired construction equipment to detect unstable areas. Unstable areas will be undercut to firm material and backfilled with suitable compacted general fill. The subgrade will also be scarified prior to placement of the first lift of clay liner.

Subgrade voids and cracks are expected to be minor. However, the subgrade will be re-worked as necessary to provide a foundation suitable for soil liner placement. Visual examination of the subgrade preparation by the CQA monitor will generally be sufficient to evaluate its suitability as a foundation for the subgrade. The CQA monitor may find that physical testing is necessary to evaluate the prepared subgrade or general fill placed in large voids.

The POR will approve the prepared subgrade prior to the placement of soil liner or general fill. Approval will be based on a review of test information, if applicable, and CQA monitoring of the subgrade preparation.

#### 2.2.2 General Fill

General fill material placed below the floor of the composite liner will be placed in uniform lifts to an elevation of subgrade minus 1 foot and proof-rolled with a heavy, rubber tired construction equipment to detect unstable areas. Unstable areas will be undercut to firm material and backfilled with suitable compacted general fill. The remaining 1 foot will be placed in uniform lifts that do not exceed 9 inches in loose thickness and are compacted to at least 90 percent of the maximum dry density as determined by the Standard Proctor (ASTM D698) at a moisture content equal to or greater than the optimum moisture content.

General fill material placed as part of the perimeter berm will be placed in uniform lifts that do not exceed 9 inches in loose thickness and are compacted to at least 95 percent of the maximum dry density as determined by the Standard Proctor (ASTM D698) at a moisture content equal to or greater than the optimum moisture content.

General fill material will be relatively homogeneous clay, silty clay, sandy clay, or clayey sand. The material shall classified as CL, CH, ML, SM, or SC according to the Unified Soil Classification System (USCS). General fill shall be tested to determine the USCS classification at a frequency of 1 per 50,000 cy. The general fill material interface strength parameters will be verified by the Design Engineer prior to construction by review of existing data or completion of additional testing to verify the assumed strength parameter values utilized in the site slope stability analysis. The analysis was developed using peak strength values and a factor of safety of 1.5 (long-term condition), 1.3 (short-term condition), and 1.15 (seismic condition). If test results differ from assumed values, the analysis will be updated to meet these minimum factor of safety values and the additional analysis will be placed in the Site Operating Record.

#### 2.2.3 Soil Liner

The soil liner will consist of a minimum 2-foot-thick (or 1-foot minimum thickness if the GCL alternative liner system is used) compacted clay liner (measured perpendicular to the subgrade surface) that will extend along the floor and side slopes of the landfill. The soil liner will be constructed in continuous, single, compacted lifts (6 inches thick) parallel to the floor and sideslope subgrades with a permeability of  $1x10^{-7}$  cm/s or less.

Surveying will be performed to verify that the excavation/bottom of clay liner grades is to the lines and grades specified in the design with a vertical tolerance of -0.2 feet to +0.0 feet to ensure that the clay liner will achieve a 2-foot minimum thickness (or 1-foot minimum thickness if the GCL alternative liner system is used).

#### 2.2.3.1 Soil Liner Material

Adequate clayey soil liner material will be available from landfill excavations and/or onsite borrow sources. The liner soil will be free of debris, rock greater than 1 inch in diameter, vegetative matter, frozen materials, foreign objects, and organics. Laboratory tests will verify that materials are adequate to meet the compacted clay liner requirements prior to liner construction. As necessary, an off-site borrow source can be used for soil liner and protective cover construction. Representative samples from onsite and/or offsite borrow sources will be subject to the minimum pre-construction testing program shown in Table 2-1.

## Table 2-1Pre-Construction Testing Schedule for Soil Liner Material

Test	Method Used	Frequency ⁴
Soil Classification	ASTM D2487	1 per 10,000 cy
Particle Size Analysis (including % passing No. 200 Sieve)	ASTM D422 or ASTM D1140	1 per 10,000 cy
Atterberg Limits	ASTM D4318	1 per 10,000 cy
Moisture Content	ASTM D2216 or ASTM D4643	1 per 10,000 cy
Standard Proctor Test or Modified Proctor Test ³	ASTM D698, if light weight compactor is to be used ASTM D1557, if heavy weight	1 per 10,000 cy
Hydraulic Conductivity ¹	ASTM D5084 ²	1 per 10,000 cy

¹ Conduct this test on remolded sample that is compacted to 95% of the maximum dry density and at the optimum moisture content, as determined from the Standard Proctor test or compacted to 90% and at optimum moisture content for Modified Proctor test. Allow 1% tolerance for both dry density and moisture content. The sample fabricated with lower-bound density and moisture should represent worst case conditions for hydraulic conductivity results.

² Testing procedures in Appendix VII of the Corps of Engineers Manual EM 1110-2-1906, November 30, 1970, Laboratory Soils Testing, is an alternative method.

³ Soil types or blends proposed for control using alternative moisture-density acceptance criteria shall be tested by both Standard and Modified Proctor methods, along with their corresponding hydraulic conductivity tests.

⁴ 1 per 10,000 cubic yard (cy) or a minimum of 1 test per source or soil type, whichever is greater. Cubic yard to be calculated from in-place volumes of certified liner construction. The calculated number of tests shall be rounded up to the nearest whole number.

The Proctor moisture-density curves shall be developed for each type of soil determined suitable as soil liner material and shall be used during the construction phase as a performance reference for compaction and moisture control. However, if soil types vary substantially and cannot or will not be segregated, representative blends of the soil types anticipated to be utilized for soil liner construction should also be sampled and tested. Separate but equivalent portions of the sample should be used if both Standard and Modified Proctor tests are to be performed for a given soil type or soil blend. Samples should not be oven-dried nor dried back more than two to three percent drier than necessary to obtain the desired test point. The zero air voids line shall be computed and included along with the Proctor curves, indicating the specific gravity value used.

An alternative moisture-density acceptance criteria may be established for a particular soil type or blend using both the Standard and Modified Proctor relationships. In this case, both tests must be run for that soil, and an acceptable range of moisture and density may be defined as shown in Figure 2-1. This approach will allow a rational method for accepting or disapproving compaction results when more than one type of compactor or compaction effort is used or when variations in Proctor relationships make choosing the most appropriate curve difficult. The acceptable moisture-density range shall be determined by the POR.

As a general rule, pre-construction tests will be performed at a frequency not less than one test series for every 10,000 cubic yards of soil to be used in soil liner

construction, unless soil types are limited and easily distinguished. As soil is usually made available subsequent to excavation during soil liner construction, additional pre-construction samples should be taken and tests performed when soils vary, or when the initial pre-construction test results appear inappropriate or questionable. If and when the same borrow source is utilized for the soil supply of more than one soil liner area, results from previous tests may be used to supplement the pre-construction data.

Soils used in soil liners will have the following minimum values verified by testing in a soil laboratory prior to liner construction.

Test ¹	Specification
Coefficient of Permeability (Remolded Sample)	1.0x10 ⁻⁷ cm/s or less (see Note 2)
Plasticity Index	≥ 10
Liquid Limit, percent	≥ 24
Percent Passing No. 200 Mesh Sieve	≥ 30
Percent Retained on #4 Sieve	≤ 20

Table 2-2Required Soil Liner Material Properties

¹ Testing will be performed in accordance with the test methods included in Section 2.3.

 2  Coefficient of permeability for 1-foot-thick soil liner for the GCL alternative liner system will be less than or equal to  $1 \times 10^{-7}$  cm/s.

#### 2.2.3.2 Liner Construction

The soil liner material will be placed in maximum 9-inch-thick loose lifts to produce compacted lift thickness of approximately 6 inches. The material will be compacted to a minimum of 95 percent of the maximum dry density determined by Standard Proctor (ASTM D698), or 90 percent of the maximum dry density as determined by the Modified Proctor (ASTM D1557) at a moisture content equal to or greater than the optimum moisture content.

The soil liner must be compacted with a pad/tamping-foot. The lift thickness will be controlled so that there is total penetration through the loose lift under compaction into the top of the previously compacted lift; therefore, the lift thickness must not be greater than the pad or prong length. Use of pad/tamping-foot or prong-foot rollers will provide sufficient roughening of liner lifts surface for bonding between lifts. These procedures are necessary to achieve adequate bonding between lifts and reduce seepage pathways. Adequate cleaning devices must be in place and maintained on the compaction roller so that the prongs or pad feet do not become clogged with clay soils to the point that they cannot achieve full penetration during initial compaction. The footed roller is necessary to achieve this bonding and to reduce the individual clods and achieve a blending of the soil matrix through its kneading action. In addition to the kneading action, weight of the compaction equipment is important. Multiple passes are recommended for a vehicle with front

and rear drums. The soil liner will not be compacted with a bulldozer or any trackmobilized equipment unless it is used to pull a pad-footed roller.

Water shall be applied as necessary to the material and worked evenly into the material with the compaction equipment. Water used for the soil liner must be clean and not contaminated by waste or any objectionable material. Collected onsite stormwater may be utilized if it has not come into contact with the solid waste.

Soil liner construction should not be conducted in adverse weather conditions (heavy rain, freezing temperatures, etc.).

The soil liner will be visually inspected to evaluate its integrity during and after construction. CQA testing of the soil liner will also be performed as the soil liner is being constructed. Testing of the soil liner is addressed in Section 2.3. Sections of compacted soil liner which do not pass both the density and moisture requirements will be reworked with additional passes of the compactor until the section in question passes. All field density and moisture test results will be incorporated into the LIT Report.

Hydraulic conductivity samples will be obtained by pushing a sampler through the constructed clay liner. The sample from each test location will be sealed and transported to the laboratory. Two samples may be collected at each sample location and labeled the "A" and "B" sample. The sampling holes (e.g., samples for hydraulic conductivity) will be backfilled with bentonite or a bentonite/clay liner soil material mixture consisting of at least 20 percent bentonite.

If the integrity of the "A" sample appears to have been compromised during the transportation of the sample prior to testing, the "B" sample may be tested. In addition, if an "A" sample hydraulic conductivity test does not comply with the minimum allowable value, the "B" sample collected at the same location may be tested to determine compliance with the hydraulic conductivity requirements if during testing of the "A" sample, the ASTM D5084 or EM 1110-2-1906 procedure was not followed or the permeameter malfunctioned.

The POR will provide a detailed justification of the use of the "B" sample, if applicable, in the LIT Report.

If the "B" sample passes, the area will be considered in compliance. If the "B" sample fails (or sample "A" fails in such a way that there is not an option to use the "B" sample), the test interval will be considered unsatisfactory for the area bounded by passing test locations (but not extending past a satisfactory test location). Additional tests may be taken to further define the unsatisfactory area. The area defined unsatisfactory will be reworked and retested in accordance with this section.

Furthermore, if it is determined that the "B" sample may not be used to replace the "A" sample result, then the test interval will be considered unsatisfactory for the area bounded by passing test locations (but not extending past a satisfactory test location).

Once the exact area is determined, the constructed liner lifts will be scarified to the bottom of the lift that did not pass the hydraulic conductivity test, and reconstructed. Reconstructed liner area is subject to the testing frequencies listed in Table 2-3. Additionally, each lift of the reconstructed liner area will be tested for hydraulic conductivity. Reconstruction activities, including additional testing and surveying, will be incorporated into the LIT Report.

The finished surface of the final lift of soil liner must be rolled with a smooth, steelwheeled roller to obtain a hard, uniform, and smooth surface. The surface of the final lift of soil liner will then be inspected by the CQA monitor. All undesired materials will be removed from the liner surface, including removing rocks, cobbles, roots and other foreign objects over one inch in diameter, as well as all surface rocks regardless of size if FML and/or GCL is to be laid on top. Any voids created by removing undesired materials will be backfilled with liner material to the density specifications outlined for liner construction and tested at the discretion of the CQA monitor.

Surveying will be performed to verify that the finished top of liner grade is to the lines and grades specified in the design with a vertical tolerance of -0.0 feet to +0.2 feet to ensure that the clay liner achieves a 2 foot minimum thickness (or 1-foot minimum thickness if the GCL alternative liner system is used).

The POR will submit to the ODEQ a LIT Report for approval of each soil liner area.

The soil liner will be prevented from losing moisture, protected from desiccation cracking, frost damage, or damage from equipment prior to placement of the GCL and FML. Preserving and protecting the installed soil liner will be dependent on the earthwork contractor's means and methods and is subject to POR approval.

#### 2.2.4 Leachate Collection/Protective Cover

The leachate collection layer consists of a 12-inch-thick soil layer with a permeability of  $1 \times 10^{-3}$  cm/s or greater and less than 5% fines (passing no 200 sieve). The protective cover layer consists of an additional 12-inch-thick leachate collection layer.

The leachate collection/protective cover material will consist of soil materials that have not previously come in contact with solid waste and do not contain materials detrimental to the underlying geosynthetics. The leachate collection/protective cover material will be free of organics, foreign objects, or other deleterious materials. The physical characteristics of the leachate collection/protective cover material will be evaluated through visual observation and laboratory testing during construction. Leachate collection/protective cover (soil) material will be tested for permeability and fines at one test per 100,000 square feet. Additional testing during construction will be at the discretion of the CQA monitor.

The leachate collection/protective cover material will be placed using any low ground pressure equipment as outlined in Section 3.6. The leachate collection/protective cover material will be placed by spreading in front of the spreading equipment with a minimum of 12 inches of soil between the spreading equipment and the installed geosynthetics. Under no circumstances will the construction equipment come in direct contact with the installed geosynthetics.

Surveying will be performed to ensure that the leachate collection/protective cover material achieved a 2-foot minimum thickness.

During construction the CQA monitor will:

- Verify that grade control is performed prior to work.
- Verify that underlying geosynthetic installations are not damaged during placement operations or by survey grade controls. Mark damaged geosynthetics and verify that damage is repaired.
- Verify that the cover soil for sideslopes is pushed from the toe up the slope.
- Monitor haul road thickness over geosynthetic installations and verify that equipment hauling and materials placement meet equipment specifications (see Section 3.7).
- The POR will coordinate with the project surveyor to perform a thickness verification survey of the leachate collection/protective cover materials upon completion of placement operations. Verify corrective action measures as determined by the verification survey.

#### 2.2.5 Anchor Trench Backfill

The anchor trench backfill material for geosynthetic anchoring will be placed in uniform lifts which do not exceed 12 inches in loose thickness and will be compacted by reasonable means. Tests may be taken at the discretion of the CQA monitor to evaluate the quality of the backfill. The test results will not be required as part of the LIT Report.

## 2.3 Construction Testing

#### **2.3.1 Testing Procedures**

Testing and evaluation of the soil liner during construction will be in accordance with the ODEQ Municipal Solid Waste Landfill Regulations (MSWLR). The

construction methods and test procedures documented in the LIT report (see Section 6) will be consistent with this QA/QC Plan and the MSWLR.

CQA monitors will perform field and laboratory tests in accordance with applicable standards outlined in the specifications. The following test standards apply:

<u>Standard</u>	Test Description
ASTM D698	Moisture-density relations of soils and soil- aggregate mixtures, using 5½-lb rammer and 12- inch drop
ASTM D1557	Moisture-density relations of soils and soil- aggregate mixtures using 10-lb rammer and 18-inch drop
ASTM D422	Particle size analysis of soils
ASTM D6938	Density and water content of soil and soil aggregate in place by nuclear methods
ASTM D2216	Laboratory determination of water (moisture) content of soil and rock by mass

<u>Standard</u>	Test Description			
ASTM D5084	Hydraulic conductivity of saturated porous materials			
ASTM D4318	Liquid limit, plastic limit, and plasticity index of soils			
ASTM D1140	Amount of material in soils finer than No. 200 sieve			
ASTM D2487	Classification of soils for engineering purposes			
EM 1110-2- 1906	US Army Corps of Engineers permeability test			
ASTM D2488	Description and identification of soils (visual- manual procedure)			
ASTM D2937	Density of soil in place by the drive-cylinder method			
ASTM D2167	Density and unit weight of soil in place by the rubber balloon method			
ASTM D1556	Density and unit weight of soil in place by the sand- cone method			
ASTM D4643	Determination of water (moisture) content of soil by the microwave oven method			
ASTM D1587	Thin-walled tube sampling of soils for geotechnical purposes			

#### 2.3.2 Test Frequencies

OAC 252:515-11-32, 33, 34, 36, & 37 establish the required tests, frequencies, and properties for the earthwork construction quality assurance. The pre-construction testing schedule for soil liner material is listed on Table 2-1. The testing frequencies for the earthwork are listed in Table 2-3 and establish a minimum number of required tests. Additional testing must be conducted whenever work or materials are suspect, marginal, or of poor quality. Additional testing may also be performed to provide additional data for engineering evaluation. Any retests performed as a result of a failing test do not contribute to the total number of tests performed to satisfy the minimum test frequency.

Table 2-3Minimum Earthwork Construction Testing Frequencies

Test (ASTM No.)	Subgrade	General Fill ⁽²⁾	Soil Liner Construction	Leachate Collection/ Protective Cover
Moisture/Density of Soil In-Place (D6938) ⁽⁵⁾	3 per acre for upper 6 inches only	3 per acre per 6-inch lift ⁽¹⁾	3 per acre per 6-inch lift ⁽⁴⁾	N/A
Visual Classification (D2488)	N/A	Continual during placement	Continual during placement	Continual during placement
Hydraulic Conductivity (D2434, D5084, or Corps of Engineers EM 1110-2- 1906, Appendix VII)	N/A	N/A	2 per acre for top 12 inches of bottom liner; 1 per acre for top 12 inches of sidewall liner with $k \le 1x10^{-7} \text{ cm/s}^{(6, 7)}$	Only applicable for 12 inch or 24 inch leachate collection/protective cover material with k≥1x10 ⁻³ cm/s. The frequency shall be 1/100,000 sf.
Survey/Thickness Verification ⁽³⁾	100-foot square grid points with a vertical tolerance of -0.2 feet to 0.0 feet on the top of subgrade (bottom of clay) surface	100-foot square grid points with a vertical tolerance of -0.2 feet to 0.0 feet on the top of general fill (bottom of clay) surface	100-foot square grid points with a vertical tolerance of 0.0 feet to +0.2 feet on the top of soil liner surface, minimum 2 reference points, minimum thickness required	100-foot square grid points with a minimum of 2 reference points, minimum thickness required

¹ General fill material placed below the floor of the composite liner will be placed in uniform lifts to an elevation of subgrade minus 1 foot and proof-rolled with a heavy, rubber tired construction equipment to detect unstable areas. Unstable areas will be undercut to firm material and backfilled with suitable compacted general fill. The remaining 1 foot will be placed in uniform lifts that do not exceed 9 inches in loose thickness and are compacted to at least 90 percent of standard Proctor (ASTM D698) at a moisture content equal to or greater than the optimum moisture content.

² General fill material will be relatively homogeneous clay, silty clay, sandy clay, or clayey sand. The material shall classify as CL, CH, ML, SM, or SC according to the Unified Soil Classification System (USCS). General fill shall be tested at a frequency of 1 per 50,000 cy.

³ Surveying to be completed by a registered Oklahoma surveyor. The thickness verification will be supplemented by additional points deemed necessary by the POR, such as breaks in grade and at the leachate piping and sump locations. The selected grid should be the same for both beginning and finished elevations of the soil liner so that the minimum liner thickness can be verified.

⁴ A minimum of two tests shall be performed on the bottom and one on sideslope areas.

⁵ To include the conventional oven drying method (ASTM D2216) performed on every 10 samples.

⁶ In-situ samples shall be retrieved in accordance with ASTM D1587.

⁷ Hydraulic conductivity for 1-foot-thick compacted clay component of GCL alternative liner system will be k<1.0x10⁻⁷ cm/s at frequency listed.



## **3** CONSTRUCTION QUALITY ASSURANCE FOR GEOSYNTHETICS

### 3.1 Introduction

This section of the QA/QC Plan addresses the construction of the geosynthetic components of the liner and leachate collection systems and outlines the QA/QC Plan program to be implemented with regard to materials selection and evaluation, laboratory test requirements, field test requirements, and treatment of problems.

The overall goal of the geosynthetics quality assurance program is to assure that proper construction techniques and procedures are used and the geosynthetic contractor implements his quality control plan in accordance with this QA/QC Plan. The quality assurance program is intended to identify and define problems that may occur during construction and to observe that these problems are avoided and/or corrected before construction is complete.

## **3.2 Geosynthetics Quality Assurance**

#### 3.2.1 General

The composite liner system provides the primary means for preventing leachate infiltration into groundwater. A GCL and/or geomembrane is a component of the composite liner. Proper geosynthetic installation is a crucial work element, which greatly affects the performance of the composite liner system. Construction quality control for the geosynthetic installation will be performed by the geosynthetic installation contractor. Construction quality assurance for the geosynthetic installation will be performed by the POR to assure the geosynthetics is constructed as specified in the design. Construction must be conducted in accordance with the procedures outlined in this QA/QC Plan. To monitor compliance, a quality assurance program will include the following:

- A review of the manufacturer's quality control testing
- Material conformance testing by an independent third party laboratory
- Field and construction testing
- Construction monitoring

Conformance testing refers to material testing performed by an independent third party laboratory that takes place prior to material installation. Field and construction testing includes testing that occurs during geosynthetics installation.

Quality assurance testing will be conducted in accordance with this QA/QC Plan. Field testing will be observed by the CQA monitor. Documentation must meet the requirements of this QA/QC Plan.

## 3.3 Geomembrane

The composite liner system will include a 60-mil HDPE geomembrane. The geomembrane will be smooth on both sides on the liner floor and textured on both sides on the liner sideslopes. Geomembrane liner will conform to the current specifications found in Geosynthetic Research Institute (GRI) Test Method GM-13. Required quality control tests for the geomembrane are included in Table 3-1 and required material properties for the geomembrane are included in Table 3-2.

### 3.3.1 Delivery

Upon delivery of FML, the CQA monitor will observe that:

- The geomembrane is delivered in rolls and is not folded. Folded geomembrane is not acceptable because the highly crystalline structure of the geomembrane will be damaged if it is folded. Any evidence of folding (other than from the manufacturing process) or other shipping damage is cause for rejection of the material.
- Equipment used to unload and store the rolls or pallets does not damage the geomembrane.
- The geomembrane is stored in an acceptable location in accordance with the manufacturer's specifications and stacked not more than 5 rolls high. The geomembrane is protected from puncture, dirt, grease, water, moisture, mud, mechanical abrasions, excessive heat, or other damage.
- All manufacturing documentation required by the specifications outlined in this QA/QC Plan has been received and reviewed for compliance prior to shipment of FML to the site. This documentation will be included in the LIT Report.
- The geosynthetics receipt log form has been completed for all materials received.

Damaged geomembrane will be rejected and removed from the site or stored at a location separate from accepted geomembrane. Geomembrane that does not have
proper manufacturer's documentation must be stored at a separate location until all documentation has been received, reviewed, and accepted.

#### **3.3.2** Conformance Testing

The geomembrane material and its components will be tested accordance with Table 3-1 by the supplier/manufacturer. A certificate of analysis will be submitted as part of the quality control documentation.

A third party independent lab will test the material components. A sample will be obtained for every resin lot of material supplied of geomembrane installed. The material will be sampled at the site by the CQA monitor or independent third-party representation. The samples will be forwarded to the independent third-party laboratory for the following conformance tests:

- Specific gravity/density (ASTM D1505 or alternate ASTM D792, if approved by the POR)
- Carbon black content (ASTM D4218)
- Carbon black dispersion (ASTM D5596)
- Thickness (ASTM D5199 for smooth FML and for textured FML use ASTM D5994 or alternate ASTM D1593 if approved by POR)
- Tensile properties (ASTM D6693/Type IV, ASTM D638 may be used upon approval by POR)

The density of the geomembrane must be greater than 0.94 g/cc; the carbon black content must be between 2 percent and 3 percent; and recycled or reclaimed material must not be used in the manufacturing process.

The design engineer may require additional test procedures and will inform the third party laboratory in writing. The POR must review all test results and report any nonconformance to the design engineer prior to product installation. Refer to Table 3-2 for a complete listing of the material requirements for both smooth and textured geomembranes that will be used for the Subtitle D composite liner.

**Sampling Procedure**. Samples will be taken across the entire roll width. Unless otherwise specified, samples will be approximately 15 inches long by the roll width. The CQA monitor must mark the machine direction and the manufacturer's roll identification number on the sample. The CQA monitor or independent third-party representation must also assign a conformance test number to the sample and mark the sample with that number.

# Table 3-1Required Testing for 60-mil-thick Smooth andTextured (Both Sides) HDPE Geomembranes1

Test	Type of Test	Standard Test Method	Frequency of Testing (Minimum)
Resin	Specific Gravity/Density	ASTM D792, Method A	Every resin lot
		or ASTM D1505	
	Melt Flow Index	ASTM D1238	Every resin lot
Manufacturer's	Thickness	ASTM D5199 (smooth)	Per Roll of Geomembrane
Quality Control		or ASTM D5994 ² (textured)	
	Specific Gravity/Density	ASTM D1505/D792	Per 200,000 pounds
	Carbon Black Content	ASTM D4218	Per 20,000 pounds
	Carbon Black Dispersion	ASTM D5596	Per 45,000 pounds
	Tensile Properties	ASTM D6693 / Type IV	Per 20,000 pounds
		(ASTM D638 may be used as	
		an alternative upon POR's	
		approval)	
	Tear Resistance	ASTM D1004	Per 45,000 pounds
	Puncture Resistance	ASTM D4833	Per 45,000 pounds
	Stress Crack Resistance	ASTM D5397	Per GRI-GM 10
	Oxidative Induction Time	ASTM D3895 or	Per 200,000 pounds
		ASTM D5885	
	Oven Aging @ 85°C	ASTM D5721	Per each formulation
	Standard OIT (min. avg.)	ASTM D3895	Per each formulation
	- % retained after 90 days		
	UV Resistance ³	ASTM D7238	Per each formulation
	High Pressure OIT (min.	ASTM D5885	
	avg.) - % retained after		
	1,600 hours		
	Asperity Height	ASTM D7466	Every 2 nd roll ⁴

¹ All tests will conform to the minimum requirements set forth by GRI testing standard GM13 and will meet manufacturer's standards. Required values for the parameters are listed in Table 3-2.

² ASTM D1593 may also be used for thickness of textured geomembrane at the option of the POR.

³ 20 hours of UV cycle at 75°C followed by 4 hours condensation at 60°C.

⁴ Measurement side will be alternated for double-sided textured sheet. This testing is specified for textured geomembrane only.

#### 3.3.3 Geomembrane Installation

**Surface Preparation.** Prior to any geomembrane installation, the installed soil liner or GCL will be inspected by the CQA and geosynthetics contractor. ODEQ shall be notified at least 48 hours before installation of the geomembrane. The POR or CQA monitor must observe the following:

- All lines and grades for the soil liner or GCL have been verified by the surveyor and accepted by the contractor for geosynthetic installation. The POR or his representative, the owner, and geosynthetic installer will certify and accept in writing the finished final lift of the soil liner.
- The soil liner or GCL has been prepared in accordance with the earthwork construction plans and specifications as outlined in Section 2.

- The soil liner or GCL surface is free of surface irregularities and protrusions. The soil liner will be rolled and compacted to ensure a clean surface.
- The soil liner or GCL surface does not contain stones or other objects that could damage the geomembrane and underlying soil liner or GCL. The surface of the soil liner or GCL will be smooth and free of foreign and organic material, sharp objects, exposed soil, aggregate particles greater than 1 inch (or less if recommended by the geosynthetic manufacturer), surface rocks, or other deleterious material.
- The anchor trench dimensions have been checked, and the trenches are free of sharp objects and stones.
- There are no excessively soft areas in the soil liner that could result in geomembrane damage.
- The geomembrane will not be placed over soil liner or GCL during inclement weather such as rain or high winds.
- The soil liner or GCL is not saturated, and no standing water is present above the soil liner or GCL .
- The soil liner has not desiccated (e.g., areas with desiccation cracks).
- All construction stakes and hubs have been removed and the resultant holes have been backfilled. There are no rocks, debris, or any other objects on the soil liner or GCL surface.
- The geosynthetics contractor has certified in writing that the soil liner or GCL surface on which the geomembrane will be installed is acceptable.

**Panel Placement.** Prior to the installation of the geomembrane, the contractor must submit drawings showing the panel layout, indicating panel identification number, both fabricated (if applicable) and field seams, as well as details not conforming to the drawings.

The CQA monitor must maintain an up-to-date panel layout drawing showing panel numbers that are keyed to roll numbers on the placement log. The panel layout drawing will also include seam numbers and destructive test locations.

During panel placement, the POR or CQA monitor must:

- Observe that geomembrane is placed in direct and uniform contact with the underlying soil liner or GCL .
- Record roll numbers, panel numbers, and dimensions on the panel or seam logs. Measure and record thickness of leading edge of each panel at 5-foot maximum intervals. No single thickness measurement can be less than 10 percent below the required nominal thickness.

#### Table 3-2

#### Minimum Required Properties of 60-mil-thick Smooth and Textured (Both Sides) HDPE Geomembranes

<b>.</b> .		Minimum Required Property ⁸	
Property	lest Method	Smooth	Textured
Thickness, mils			
Minimum average	ASTM D5199 (smooth)	60	57
Lowest individual reading	ASTM D5994 (textured)	54	51
Lowest individual of 8 of 10 readings		NA	54
Density, g/cc	ASTM D1505/D792	0.940	0.940
Asperity Height, mils ⁹	ASTM D7466	NA	16
Tensile Properties ¹	ASTM D6693		
1. Yield Strength, lb/in	(Type IV Specimen	126	126
2. Break Strength, lb/in	@ 2 in/min)	228	90
3. Yield Elongation, %	(ASTM D638 may be used	12	12
4. Break Elongation, %	as an alternative upon	700	100
	approval by POR)		
Tear Resistance, lb	ASTM D1004	42	42
Puncture Resistance, lb	ASTM D4833	108	100
Stress Crack Resistance ² , hrs	ASTM D5397	500	500
Carbon Black Content ³ , %	ASTM D4218	2.0 - 3.0	2.0 - 3.0
Carbon Black Dispersion ⁴ , Category	ASTM D5596	1 or 2 and 3	1 or 2 and 3
Oxidative Induction Time (OIT) ⁵			
(Minimum Average)			
Standard OIT, minutes	ASTM D8117	100	100
High Pressure OIT, minutes	ASTM D5885	400	400
Oven Aging at 85°C	ASTM D5721		
Standard OIT – % retained after 90	ASTM D8117	55	55
days	ASTM D5885	80	80
High Pressure OIT – % retained after			
90 days			
UV Resistance ⁶	ASTM D5885		
High Pressure OIT ⁷ – % retained after		50	50
1600 hrs			
Seam Properties (4 out of 5 specimens.	ASTM D6392		
5 th specimen can be as low as 80% per			
GRI-GM19)			
1. Shear Strength, lb/in		120	120
2. Peel Strength, lb/in		91 & FTB	91 & FTB
		(78, Extrusion	(78, Extrusion
		Weld)	Weld)

¹ Machine direction (MD) and cross machine direction (XMD) average values will be on the basis of 5 test specimens each direction. Yield elongation is calculated using a gauge length of 1.3 inches; break elongation is calculated using a gauge length of 2.0 inches.

² The yield stress used to calculate the applied load for the Single Point Notched Constant Tensile Load (SP-NCTL) test will be the mean value via MQC testing.

³ Other methods such as ASTM D4218 or microwave methods are acceptable if an appropriate correlation can be established.

⁴ Carbon black dispersion for 10 different views in Categories 1 and 2 and 1 in Category 3.

⁵ The manufacturer has the option to select either one of the OIT methods listed to evaluate the antioxidant content in the geomembrane.

 $^{\rm 6}$   $\,$  The condition of the test will be 20 hr UV cycle at 75°C followed by 4 hr. condensation at 60°C.

⁷ UV resistance is based on percent retained value regardless of the original HP-OIT value.

⁸ Minimum required properties are based on GRI-GM13, except for the seam properties which are based on GRI-GM19.

 9  Of 10 readings, 8 out of 10 must be  $\geq$  14 mils, and lowest individual reading must be  $\geq$  12 mils.

- Observe the sheet surface as it is deployed and record all panel defects and repair of the defects (panel rejected, patch installed, extradite placed over the defect, etc.) on the repair sheet. All repairs must be made in accordance with the specifications as outlined in Section 3.3.5 and located on a repair drawing.
- Observe that support equipment is not allowed on the geomembrane during handling (see Section 3.7 also).
- Observe that the surface beneath the geomembrane has not deteriorated since previous acceptance.
- Observe that there are no stones, construction debris, or other items beneath the geomembrane that could cause damage to the geomembrane.
- Observe that the geomembrane is not dragged across a surface that could damage the material. If the geomembrane is dragged across an unprotected surface, the geomembrane must be inspected for scratches and repaired or rejected, as necessary.
- Record weather conditions including temperature, wind, and humidity. The geomembrane must not be deployed in the presence of excess moisture (fog, dew, mist, or wind, etc.). In addition, geomembrane will not be placed when standing water or frost is on the ground, unless this requirement is waived by the design engineer and ODEQ and according to GRI GM9. Most federal and state environmental regulations call for special procedures for field seaming of geomembranes when sheet temperatures are less than 32°F. This standard practice is meant to give procedure guidance for seaming of geomembranes at sheet temperatures down to 5°F. Excessive wind is that which can lift and move the geomembrane panels.
- Observe that people working on the geomembrane do not smoke, wear shoes that could damage the liner, or engage in activities that could damage the liner.
- Observe that the method used to deploy the sheet minimizes wrinkles but does not cause bridging and that the sheets are anchored to prevent movement by the wind (the contractor is responsible for any damage to or from windblown geomembrane). Excessive wrinkles will be walked-out or removed at the discretion of the CQA monitor.
- Observe that no more panels are deployed than can be seamed on the same day.
- Observe that there are no horizontal seams on side slopes, and the textured material extends a minimum of approximately 5 feet out past the toe of the slope where textured geomembrane is used.

The CQA monitor must inform both the contractor and the POR of the above conditions.

**Field Seaming.** The contractor must provide the POR with a seam and panel layout drawing and update this drawing daily as the job proceeds. No panels will be seamed until the panel layout drawing has been accepted by the POR. A seam numbering system must provide a unique number for each seam and be agreed to by the POR and contractor prior to the start of seaming operations. One procedure is to identify the seam by adjacent panels. For example, the seam located between Panels 306 and 401 would be Seam No. 306/401.

Prior to geomembrane welding, each welder and welding apparatus (both wedge and extrusion welders), must be tested, at a minimum, at daily start-up and at midday break, or any break that the seaming machine is stopped more than 30 minutes to determine if the equipment is functioning properly. The LIT Report will include the names for each seamer and the time and the temperatures for each seaming apparatus used each day. One trial weld will be taken prior to the start of work. The trial weld sample must be 3 feet long and 12 inches wide, with the seam centered lengthwise. The minimum number of specimens per trial weld test must be two coupons for shear and two coupons for peel. Both the inner and outer welds of dual track fusion welds must be tested for each peel test coupon (or additional coupons will be required). Trial weld samples must comply with "Passing Criteria for Welds" included in Section 3.3.4 – Construction Testing. The CQA monitor must observe all welding operations, quantitative testing of each trial weld for peel and shear, and recording of the results on the trial weld form. The trial weld be completed under conditions similar to those under which the panels will be welded. Regarding the locus-of-break patterns of the different seaming methods in shear and peel, the following are unacceptable break codes per their description in ASTM D6392 and GRI-GM19:

Hot Wedge:AD and AD-Brk>25%Extrusion Fillet:AD1, AD2, AD-WLD (unless strength is achieved)

Additionally, there will be no apparent weld separation (i.e., greater than 1/8 inch). The third party strength tests must meet the manufacturer's specifications for the sample sheets, or the percentage of the manufacturer's parent sheet strength as determined by the manufacturer. For dual-track fusion welds, both sides (the inner and outer weld) must meet the minimum requirements for a satisfactory peel test. If, at any time, the CQA monitor believes that an owner or welding apparatus is not functioning properly, a weld test must be performed. If there are wide changes in temperature ( $\pm 30^{\circ}$  Fahrenheit), humidity, or wind speed, the test weld will be repeated. The test weld must be allowed to cool to ambient temperature before testing. If a welded area fails the shear or peel test, the length of the non-passing weld will be identified at a 10-foot interval and the failed area will be patched. Patching will performed by placing additional geomembrane over the failed area or removing the failed area geomembrane weld and patching it with additional geomembrane per POR's direction. Welding for patches must comply with the welding passing criteria requirements outlined in this section.

Construction quality assurance documentation of trial seam procedures will include, at a minimum, the following:

- Documentation that trial seams are performed by each welder and welding apparatus prior to commencement of welding and prior to commencement of the second half of the workday.
- The welder, the welding apparatus number, time, date, ambient air temperature, and welding machine temperatures.

During geomembrane welding operations, the CQA monitor must observe the following:

- The contractor has the number of welding apparatuses and spare parts necessary to perform the work.
- Equipment used for welding will not damage the geomembrane.
- The extrusion welder is purged prior to beginning a weld until all the heat-degraded extradite is removed (extrusion welding only).
- Seam grinding has been completed less than one hour before seam welding, and the upper sheet is beveled (extrusion welding only).
- The ambient temperature, measured 6 inches above the geomembrane surface, is no less than 32° Fahrenheit unless more stringent limits are required by the manufacturer.
- The end of old welds, more than 5 minutes old, are ground to expose new material before restarting a weld (extrusion welding only).
- The contact surfaces of the sheets are clean, free of dust, grease, dirt, debris, and moisture prior to welding.
- The weld is free of dust, rocks, and other debris.
- The seams are overlapped a minimum of 3 inches for extrusion and hot-wedge welding, or in accordance with manufacturer's recommendations, whichever is more stringent. Panels will be overlapped (shingled) in the downgrade direction.
- No solvents or adhesives are present in the seam area.
- The procedure used to temporarily hold the panels together does not damage the panels and does not preclude CQA testing.
- The panels are being welded in accordance with the plans and specifications that will be developed in accordance with this section for each liner construction. Seams will be oriented parallel to the line of maximum slope with no horizontal seams on side slopes. In corners and odd-shaped geometric locations, the number of field seams will be minimized.

- There is no free moisture in the weld area.
- Measure surface sheet temperature every two hours.
- Observe that at the end of each day or installation segment, all unseamed edges are anchored with sandbags or other approved device. Penetration anchors will not be used to secure the geomembrane.

#### **3.3.4** Construction Testing

**Nondestructive Seam Testing.** The purpose of nondestructive testing is to detect discontinuities or holes in the seam. It also indicates whether a seam is continuous and non-leaking. Nondestructive tests for geomembrane include vacuum testing and air pressure testing. Nondestructive testing must be performed over the entire length of the seam.

Nondestructive testing is performed entirely by the contractor. The CQA monitor's responsibility is to document the date, time and location of seaming and testing, and to observe and document that testing was performed in compliance with this section and document any seam defects and their repairs.

Nondestructive testing procedures are described below.

- For welds tested by vacuum method, the weld is placed under suction utilizing a vacuum box made of rigid housing with a transparent viewing window, a soft neoprene rubber gasket attached to the open bottom perimeter, a vacuum gauge on the inside, and a valve assembly attached to the vacuum hose connection. The box is placed over a seam section, which has been thoroughly saturated with a soapy water solution (1 oz. soap to 1 gallon water). The rubber gasket on the bottom perimeter of the box must fit snugly against the soaped seam section of the liner, to ensure a leak-tight seal. The vacuum pump is energized, and the vacuum box pressure is reduced to approximately 3 to 5 psi gauge. Any pinholes, porosity or nonbonded areas are detected by the appearance of soap bubbles in the vicinity of the defect. Dwell time must not be less than ten seconds.
- Air pressure testing is used to test double seams with an enclosed air space. Both ends of the air channel will be sealed. The pressure feed device, usually a needle equipped with a pressure gauge, is inserted into the channel. Air is then pumped into the channel to a minimum pressure of 30 psi or ½ psi per mil of geomembrane thickness, whichever is greater. The air chamber must sustain the pressure for five minutes without losing more than 4 psi. Following a passed pressure test, the opposite end of the tested seam must be punctured to release the air. The pressure gauge must return to zero; if not, a blockage is most likely present in the seam channel. Locate the blockage and test the seam on both sides of the blockage. The penetration holes must be sealed after testing.

During nondestructive testing, the CQA monitor must perform the following work:

- Review technical specifications regarding test procedures.
- Observe that equipment operators are fully trained and qualified to perform their work.
- Observe that test equipment meets project specifications that will be developed in accordance with this QA/QC Plan for each liner construction.
- Observe that the entire length of each seam is tested in accordance with the specifications outlined in this section.
- Observe all continuity testing and record results on the appropriate log.
- Observe that all testing is completed in accordance with the specifications outlined in this section.
- Identify the failed areas by marking the area with a waterproof marker compatible with the geomembrane and inform the contractor of any required repairs, then record the repair area on the repair log.
- Observe that all repairs are completed and tested in accordance with the project specifications outlined in this section and Section 3.3.5.
- Record all completed and tested repairs on the repair log and the repair drawing.

**Destructive Seam Testing.** Destructive seam tests for geomembrane seams will be performed at intervals of at least one test per 500 linear feet. A destructive testing will also be performed for individual repairs (or additional seaming for the failed seams) of more than 10 feet of seaming. The CQA monitor must perform additional tests if he suspects a seam does not meet specification requirements outlined in this section. Reasons for performing additional tests may include, but are not limited to the following:

- Wrinkling in seam area
- Non-uniform weld
- Excess crystallinity
- Suspect seaming equipment or techniques
- Weld contamination
- Insufficient overlap
- Adverse weather conditions
- Possibility of moisture, dust, dirt, debris, and other foreign material in the seam
- Failing tests

There are two types of destructive testing required for the geomembrane installation: peel adhesion (peel) and bonded seam strength (shear) in accordance with ASTM D6392. The purpose of peel and shear tests is to evaluate seam strength and to evaluate long-term performance. Shear strength measures the continuity of tensile strength through the seam and into the parent material. Peel strength determines weld quality. Test welds must be allowed to cool naturally to ambient temperature prior to testing.

The CQA monitor selects locations where seam samples will be cut for laboratory testing. Select these locations as follows:

- A minimum of one random test within each 500 feet of seam length. This is an average frequency for the entire installation; individual samples may be taken at greater or lesser intervals.
- Sample locations will not be disclosed to the contractor prior to completion of the seam.
- A maximum frequency must be agreed to by the contractor, POR, and the Operator at the preconstruction meeting. However, if the number of failed samples exceeds 5 percent of the tested samples, this frequency may be increased at the discretion of the POR. Samples taken as the result of failed tests do not count toward the total number of required tests.

**Sampling Procedures.** The contractor will remove samples at locations identified by the CQA monitor. The CQA monitor must:

- Observe sample cutting.
- Mark each sample with an identifying number, which contains the seam number and destructive test number.
- Record sample location on the panel layout drawing and destructive seam log.
- Record the sample location, weather conditions, and reason sample was taken (e.g., random sample, visual appearance, result of a previous failure, etc.).

For each destructive test obtain one sample approximately 45 inches long by 12 inches wide, with the weld centered along the length. Cut two 1-inch-wide coupons from each end of the sample. The contractor must test two of these coupons in shear and two in peel (one shear and one peel from each end) using a tensiometer capable of quantitatively measuring the seam strengths. For double wedge welding, both sides of the air channel will be tested in peel. The CQA monitor must observe the tests and record the results on the destructive seam test log. A geomembrane seam sample passes the field testing when the break is Film Tear Bond (FTB) and the seam strength meets the required strength values for peel and

shear given previously for trial seams under field seaming and below for third party laboratory testing. As previously discussed, both welds have to pass for dual-track welds. Also, it is recommended that additional samples be obtained as discussed in the following paragraph if there is apparent separation of the weld (i.e., greater than 1/8 inch) during peel testing.

If one or both of the 1-inch specimens fail in either peel or shear, the contractor can, at his discretion: (1) reconstruct the entire seam between passed test locations, or (2) take two additional test samples 10 feet or more in either direction from the point of the failed test and repeat this procedure. For tracking purposes the additional samples will be identified by assigning an identifying letter to the initial destructive test sample number (e.g., DS-6A and B). Only satisfactory tests count toward the required minimum number, and additional tests (i.e., A and B) count as one test, if passing. If the second set of tests pass, the contractor can reconstruct or cap-strip the seam between the two passed test locations. If subsequent tests fail, the sampling and testing procedure is repeated until the length of the poor quality seam is established. Repeated failures indicate that either the seaming equipment or operator is not performing properly, and appropriate corrective action must be taken immediately.

If the field test coupons are satisfactory, divide the remaining sample into three parts: one 12-inch by 12-inch section for the contractor, one 12-inch by 16-inch section for the third party laboratory for testing, and one 12-inch by 12-inch section for the operator to archive. The laboratory sample will be shipped to the third party laboratory for over-night delivery and next day testing.

If the laboratory test fails in either peel or shear, the contractor must either reconstruct the entire seam between passing test locations or recover additional samples at least 10 feet on either side of the failed sample for retesting. Sample size and disposition must be as described in the preceding paragraph. This process is repeated until passed tests bracket the failed seam section. All seams must be bounded by locations from which passing laboratory tests have been taken. Laboratory testing governs seam acceptance. In no case can field testing of repaired seams be used for final acceptance.

**Third Party Laboratory Testing**. Destructive samples must be shipped to the third party laboratory for seam testing. Testing for each sample will include 5 bonded seam shear strength tests and 5 peel adhesion tests (10 for dual-track welds). For dual-track welds each peel test specimen (coupon) will be tested on both sides of the air channel (i.e., the inner and outer welds). At least four of the five specimens tested in peel and shear will meet the minimum strength requirements. The minimum peel strength and the minimum shear strength values must meet the passing criteria listed below. Additionally, 4 out of 5 of the peel test coupons must have no greater than 25 percent seam separation. For dual-track welds if either weld exhibits greater than 25 percent separation or does not meet the required strength, that coupon is considered out of compliance and two out of compliance

coupons cause the weld to fail. The third party laboratory must provide test results within 24 hours, in writing or via telephone, to the CQA monitor. Certified test results are to be provided within 5 days. The CQA monitor must immediately notify the POR in the event of a calibration discrepancy or failed test results.

**Passing Criteria for Welds.** Passing criteria are established by Geosynthetic Research Institute GRI Test Method GM19 for geomembranes. A passing extrusion or fusion welded seam will be achieved when the following values are tested. The following values listed for shear and peel strengths are for 4 out of 5 test specimens (the 5th specimen can be as low as 80 percent of the listed values). Elongation measurements will be omitted for field testing.

- Shear strength (lb/in) 120
- Shear elongation at break (%) 50
- Peel strength (lb/in) 91 (78, Extrusion Weld) & FTB
- Peel separation (%) 25

A passing extrusion or fusion welded seam will be achieved in peel when:

- Yield strength for 4 of 5 specimens (10 tests for dual-track welds) is not less than the above minimum peel strength value and the average of all 5 specimens is not less than the minimum value.
- No greater than 25 percent of the seam width peels (separates) at any point for 4 of 5 specimens (both inner and outer welds for dual-track welds).

A passing extrusion or fusion weld will be achieved in shear when:

- Yield strength for 4 of 5 specimens is not less than the above minimum shear strength value and the average for all 5 specimens is not less than the minimum value.
- Yield strain for 4 out of 5 specimens is at least 25 percent.
- Break strain for 4 out of 5 specimens is at least 50 percent.

#### 3.3.5 Repairs

Any portion of the geomembrane with a detected flaw, or which fails a nondestructive or destructive test, or where destructive tests were cut, or where nondestructive tests left cuts or holes, must be repaired in accordance with the specific liner construction specifications and consistent with all the applicable parts (e.g., material requirement, installation, testing, etc.) of this section. The CQA monitor must locate and record all repairs on the repair sheet and panel layout drawing. Repair techniques include the following:

- Patching used to repair large holes, tears, large panel defects, undispersed raw materials, contamination by foreign matter, and destructive sample locations.
- Extrusion used to repair small defects in the panels and seams. In general, this procedure will be used for defects less than -inch in the largest dimension.
- Capping used to repair failed welds or to cover seams where welds or bonded sections cannot be nondestructively tested.
- Removal used to replace areas with large defects where the preceding methods are not appropriate. Also used to remove excess material (wrinkles, fishmouths, intersections, etc.) from the installed geomembrane. Areas of removal will be patched or capped.

Repair procedures include the following:

- Abrade geomembrane surfaces to be repaired (extrusion welds only) no more than one hour prior to the repair.
- Clean and dry all surfaces at the time of repair.
- Extend patches or caps at least 6 inches beyond the edge of the defect, and round all corners of material to be patched and the patches to a radius of at least 3 inches. Bevel the top edges of patches prior to extrusion welding.
- Testing of repaired seams consistent with Section 3.3.4 Construction Testing.

#### 3.3.6 Wrinkles

During placement of cover materials over the geomembrane, temperature changes or creep can cause wrinkles to develop in the geomembrane. Any wrinkles which can fold over must be repaired either by cutting out excess material or, if possible, by allowing the liner to contract by temperature reduction. In no case can material be placed over the geomembrane, which could result in the geomembrane folding. The CQA monitor must monitor geomembrane for wrinkles and notify the contractor if wrinkles are being covered by soil. The CQA monitor is then responsible for documenting corrective action to remove the wrinkles.

#### 3.3.7 Folded Material

All folded geomembrane must be removed. Remnant folds evident after deployment of the roll, which are due to manufacturing process, are acceptable.

#### 3.3.8 Geomembrane Anchor Trench

The geomembrane anchor trench will be left open until seaming is completed. Expansion and contraction of the geomembrane will be accounted for in the liner placement. Prior to backfilling, the depth of penetration of the geomembrane into the anchor trench must be verified by the CQA monitor. The anchor trench will be filled in the morning when temperatures are coolest to reduce bridging of the geomembrane.

#### **3.3.9 Geomembrane Acceptance**

The contractor retains all ownership and responsibility for the geomembrane until acceptance by the Operator. In the event the contractor is responsible for placing cover over the geomembrane, the contractor retains all ownership and responsibility for the geomembrane until all required documentation is complete, and the cover material is placed. After panels are placed, seamed, tested successfully, and any repairs are made, the completed installation will be walked by the Operator's and contractor's representatives. Any damage or defect found during this inspection will be repaired properly by the installer. The installation will not be accepted until it meets the requirements of both representatives. In addition, the geomembrane will be accepted by the POR only when the following has been completed:

- The installation is finished.
- All seams have been inspected and verified to be acceptable.
- All required laboratory and field tests have been completed and reviewed.
- All required contractor-supplied documentation has been received and reviewed.
- All as-built record drawings have been completed and verified by the POR. The as-built drawings show the true panel dimensions, the location of all seams, trenches, pipes, appurtenances, and repairs.
- Acceptance of the LIT Report by ODEQ.

#### 3.3.10 Bridging

Bridging must be removed.

# 3.4 Geotextiles

Geotextiles will be used to prevent clogging of drainage materials and as a cushion to protect the geomembrane. The main usage of geotextiles will be enveloping drainage stone used for chimney drains in the leachate collection system (LCS). Geotextiles for the LCS will meet the design requirements set forth in Table 3-3 of this QA/QC Plan.

#### 3.4.1 Delivery

During delivery the CQA monitor must observe the following:

- Equipment used to unload the rolls will not damage the geotextile.
- Rolls are wrapped in impermeable and opaque protection covers.
- Care is used when unloading the rolls.
- All documentation required by this QA/QC Plan and the specifications has been received and reviewed for compliance with this QA/QC Plan.
- Each roll is marked or tagged with the manufacturer's name, project identification, lot number, roll number, and roll dimensions.
- Materials are stored in a location that will protect the rolls from precipitation, mud, dirt, dust, puncture, cutting, or any other damaging or deleterious conditions.

Any damaged rolls must be rejected and removed from the site or stored at a location separate from accepted rolls, designated by the Operator. All rolls which do not have proper manufacturer's documentation must also be stored at a separate location until all documentation has been received and approved.

#### 3.4.2 Testing

The geotextile manufacturer will conduct manufacturer quality control (MQC) testing and certify that the materials delivered to the site comply with project specifications outlined in this QA/QC Plan. The material certification will be reviewed by the POR and approved for the project prior to acceptance of any of the material. The MQC testing will include the following tests with at least one test for each 100,000 square feet of geotextile delivered.

- Grab tensile strength/elongation (ASTM D4632)
- Mass per unit area (ASTM D5261)
- Puncture strength (ASTM D6241)
- Trapezoidal tear strength (ASTM D4533)
- Permittivity (ASTM D4491)
- Apparent opening size (ASTM D4751)

Where optional procedures are noted in the test method, the specification requirements of this QA/QC Plan prevail. The POR will review all test results and

report any nonconformance. Geotextile testing will conform to current specifications found in GRI Test Method GT13a.

Responsible Party	Material	Test	Standard	Required Property ^{2, 3}
Manufacturer	Geotextile	Unit Weight	ASTM D5261	8 oz/sy
		Apparent Opening Size	ASTM D4751	0.024 in (max)
		Grab Strength	ASTM D4632	158 lb
		Tear Strength	ASTM D4533	56 lb
		Puncture Strength	ASTM D6241	320 lb
		Permittivity	ASTM D4491	0.02 s ⁻¹

# Table 3-3Required Testing and Properties of Geotextile1

¹ The minimum testing frequency will be one test sample per 100,000 square feet.

² All values listed are minimum except apparent opening size is maximum.

³ Required properties are based on GRI-GT13a, Class 2, Elongation  $\ge$  50%.

#### 3.4.3 Geotextile Installation

**Surface Preparation.** Prior to geotextile installation, the CQA monitor must observe the following:

- All lines and grades have been verified by the surveyor.
- The supporting surface does not contain stones that could damage the geotextile or the underlying geomembrane.
- There are no excessively soft areas that could result in damage to the geotextile, or other components of the liner system.
- Construction stakes and hubs have been removed.

**Geotextile Placement**. During geotextile placement, the CQA monitor must:

- Observe the geotextile as it is deployed, and record all defects and disposition of the defects (panel rejected, patch installed, etc.). Repairs are to be made in accordance with the specifications outlined in Section 3.4.4.
- Observe that equipment used does not damage the geotextile by handling, equipment transit, leakage of hydrocarbons, or other means.
- Observe that people working on the geotextile do not smoke, wear shoes that could damage the geotextile, or engage in activities that could damage the geotextile.
- Observe that the geotextile is securely anchored in an anchor trench.
- Observe that the geotextiles are anchored to prevent movement by the wind.

- Observe that the panels are overlapped a minimum of six inches.
- Examine the geotextile after installation to ensure that no potentially harmful foreign objects are present.
- Observe that seams (where required) are continuously sewn or thermal bonded in accordance with the manufacturer's recommendations and the project specifications outlined in this QA/QC Plan.

The CQA monitor must inform both the contractor and POR if the above conditions are not met.

#### 3.4.4 Repairs

Repair procedures include:

- Patching used to repair large holes, tears, large defects, and destructive sample locations.
- Removal used to replace areas with large defects where the preceding method is not appropriate.

Holes, tears, and defects must be repaired in the following manner. Soil or other material which may have penetrated the defect must be removed completely prior to repair. If located on a slope, the defect must be patched using the same type of geotextile and double-seamed into place. Should any tear, hole, or defect exceed 30 percent of the width of the roll, the roll will be cut off and the defect removed or the roll removed and replaced. If the defect is not located on a slope, the patch must be made using the same type of material seamed into place with a minimum of 24 inches overlap in all directions. Seams will be either thermal bonded or sewn in accordance with the manufacturer's recommendations.

# 3.5 Geosynthetic Clay Liner (GCL)

An alternative composite liner system consisting of a 1-foot-thick compacted clay liner ( $k \le 1 \times 10^{-7}$  cm/s) overlain by a Geosynthetic Clay Liner (GCL) may be utilized in lieu of the 2-foot-thick compacted clay liner. Material properties based on Geosynthetic Research Institute recommendations described in GRI-GCL3 have been included in Table 3-5 – Required Properties for Reinforced GCL Materials. The GCL used for the alternative liner system will meet or exceed the required properties. Only Reinforced GCL will be utilized. Unreinforced GCL will not be utilized.

#### 3.5.1 Delivery

The GCL will be labeled and shipped in rolls, which are wrapped individually in relatively impermeable and opaque protective covers. GCL must be rolled by the

manufacturer in a fashion to prevent collapse during transit. The GCL rolls must be stored above ground (i.e., wooden pallets) and covered with a waterproof tarpaulin.

A dedicated storage area will be selected at the job site or at an alternate offsite area per owner's direction. The selected area will be level, dry, and well drained. Rolls will be stored in a manner that prevents sliding or rolling from the stacks. Rolls should be stacked no higher than three rolls to protect the integrity of roll cores and ensure safe material handling. Stored GCL materials will be covered with a plastic sheet or tarpaulin until it is installed. The integrity and legibility of the labels will be preserved during storage.

Visual inspection of each GCL roll will be made during unloading to identify any packaging that has been damaged. Rolls with damaged packaging will be marked and set aside for further inspection. The packaging will be repaired, for acceptable GCL rolls, prior to being placed in storage. If necessary, the party responsible for unloading the GCL will contact the manufacturer prior to shipment to ascertain the suitability of the proposed unloading methods and equipment.

#### 3.5.2 Conformance Testing

A reinforced GCL which consists of bentonite encapsulated between two geotextiles, one nonwoven and one woven, which are needle punched together will be used for liner sideslopes. Unreinforced GCL will not be utilized. The GCL materials and its components will be tested in accordance with Table 3-4 by the supplier/GCL manufacturer and a third party independent laboratory and will have the required values listed in Table 3-5. A certificate of analysis for each GCL roll will be submitted as part of the quality control documentation. Manufacturer hydraulic conductivity testing of GCL seams must be performed by using a flow box or other suitable device per adjoining material and type. Hydraulic conductivity value must be equal to or less than the specified hydraulic conductivity value for the GCL (5x10⁻⁹ cm/s). The manufacturer will provide recommended seaming procedures and supporting test (flow box or other suitable device). The manufacturer will provide documentation showing the GCL seams are no more permeable than the GCL itself at a confining pressure anticipated in the field.

The manufacturer will provide inspection reports demonstrating that needlepunched nonwoven geotextile were inspected using metal detectors for the presence of broken needles and were found to be needle free.

The POR will review the manufacturer's certification (quality control certificate) and verify that the GCL meets the values given in the plan or specifications for those tests listed in Table 3-4. Required quality control documentation will be submitted to the POR prior to shipment of GCL to the site. Requirements for GCL materials are listed in Table 3-5.

Responsible Party	Test	Type of Test	Standard Test Method	Frequency of Testing
Supplier or GCL Manufacturer	Bentonite ¹	Free Swell	ASTM D5890	per 50 tons (minimum of 1 test for each construction event)
		Fluid Loss	ASTM D5891	
	Geotextile	Mass/Unit Area	ASTM D5261	per 25,000 sy
GCL Manufacturer	GCL Product	Bentonite Mass/Unit Area	ASTM D5993	per 5,000 sy
		Bentonite Moisture Content	ASTM D5993	
		Tensile Strength	ASTM D6768	per 25,000 sy
		Peel Strength	ASTM D6496	per 5,000 sy
		Permeability ²	ASTM D5887	per 30,000 sy
		Lap Joint Permeability	Flow box or other suitable device	per GCL adjoining material and lap type ³
Independent Laboratory (Conformance Testing)	GCL Product	Bentonite Mass/Unit Area	ASTM D5993	per 100,000 sf
		Permeability	ASTM D5887	* *

#### Table 3-4 **Required Testing for GCL Materials⁴**

Tests to be performed on bentonite before incorporation into GCL.
Report last 20 permeability values, ending on production date of supplied GCL.
May also be done as conformance testing.

⁴ Testing requirements based on current GRI-GCL3.

Table 3-5Required Properties for Reinforced GCL Materials

Broporty	Required Values ¹	
Property	Reinforced GCL	
Free Swell (milliliter/2g)	24 (minimum)	
Fluid Loss (milliliters)	18 (maximum)	
Geotextile Mass per Unit Area (oz/yd ² )		
Nonwoven	5.8 (minimum)	
• Woven	3.0 (minimum)	
Bentonite Mass per Unit Area ² (lb/sf)	0.75 (minimum)	
Bentonite Moisture Content	35 (maximum)	
Tensile Strength (lb/in)	23 (minimum)	
Peel Strength ³ (lb/in)	2.1 (minimum)	
GCL Permeability (cm/s) ⁴	5x10 ⁻⁹ (maximum)	
Lab Joint Permeability (cm/s) ^{5,6}	5x10 ⁻⁹ (maximum)	

¹ Manufacturer will demonstrate that the above listed values will be met prior to shipment in accordance with Table 3-5. Required values are based on current GRI-GCL3 standards.

² Bentonite mass per unit area of GCL must be reported at zero percent moisture content for the finished product.

³ Value is required for GCL and geotextile.

⁴ Permeability is listed for the finished product at a gradient of 1.0.

⁵ Minimum overlap is 2 feet. The values listed are minimum dry bentonite amount for 1 foot of overlap. Manufacturer specified value will be used if it is higher.

⁶ Manufacturer will provide certification that seams are no more permeable than the GCL material under similar normal stress conditions.

#### 3.5.3 GCL Installation

Installation of GCL will have continuous on-site monitoring during construction by the POR or his designated representative. The installer will provide a panel layout plan, which will be reviewed by the POR prior to any material deployment. The POR must review field conditions and approve revised panel layout plan if the field conditions vary from the original plan layout.

#### Surface Preparation

The surface of subgrade for the GCL installation will be stable. It will be smooth and free of foreign and organic material, sharp objects, exposed soil or aggregate particles greater than 3/4 inch (or less if recommended by the manufacturer), or other deleterious materials. Standing water or excessive water on the soil liner will not be allowed. Prior to GCL installation, the POR will verify the following:

- The grades below the GCL have been verified and accepted by the GCL contractor.
- Required documentation for constructed layers and soil liner preparation below the GCL have been completed and are acceptable.

• The supporting surface has been rolled to provide a smooth surface and does not contain materials, which could damage the GCL or adjacent layer. The soil liner will be rolled with a smooth-drum compactor. Protrusions extending more than 1 inch (or less if recommended by the manufacturer) from the soil liner surface, as well as all surface rocks, will be removed.

#### Deployment

Equipment used to deploy GCL over soil must not cause excessive rutting of the soil liner. Construction equipment (other than low contact pressure rubber-tired vehicles such as ATVs or golf carts) on the GCL will not be allowed. Deployed GCL panels should contain no folds or excessive slack. Generators, gasoline or solvent cans, tools, or supplies must not be stored directly on GCL and will be placed on scrap FML (rub sheet). Installation personnel must not smoke or wear damaging shoes when working on GCL.

GCL seams will be constructed overlapping their adjacent edges. GCL seams will be constructed per manufacturer's directions. GCL on sideslopes must not be unrolled in a direction perpendicular to the direction of the slope. GCL should be anchored temporarily (e.g., sandbags) at the top of the slope to prevent wrinkles and folds.

Horizontal seams will only be allowed on the slopes under one of the following conditions:

- 2 feet of overlap with horizontal seams being staggered.
- 1 foot of overlap with the underlying panel having a 1-foot runout anchored with 6 inches of either compacted clay liner or subgrade.

The POR or his designated representative will observe the GCL as it is deployed for even bentonite distribution, thin spots, or other panel defects. Defects and the disposition of the defects (panel rejected, patch installed, etc.) will be recorded. Repairs are to be made in accordance with the specifications at the discretion of the POR. The POR will verify that only panels that can be covered on the same day with a FML are deployed and that the GCL panels are not placed during wet, rainy weather or impending rain. In accordance with the construction specifications, the POR will also verify the following:

- Proper GCL deployment techniques.
- Proper overlap during deployment.
- Seams between GCL panels are constructed per manufacturer's recommendations.
- The bentonite does not exceed the specified amount of hydration prior to covering.

- Defects are patched and overlapped properly.
- On sideslopes, the GCL is anchored at the top and then unrolled.
- Observe that no debris is trapped beneath or within the GCL.
- Observe that broken needle pieces do not exist within needle-punched GCL.
- Observe that wind speed is less than 40 miles per hour unless a lower wind speed is recommended by the manufacturer.

The POR will observe the GCL for premature hydration visually and by walking over the GCL to locate soft spots. GCL that has prematurely hydrated according to the specifications will be removed and replaced with new GCL. These observations will be documented in the LIT.

The CQA monitor will verify that GCL (or overlying geosynthetics) are not displaced or damaged while overlying materials are being placed.

#### GCL Anchor Trench

The GCL anchor trench will be left open to allow installation of FML. Temporary anchoring will be provided until the placement of FML by using sandbags as discussed in Section 3.5.3.2. Slightly rounded corners will be provided in anchor trenches where the GCL enters the trench so as to avoid sharp bends in the GCL. No loose soil (e.g., excessive water content) will be allowed to underlie the anchored components of liner system. Backfilling of soil will be in accordance with Section 2.2.5.

#### Patching

Torn or otherwise damaged GCL (with no loss of bentonite from the GCL) must be patched with the same type of GCL. The GCL patch must extend at least 12 inches beyond the damaged area and must be bonded to the main GCL to avoid shifting during backfilling. If the GCL damage includes loss of bentonite, the patch must consist of full GCL extending at least 12 inches beyond the damaged area. Lapping procedures must be the same as specified for original laps of GCL panels.

# **3.6 Equipment on Geosynthetic Materials**

Construction equipment on the liner system will be minimized to reduce the potential for liner puncture. The CQA monitor will verify that small equipment such as generators are placed on scrap liner material (rub sheets) above geosynthetic materials in the liner system. Aggregate drainage layers and/or protective cover will be placed using low ground pressure equipment. The CQA monitor will verify that the geosynthetics are not displaced while the soil layers are being placed.

Unless otherwise specified by the POR, all lifts of protective soil material placed over geosynthetics will conform with the following guidelines.

<u>Equipment Ground Pressure (psi)</u>	<u>Minimum Lift Thickness (in)</u>
<5.0	12
5.1 - 8.0	18
8.1 - 16.0	24
>16.0	36

No equipment will be left running and unattended over the lined area.

# 3.7 Reporting

The POR will submit to the ODEQ a LIT Report for approval of the flexible membrane liner, leachate collection system and protective cover. Section 6 describes the documentation requirements.

# 4 CONSTRUCTION QUALITY ASSURANCE FOR PIPING

#### 4.1 Introduction

This section describes CQA procedures for the installation of HDPE pipe for the leachate collection system used for the composite liner. This plan stresses careful documentation during the quality assurance process, from the selection of materials through installation.

The goal of the pipe quality assurance program is to assure that proper construction techniques and procedures are used, and that the project is built in accordance with the project construction drawings and specifications that will be developed in accordance with this QA/QC Plan for each liner construction. The following specifications apply to the leachate collection system piping:

- Minimum internal diameter = 5.845 inches for leachate collection pipe and nominal diameter of 18 inches for riser pipe
- Standard dimension ratio = 17
- Perforation hole diameter = minimum 0.5 inches and no greater than 0.5 inches (if slotted pipe is used, standard slot width = 0.125 inches)
- Perforation spacing = two (2) rows drilled at an angle between 45° and 60° from vertical.

The quality assurance program is intended to identify and define problems that may occur during construction and to observe that these problems are corrected before construction is complete. A construction report, prepared after project completion, will document that the constructed facility meets design standards and specifications.

# 4.2 Pipe and Fittings

#### 4.2.1 General

Construction must be conducted in accordance with the project construction drawings and specifications for each liner constructed. To monitor compliance, a quality assurance program will be implemented that includes: (1) a review of the manufacturer's quality control testing, (2) material conformance testing, and (3) construction monitoring. Conformance testing refers to testing by an independent

third party laboratory that will take place prior to material installation on materials delivered to the site.

### 4.2.2 Delivery

The CQA monitor will observe:

- That upon delivery, the pipe and pipe fittings are in compliance with the requirements of the construction specifications that will be developed in accordance with this QA/QC Plan for each liner construction.
- That a storage location is selected in which the pipe and pipe fittings are protected from excessive heat, cold, construction traffic, hazardous chemicals, and solvents. If the pipe and pipe fittings are stored at a location where other construction materials are present, the CQA monitor will assure that stacking or insertion of the other construction materials onto or into the pipe and pipe fitting is prohibited. The CQA monitor will periodically examine the storage area to observe that the pipe fittings are undamaged, and have been protected.
- That upon transporting pipe and fittings from the storage location to the construction site, the contractor will use pliable straps, slings, or rope to lift the pipe. Steel cables or chains will not be allowed to transport or lift the pipe.
- That the contractor will provide that a pipe greater than 20 feet in length will be lifted with at least two support points. The contractor will not drop, impact, or bump into the pipe, particularly at the pipe ends. Pipe and fitting ends must be cleaned of all dirt, debris, oil, or any other contaminant which may prohibit making a sound joint.

The CQA monitor will document all activities associated with the handling and storage of this material in order to maintain compliance with this portion of the CQA plan.

# 4.2.3 Conformance Testing

Prior to the installation of pipe, the pipe manufacturer will provide to the Operator and the POR a quality control certificate for each lot or batch of pipe provided. The quality control certificate will be signed by a responsible party employed by the pipe manufacturer, such as the quality control manager. The quality control certificate and documentation will include:

- A description of the pipe delivered to the project, including but not limited to the strength classification, diameter, perforations, and production lot.
- Properties sheet including, at a minimum, all specified properties, measured using test methods indicated in the specifications that will be developed in accordance with this QA/QC Plan for each liner construction, or equivalent.

- A certification that property values given in the properties sheet are minimum values and are guaranteed by the pipe manufacturer.
- A list of quantities and descriptions of materials other than the base resin which comprise the pipe.
- The sampling procedure and results of testing for actual samples manufactured in the same lot as the pipe delivered to the project.

The CQA monitor will observe that:

- The property values certified by the pipe manufacturer meet all of the specifications that will be developed in accordance with this QA/QC Plan for each liner construction.
- The measurements of properties by the pipe manufacturer are properly documented and that the test methods used are acceptable.
- Verification that the quality control certificates have been provided at the specified frequency for all lots or batches of pipe, and that each certificate identifies the pipe lot/batch related to it.
- The certified properties meet the specifications that will be developed in accordance with this QA/QC Plan for each liner construction.

#### 4.2.4 Pipe and Fitting Installation

**Surface Preparation**. Prior to pipe installation, the CQA monitor must observe the following:

- All lines and grades have been verified by the contractor and project surveyor.
- The pipe trenches are swept clean of any deleterious material which may damage the pipe, geosynthetic or may clog the pipe.
- Pipe perforations for leachate collection system are drilled in the pipe outside of the drainage trench where the pipe is to be laid. The drill cuttings must be completely removed from the pipe prior to being placed in the drainage trench.
- Pipe perforations are to the correct size and spacing according to the project specifications that will be developed in accordance with this QA/QC Plan for each liner construction. Perforations can be either factory installed slots or factory predrilled holes or field drilled holes.

**Pipe and Fitting Placement**. During pipe and fitting installation, the CQA monitor will:

• Observe all pipe, pipe fittings, and joints as the pipe is being laid. The CQA monitor will observe that pipes and fittings are not broken, cracked, or

otherwise damaged or unsatisfactory. Prior to fusing (if fusion welding is utilized), the pipe installer will provide for a fusion surface area which is clean and free of moisture, dust, dirt, debris of any kind, and foreign material.

- If fusion welding is utilized, verify welder credentials and that the procedure is consistent with the pipe manufacturer's recommendations.
- Observe that the pipe and fittings are being constructed in accordance with specifications that will be developed in accordance with this QA/QC Plan for each liner construction and accepted practices.
- Observe that the people and equipment utilized to install the pipe do not damage the pipe or any other component of the liner system.

# 5 CONSTRUCTION QUALITY ASSURANCE FOR DRAINAGE MATERIAL

#### 5.1 Drainage Aggregate

The drainage aggregates that are placed in the leachate collection trench will have a hydraulic conductivity of at least  $1 \times 10^{-2}$  cm/s. The granular drainage material should be tested by the supplier for gradation (ASTM D422) and hydraulic conductivity (ASTM D5084) at the supply source at a minimum of 1 test per 5,000 cubic yards. The material shall be free of organics, foreign objects, or other deleterious materials. The physical characteristics of the material shall be evaluated through visual observation and laboratory testing before construction, and visual observations during construction. The material may be tested during construction at the discretion of the CQA monitor.

#### 5.2 Installation

The drainage aggregate will be placed on top of a geotextile that overlies the geomembrane using low ground pressure equipment as outlined in Section 3.6. The drainage aggregate shall be placed by spreading a minimum of 12 inches of material in front of the spreading equipment. Under no circumstances shall the construction equipment come in direct contact with the installed geosynthetics.

During construction, the CQA monitor will:

- Verify that underlying geosynthetic installations are not damaged during placement operations, or mark damaged geosynthetics and verify that damage is repaired.
- Monitor haul road thickness over geosynthetic installations and verify that equipment hauling and materials placement meet equipment specifications.

Documentation will consist of daily recordkeeping, testing and installation reports, nonconformance reports (if necessary), progress reports, photographic records, design and specification revisions, and a Liner Installation and Testing (LIT) Plan as required by OAC 252:515-11-6.

# 6.1 Daily Record of Construction Progress

The daily field report will summarize ongoing construction activities and will include the following:

- Date, project name, project number, and location
- Weather
- Summary of daily construction activities
- Equipment list
- Items discussed and names of parties involved in discussions
- A brief description of tests and observations
- Areas of nonconformance and any corrective actions
- Summary of materials received
- Record of site visitors
- Signature of the CQA monitor
- Signature of the POR

# 6.2 Observation and Test Data Sheets

Observation and test data sheets should include the following information:

- Date, project name, and location
- Test equipment calibrations, if applicable
- A summary of test results identified as passing, failing, or in the event of a failed test, retest.

- Signature of the CQA monitor
- Signature of the POR

# 6.3 Photographs

Construction activities may be photographed by the CQA monitor. Photographs will include any significant problems encountered and corrective actions taken, as well as document construction progress. The photographer should document the subject of the photograph, either on the back of the picture, or in a photograph log.

# 6.4 Design and Specification Changes

Design and specification changes may be required during construction. Design and specification changes will only be made with written agreement of the ODEQ, design engineer, owner, and contractor. These changes will be made by change order to the contract.

# 6.5 LIT Report

The POR will submit an LIT Report documenting the construction of the composite liner and leachate collection systems to the ODEQ for approval.

The POR will provide an engineer's certification that the composite liner and leachate collection systems were constructed in accordance with the approved construction drawings and specifications. QA/QC documentation will be included in the LIT Report.

The LIT Report shall be submitted to the ODEQ within 30 days after completion of each phase of composite liner placement. Consistent with OAC 252:515-11-7, the LIT Report shall be placed in the site operating record, and waste shall not be placed within the new phase of composite liner until ODEQ approves the LIT and provides written authorization to commence disposal.

At a minimum, the LIT Report will contain:

- A summary of construction activities.
- A summary of conformance testing.
- A summary of laboratory and field test results.
- Sampling and testing location drawings.
- A summary of repairs and their locations.

- Changes from the construction drawings and specifications and the justification for these changes.
- As-built record drawings.
- A map that shows locations of system components tied to at least two permanent monuments.
- The results of the initial leachate collection pipe clean-out.
- The method for phased tie-in of leachate collection pipes.
- A statement of compliance with the construction contract documents and design intent, signed and sealed by a professional engineer registered in the State of Oklahoma.